



City of  
**ALACHUA**

THE GOOD LIFE COMMUNITY

**FOR PLANNING USE ONLY**

Case #: \_\_\_\_\_  
Application Fee: \$ \_\_\_\_\_  
Filing Date: \_\_\_\_\_  
Acceptance Date: \_\_\_\_\_  
Review Type: P&Z

# Site Plan Application

Reference City of Alachua Land Development Regulations Article 2.4.9

**A. PROJECT**

1. Project Name: PePeDogs
2. Address of Subject Property: 10908 Rachael Blvd., Alachua, Florida
3. Parcel ID Number(s): 03962-000-000 and 03203-000-000
4. Existing Use of Property: Vacant / Undeveloped
5. Future Land Use Map Designation: Commercial
6. Zoning Designation: C1
7. Acreage: 8.04

**B. APPLICANT**

1. Applicant's Status ☐ Owner (title holder) ☒ Agent
2. Name of Applicant(s) or Contact Person(s): Sergio Reyes, P.E. Title: President  
Company (if applicable): eda engineers-surveyors-planners, inc.  
Mailing address: 2404 NW 43rd Street  
City: Gainesville State: Florida ZIP: 32606  
Telephone: 352-373-3541 FAX: 352-373-7249 e-mail: cgmuer@edafl.com
3. If the applicant is agent for the property owner\*:  
Name of Owner (title holder): RBC Bank  
Mailing Address: PO Box 25999  
City: Shawnee Mission State: KS ZIP: 66225

\* Must provide executed Property Owner Affidavit authorizing the agent to act on behalf of the property owner.

**C. ADDITIONAL INFORMATION**

1. Is there any additional contact for sale of, or options to purchase, the subject property? ☒ Yes ☐ No  
If yes, list names of all parties involved: PePeDogs, Inc.  
If yes, is the contract/option contingent or absolute? ☒ Contingent ☐ Absolute

**D. ATTACHMENTS**

1. Site Plan including but not limited to:
  - a. Name, location, owner, and designer of the proposed development.
  - b. Zoning of the subject property.
  - c. Vicinity map - indicating general location of the site and all abutting streets and properties.
  - d. Complete legal description.
  - e. Statement of Proposed Uses.
  - f. Location of the site in relation to adjacent properties, including the means of ingress and egress to such properties and any screening or buffers along adjacent properties.
  - g. Date, north arrow, and graphic scale (not to exceed one (1) inch equal to fifty (50) feet.)
  - h. Area and dimensions of site.
  - i. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
  - j. Access and points of connection to utilities (electric, potable water, sanitary sewer, gas, etc.)
  - k. Location and dimensions of all existing and proposed parking areas and loading areas.
  - l. Location, size, and design of proposed landscaped areas (including existing trees and required landscaped buffer areas) with detail illustrating compliance with Section 6.2.2 of the Land Development Regulations.

City of Alachua ♦ Planning and Community Development Department  
PO Box 9 ♦ Alachua, FL 32616 ♦ (386) 418-6121

Revised 5/1/2014



- m. Location and size of any lakes, ponds, canals, or other waters and waterways.
- n. Structures and major features – fully dimensioned – including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and floor area ratio.
- o. Location of waste receptacles and detail of waste receptacle screening.
- p. For development consisting of one or more of the following: Multi-family residential; Hotel; or Mobile Home Park:
  - i. Tabulation of gross acreage.
  - ii. Tabulation of density.
  - iii. Number of dwelling units proposed.
  - iv. Location and percent of total open space and recreation areas.
  - v. Floor area of dwelling units.
  - vi. Number of proposed parking spaces.
  - vii. Street layout.
  - viii. Layout of mobile home stands (for mobile home parks only).
  - ix. City of Alachua Public School Student Generation Form.

**Sheet Size: 24" X 36" with 3" left margin and ½" top, bottom, and right margins**

- 2. Stormwater management plan - including the following:
  - a. Existing contours at one (1) foot intervals based on U.S. Coastal and Geodetic Datum.
  - b. Proposed finished floor elevation of each building site.
  - c. Existing and proposed stormwater management facilities with size and grades.
  - d. Proposed orderly disposal of surface water runoff.
  - e. Centerline elevations along adjacent streets.
  - f. Water Management District surfacewater management Statement of proposed uses on the site plan
- 3. Fire Department Access and Water Supply: The design criteria shall be Chapter 18 of the Florida Fire Prevention Code. Plans must be on separate sealed sheets and must be prepared by a professional Fire engineer licensed in the State of Florida. Fire flow calculations must be provided for each newly constructed building. When required, fire flow calculations shall be in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office (ISO) and /or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater. All calculations must be demonstrated and provided. All calculations and specifications must be on the plans and not on separate sheets. All fire protection plans are reviewed and approved by the Alachua County Fire Marshal.
- 4. Concurrency Impact Analysis showing the impact on public facilities, including potable water, sanitary sewer, transportation, solid waste, recreation, stormwater, and public schools in accordance with Article 2.4.14 of the Land Development Regulations.
- 5. Analysis of Consistency with the City of Alachua Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies and describe in detail how the application complies with the noted Goal, Objective, or Policy.)

**For commercial project Applications:**

- a. In addition to submitting specific written information regarding your **commercial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your commercial development will comply with these standards.

**Policy 1.3.d      Design and performance standards**

The following criteria shall apply when evaluating commercial development proposals:

- 1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
- 2. Buffering from adjacent existing/potential uses;
- 3. Open space provisions and balance of proportion between gross floor area and site size;
- 4. Adequacy of pervious surface area in terms of drainage requirements;
- 5. Placement of signage;
- 6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
- 7. Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;



8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
11. Commercial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.

**For industrial project Applications:**

- b. In addition to submitting specific written information regarding your **industrial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your industrial development will comply with these standards.

**Policy 1.5.d**

The City shall develop performance standards for industrial uses in order to address the following:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
  2. Buffering from adjacent existing/potential uses;
  3. Open space provisions and balance of proportion between gross floor area and site size;
  4. Adequacy of pervious surface area in terms of drainage requirements;
  5. Placement of signage;
  6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
  7. Safety of on-site circulation patterns (patron, employee and delivery vehicles, trucks), including parking layout and drive aisles, and points of conflict;
  8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
  9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
  10. Performance based zoning requirements that may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
  11. Industrial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres by 5 acres or greater, .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.
6. For Site Plans for Buildings Less than 80,000 Square Feet in Area: One (1) set of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.
- For Site Plans for Buildings Greater than or Equal to 80,000 Square Feet in Area: Two (2) sets of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.
7. Neighborhood Meeting Materials, including:
    - i. Copy of the required published notice (advertisement) – must be published a newspaper of general circulation, as defined in Article 10 of the City's Land Development Regulations
    - ii. Copy of written notice (letter) sent to all property owners within 400 feet and to all persons/organizations registered with the City to receive notice, and mailing labels or list of those who received written notice
    - iii. Written summary of meeting – must include (1) those in attendance; (2) a summary of the issues related to the development proposal discussed; (3) comments by those in attendance about the development proposal; and, (4) any other information deemed appropriate.
  8. Legal description with tax parcel number.
  9. Proof of ownership.
  10. Proof of payment of taxes.



11. Environmental Resource Permit (or Letter of Exemption) from the Suwannee River Water Management District or Self-Certification for a Stormwater Management System in Uplands Serving Less than 10 Acres of Total Project Area and Less than 2 Acres of Impervious Surfaces from the Florida Department of Environmental Protection pursuant to Section 403.814(12), Florida Statutes.
12. If access is from a County Road, access management permit from Alachua County Public Works (or documentation providing evidence that a permit application has been submitted).
13. If access is from a State Road, access management permit from Florida Department of Transportation (or documentation providing evidence that a permit application has been submitted).
14. **Fee.** Please see fee schedule for fee determination. No application shall be accepted for processing until the required application fee is paid in full by the applicant. Any necessary technical review or additional reviews of the application beyond the initial engineering review fee will be billed to the applicant at the rate of the reviewing entity. The invoice shall be paid in full prior to any legislative and/or quasi-judicial action of any kind on the petition, appeal, or development application.

**All 14 attachments are required for a complete application. A completeness review of the application will be conducted within five (5) business days of receipt. If the application is determined to be incomplete, the application will be returned to the applicant.**

I/We certify and acknowledge that the information contained herein is true and correct to the best of my/our knowledge.



Signature of Applicant

Signature of Co-applicant

Sergio Reyes, P.E.

Typed or printed name and title of applicant

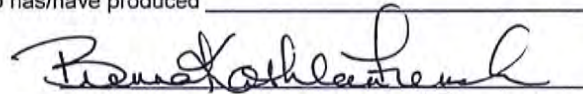
Typed or printed name of co-applicant

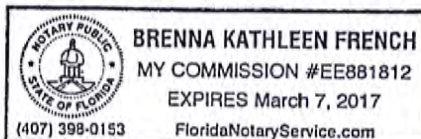
State of Florida

County of Alachua

The foregoing application is acknowledged before me this 2<sup>nd</sup> day of April, 2015 by Sergio Reyes

\_\_\_\_\_, who is/are personally known to me, or who has/have produced \_\_\_\_\_ as identification.

  
Signature of Notary Public, State of Florida



## FLORIDA DEPARTMENT OF STATE DIVISION OF CORPORATIONS



### Detail by Entity Name

#### Florida Profit Corporation

EDA ENGINEERS-SURVEYORS-PLANNERS, INC.

#### Filing Information

Document Number	520946
FEI/EIN Number	591714370
Date Filed	12/27/1976
State	FL
Status	ACTIVE
Last Event	NAME CHANGE AMENDMENT
Event Date Filed	04/17/2014
Event Effective Date	NONE

#### Principal Address

2404 NW 43RD ST  
GAINESVILLE, FL 32606-6602

Changed: 05/31/1995

#### Mailing Address

2404 NW 43RD ST  
GAINESVILLE, FL 32606-6602

Changed: 05/31/1995

#### Registered Agent Name & Address

REYES, SERGIO J  
2404 NW 43RD ST  
GAINESVILLE, FL 32606-6602

Name Changed: 09/28/2012

Address Changed: 01/21/2000

#### Officer/Director Detail



## Name & Address

Title PT

REYES, SERGIO J  
2404 NW 43RD ST.  
GAINESVILLE, FL 32606

[SupraSavings](#)

GRAVER, ROBERT W  
2404 NW 43RD ST  
GAINESVILLE, FL 32606

Title S

SWEGER, CLAY B  
2404 NW 43RD ST  
GAINESVILLE, FL 32606

## Annual Reports

<u>Report Year</u>	<u>Filed Date</u>
2013	01/28/2013
2014	04/01/2014
2015	03/12/2015

## Document Images

<a href="#">03/12/2015 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/17/2014 -- Name Change</a>	View image in PDF format
<a href="#">04/01/2014 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">01/28/2013 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">12/03/2012 -- Amendment</a>	View image in PDF format
<a href="#">09/28/2012 -- Reg. Agent Change</a>	View image in PDF format
<a href="#">01/03/2012 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/08/2011 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/20/2010 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/06/2009 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/08/2008 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">01/23/2007 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/12/2006 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">04/13/2005 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">03/31/2004 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">10/27/2003 -- Reg. Agent Change</a>	View image in PDF format
<a href="#">03/31/2003 -- ANNUAL REPORT</a>	View image in PDF format
<a href="#">02/21/2002 -- ANNUAL REPORT</a>	View image in PDF format

<a href="#">02/01/2001 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>
<a href="#">01/21/2000 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>
<a href="#">02/20/1999 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>
<a href="#">01/29/1998 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>
<a href="#">04/09/1997 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>
<a href="#">04/23/1996 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>
<a href="#">05/31/1995 -- ANNUAL REPORT</a>	<a href="#">View image in PDF format</a>



# Authorized Agent Affidavit

## A. PROPERTY INFORMATION

Address of Subject Property: 10908 Rachael Blvd  
Parcel ID Number(s): 03962-000-000 and 03203-000-000  
Acreage: 8.04

## B. PERSON PROVIDING AGENT AUTHORIZATION

Name: John R. Gassie Title: Senior Vice President  
Company (if applicable): PNC Bank, NA successor by merger to RBC Bank (USA)  
Mailing Address: 201 E. Pine St. Suite 100  
City: Orlando State: FL ZIP: 32801  
Telephone: \_\_\_\_\_ FAX: \_\_\_\_\_ e-mail: \_\_\_\_\_

## C. AUTHORIZED AGENT

Name: Sergio Reyes, P.E. Title: President  
Company (if applicable): eda engineers - surveyors planners, inc.  
Mailing address: 2404 NW 43rd Street  
City: Gainesville State: FL ZIP: 32606  
Telephone: 373-3541 FAX: 373-7249 e-mail: sreyes@edafl.com

## D. REQUESTED ACTION:

Site plan review  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

I hereby certify that I am the property owner of record, or I have received authorization from the property owner of record to file an application for a development permit related to the property identified above. I authorize the agent listed above to act on my behalf for purposes of this application.

John R. Gassie  
Signature of Applicant

\_\_\_\_\_  
Signature of Co-applicant

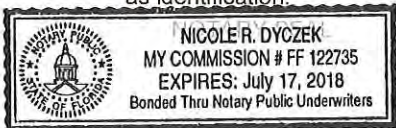
John R. Gassie, SVP of PNC Bank, NA  
Typed or printed name and title of applicant

\_\_\_\_\_  
Typed or printed name of co-applicant

State of Florida County of Orange

The foregoing application is acknowledged before me this 7<sup>th</sup> day of April, 2015 by John R.

Gassie, who is/are personally known to me, or who has/have produced \_\_\_\_\_  
as identification.



Nicole R. Dyczek  
Signature of Notary Public, State of Florida

Nicole R. Dyczek

City of Alachua ♦ Planning and Community Development Department  
PO Box 9 ♦ Alachua, FL 32616 ♦ (386) 418-6121  
Revised 9/30/2014





## CERTIFICATE

The undersigned, Ruby Altizer, a duly appointed Assistant Secretary of PNC Bank, National Association (the "Bank"), does hereby certify that:

- (1) the following is a true and correct copy of an excerpt from the By-Laws of the Bank and a true and correct copy of Resolutions adopted by the Board of Directors of the Bank on April 23, 2013;
- (2) the excerpt from the By-Laws of the Bank and Resolutions described above are in full force and effect as of the date of this Certificate; and
- (3) John R. Gassie is a duly appointed Senior Vice President of the Bank.

*Excerpt from By-Laws of PNC Bank, National Association*

"Article VI. General Powers of Officers

Section 1. The corporate seal of the Bank may be imprinted or affixed by any process. The Secretary and any other officers authorized by resolution of the Board of Directors shall have authority to affix and attest the corporate seal of the Bank.

Section 2. The authority of officers and employees of this Bank to execute documents and instruments on its behalf in cases not specifically provided for in these By-Laws shall be as determined from time to time by the Board of Directors, or, in the case of employees, by officers in accordance with authority given them by the Board of Directors."

*Board Resolutions Adopted April 23, 2013*

NOW, THEREFORE, BE IT RESOLVED, that the Chief Executive Officer, the President, each Senior Vice Chairman, each Vice Chairman, each Executive Vice President, each Senior Vice President, each Vice President, each Assistant Vice President, the Treasurer and each Assistant Treasurer, the Cashier and each Assistant Cashier, the Secretary and each Assistant Secretary, each Trust Officer and Assistant Trust Officer, each Chief Investment Officer, each Regional President or chief executive of a business region, the General Counsel, each Deputy General Counsel and each Chief Counsel (the "Authorizing Officers") of PNC Bank, National Association (the "Bank") shall have the authority to affix and attest the seal of the Bank;

RESOLVED FURTHER, that the Authorizing Officers of the Bank, and any other officers acting at the discretion of any officer authorized to affix and attest the seal of the Bank, are and each of them is hereby authorized and empowered in the name and on behalf of the Bank to execute, acknowledge and deliver any and all agreements, instruments, or other documents relating to the property or rights of all kinds held or owned by the Bank or to the operation of the Bank, either for its own account or in any agency or fiduciary capacity. Notwithstanding the foregoing, any and all agreements of sale, contracts, deeds and other documentation pertaining to the purchase, sale or transfer of real estate or buildings occupied by the Bank in the transaction of its business shall be executed in accordance with the terms of resolutions adopted from time to time in connection therewith and specifically designating the officer or officers authorized to execute the same;

The PNC Financial Services Group

One PNC Plaza 249 Fifth Avenue Pittsburgh Pennsylvania 15222-2707

M:\Pittsburgh\CF\CERT\04-23-13 Signature Authority Templates and Certificates\GASSIE, JOHN R.doc

RESOLVED FURTHER, that the Bank's Chief Executive Officer, President, Secretary, or any Senior Vice Chairman, Vice Chairman, or Executive Vice President or any of them, is authorized to name, constitute and appoint such person or persons as they or any of them deem necessary as attorney-in-fact for the Bank, to execute documents for and in its name and stead, and to perform all other acts, deeds and things as may be required to effect the particular transactions for which the appointment is made;

RESOLVED FURTHER, that the Bank's Chief Executive Officer, President, Secretary, or any Senior Vice Chairman, Vice Chairman, or Executive Vice President or any of them, is authorized to name, constitute and appoint such person or persons employed by the Corporation or any of its wholly owned direct or indirect subsidiaries as they or any of them deem necessary as attorney-in-fact for the Bank, to execute documents for and in its name and stead, and to perform all other acts, deeds and things as may be required to effect the particular transactions for which the appointment is made;

RESOLVED FURTHER, that any officer of the Bank and any non-officer employee of the Corporation or the Bank (or any affiliate of the Corporation or Bank) designated in writing by the Chief Executive Officer, the President, any Senior Vice Chairman, Vice Chairman, Executive Vice President or Senior Vice President of the Corporation or Bank, are each hereby authorized and empowered:

- (a) To sign or countersign checks, drafts, acceptances, guarantees of signatures on assignments of securities, certificates of securities of entities for whom the Bank is acting as registrar or transfer agent or in a fiduciary or representative capacity, correspondence or other papers or documents not ordinarily requiring execution under seal; and
- (b) To receive any sums of money or property due or owing to the Bank in its own right, as an agent for another party, or in any fiduciary or representative capacity and, either as attorney-in-fact for the Bank or otherwise, to sign or countersign agreements, instruments, or other documents related to the foreclosure of residential real estate loans owned or serviced by the Corporation or the Bank or the enforcement of any other rights and remedies with respect to such loans (including, without limitation, in a bankruptcy or insolvency proceeding), including, without limitation, correspondence, affidavits, certifications, declarations, deeds, substitutions of trustee, verifications, assignments, powers of attorney, sales contracts or any other papers or documents, to execute any instrument of satisfaction for any mortgage, deed of trust, judgment or lien in the Office of the Recorder of Deeds, Prothonotary, or other office or court of record in any jurisdiction, provided, however, that in respect to any mortgage or deed of trust made to this Bank as trustee for bondholders, the foregoing authority shall be exercised only pursuant to an authorization of the Board of Directors or committee of the Board of Directors with oversight of fiduciary risk.

#### General

RESOLVED FURTHER, that the Authorized Officers of the Bank, and each of them, are authorized to do any and all things and to take any and all actions in connection with these resolutions, including, but not limited to, the execution, delivery, acknowledgement, submitting, filing, recording and sealing of all documents, certificates, statements or other instruments, and the making of any expenditures, which such officers may deem necessary or advisable in order to carry out the intent and purposes of these resolutions; and

RESOLVED FURTHER, that all actions heretofore taken by any of the officers, representatives or agents of the Bank, by or on behalf of the Bank or any of its affiliates in connection with the foregoing resolutions be, and each of the same is, ratified and approved; and



RESOLVED FURTHER, that for purposes of the foregoing resolutions, the term "Authorized Officer" shall mean and include, as applicable, the Chairman, Chief Executive Officer, President, Senior Vice Chairman, Chief Financial Officer, Secretary or Treasurer of the Bank, or any Vice Chairman, Executive Vice President, Senior Vice President, Vice President, Assistant Secretary or Assistant Treasurer of the Bank or any other duly appointed officer of the Bank.

IN WITNESS WHEREOF, the undersigned has hereunto set her hand and affixed the seal of the Association this 19<sup>th</sup> day of July, 2013.

  
\_\_\_\_\_  
Ruby Altizer



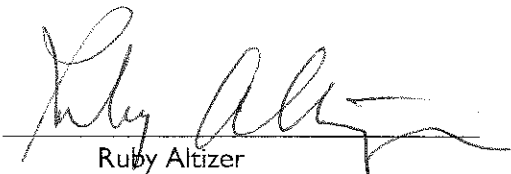


## CERTIFICATE

The undersigned, Ruby Altizer, Assistant Secretary of PNC Bank, National Association, does hereby certify as follows:

1. Effective as of March 2, 2012 and pursuant to approval granted by the United States Office of the Comptroller of the Currency (as evidenced by the official certification dated March 2, 2012 attached hereto as Exhibit "A"), RBC Bank (USA) was merged with and into PNC Bank, National Association.
2. PNC Bank, National Association is a duly organized and existing national banking association (Charter Number 1316) and wholly owned subsidiary of PNC Bancorp, Inc. (a wholly owned subsidiary of The PNC Financial Services Group, Inc.), having its main office located at 222 Delaware Avenue, Wilmington, Delaware 19801 and using federal Employer Identification Number 22-1146430.

IN WITNESS WHEREOF, the undersigned has hereunto set her hand and affixed the seal of this Association this 7<sup>th</sup> day of November, 2013.

  
Ruby Altizer

The PNC Financial Services Group

One PNC Plaza 249 Fifth Avenue Pittsburgh Pennsylvania 15222-2707

M:\Pittsburgh\CF\CERTS\BY ENTITY\ROYAL BANK OF CANADA\RBC (USA).docx



EXHIBIT A



Comptroller of the Currency  
Administrator of National Banks

Northeastern District Office  
340 Madison Avenue, 5<sup>th</sup> Floor  
New York, New York 10173

Licensing Division  
Telephone No.: 212.790.4055  
Fax No.: 301.333.7015

March 2, 2012

James S. Keller  
Chief Regulatory Counsel  
The PNC Financial Services Group, Inc.  
249 Fifth Avenue  
One PNC Plaza, 21<sup>st</sup> Floor  
Pittsburgh, Pennsylvania 15222-2707

Re: Application to merge RBC Bank (USA), Raleigh, North Carolina, with and into PNC Bank,  
National Association, Wilmington, Delaware under the charter and title of the latter  
Control No: 2011 NE 02 0021 Charter No: 1316

Dear Mr. Keller:

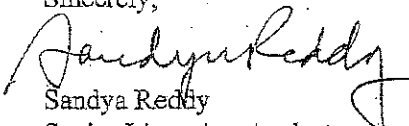
This letter is the official certification of the approval of the Comptroller of the Currency ("OCC") to merge RBC Bank (USA), Raleigh, North Carolina with and into PNC Bank, National Association, Wilmington, effective March 2, 2012 at 11:59 pm. The resulting bank title is PNC Bank, National Association, Charter No. 1316.

This is also the official authorization given to PNC Bank, National Association, to operate the branches and the main office of RBC Bank (USA) as branches. A listing of each newly authorized branch and its assigned OCC branch number is attached.

Please notify this office of any increase in capital stock or surplus as a result of this transaction so that we may certify the increase in permanent capital.

If you have any questions, please contact me at (212) 790-4055.

Sincerely,

  
Sandya Reddy  
Senior Licensing Analyst



engineers • surveyors • planners, inc.

## **PePeDogs Site Plan Application – Required Attachments**

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- 1. Site Plan**
  - Site Plan attached in submittal
- 2. Stormwater Management Plan**
  - Stormwater Management Plan included in Submittal
- 3. Fire Department Access and Water Supply**
  - Fire Flow Calculations are included in site plan sheets.
- 4. Concurrency Impact Analysis**
  - Report included in submittal
- 5. Analysis of Consistency with the City of Alachua Comprehensive Plan**
  - Included in submittal
- 6. Labels (1 set) of all property owners within 400 feet of the subject property boundaries**
  - Included in submittal
- 7. Neighborhood Meeting Materials**
  - Included in submittal
- 8. Legal Description with Tax Parcel Number**
  - Included in submittal
- 9. Proof of Ownership**
  - Included in submittal
- 10. Proof of Payment of Taxes**
  - Included in submittal



**11. Environmental Resource Permit**

- A permit application will be submitted to SRWMD for review. The approved permit will be forwarded to the City of Alachua.

**12. County Access**

- The project does not access a county road and therefore, a permit is not required for this project.

**13. FDOT Access Management Permit**

- The project does not access a state road and therefore, a permit is not required for this project.

**14. Fee**

- Included in submittal



engineers • surveyors • planners, inc.

## Worksheet for Needed Fire Flow

Project Name : PePeDogs  
Customer / Builder Name : Jose Peruyero  
Property Address : 10908 Rachael Blvd  
Alachua, FL

### NFPA Calculation

Fire Flow Area (SF)	=	6,600
Type of Construction	=	Type II
Required Needed Fire Flow Per NFPA	=	1,500 gpm
Per 18.4.5.2.1, 75% reduction for Sprinkler System	=	375 gpm
(Resulting Fire Flow shall not be less than 1,000 gpm)		

<b>REQUIRED Needed Fire Flow</b>	<b>1,000</b>	<b>gpm</b>
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## Concurrency Impact Analysis

### Pepedogs

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This proposed facility includes a 6,720 square foot kennel and dog training facility.

**Stormwater:** Stormwater management areas have been designed in accordance with the applicable criteria outlined by City of Alachua and the Suwannee River Water Management District. In addition, a permit shall be submitted to the Suwannee River Water Management District for the proposed improvements to the site and shall be processed concurrently with the proposed Site Plan. A condition of Site Plan approval shall include the provision of the SRWMD permit.

**Potable Water:**

*Goal 4: Provide an adequate supply of high quality potable water to customers throughout the service area.*

*Objective 4.1 Achieve and maintain acceptable levels of service for potable water quality and quantity.*

**Project Impact:** For the proposed commercial development, it is estimated that approximately 15 G.P.D. will be used per 100 square feet of building area (Ch. 64E-6, F.A.C.). The 6,720 square foot building will generate approximately 1,008 G.P.D., based on this calculation ( $6,720 \text{ SF} / 100 \text{ SF} \times 15 \text{ G.P.D.} = 1,008 \text{ G.P.D.}$ ). As shown in the following table, there is adequate capacity available to support this development.

Potable Water Impacts	
System Category	Gallons Per Day
Current Permitted Capacity	2,300,000
Less Actual Potable Water Flows	1,140,000
Reserved Capacity	108,775
Project Demand	1,008
<b>Residual Capacity</b>	<b>1,050,217</b>
<b>Percentage of Permitted Design Capacity Utilized</b>	<b>54.3%</b>

### **Sanitary Sewer:**

*Goal 1: Plan for and provide adequate, high quality and economical wastewater service while protecting the environment, especially groundwater resources.*

*Objective 1.2 Wastewater service will be made available to new development in a manner to promote compact urban growth, promoting development where wastewater service is available, and discouraging urban sprawl.*

*Policy 1.2.a: The City shall establish a Community Wastewater Service Area, which includes all areas where wastewater service is available.*

*Project Impact:* For the proposed commercial development, it is estimated that approximately 15 G.P.D. will be used per 100 square feet of building area (Ch. 64E-6, F.A.C.). The 6,720 square foot building will generate approximately 1,008 G.P.D., based on this calculation ( $6,720 \text{ SF} / 100 \text{ SF} \times 15 \text{ G.P.D.} = 1,008 \text{ G.P.D.}$ ). The proposed development will be served by a septic system. The proposed development is not located within the City of Alachua's Community Wastewater Service Area as stated by Marcus Collins, Public Services Director, in a letter dated January 28, 2015. There shall be no Sanitary Sewer Impacts.

### **Solid Waste:\***

*Goal 2: The City of Alachua will provide for solid waste disposal service in a sanitary, economic, and environmentally safe manner.*

*Project Impact:* Commercial uses generate approximately 12 pounds per day of solid waste per 1,000 square feet (Environmental Engineering: A Design Approach, Cincero and Cincero, 1996). The proposed facility will generate approximately 81 pounds of solid waste will be generated per day ( $6,720 \text{ SF} / 1,000 \text{ SF} \times 12 = 81 \text{ pounds per day}$ ).

<b>Solid Waste Impacts</b>		
<b>System Category</b>	<b>Lbs Per Day</b>	<b>Tons Per Year</b>
Existing Demand <sup>1</sup>	37,200.00	6,789.00
Reserved Capacity <sup>2</sup>	5,284.50	964.42
<b>New River Solid Waste Facility Capacity<sup>3</sup></b>	<b>50 years</b>	

**Traffic:** The proposed use of the project site as a commercial use will not create a traffic impact that will exceed the approved level of service standards for the impacted roadways (US Highway 441 and CR 2054), as indicated below.

### **Trip Generation**

The Institute of Transportation Engineers, 9<sup>th</sup> Edition Trip Generation Manual, does not specify a generator for this type of proposed activity. Therefore, the trip generation proposed is based on the New Jersey Department of Transportation HAPS program, dated March 1, 2013, that provides trip generation data for a "Dog Boarding Business" use type.

<b>Trip Generation</b>								
Land Use Description	Units of Measure	Number of Employees	Week Day Daily Trips Multiplier	Week Day Daily Trips	AM Peak Hour Multiplier	AM Peak Hour	PM Peak Hour Multiplier	PM Peak Hour
Dog Boarding Business	Employees	9	6	54	1.5	13.5	1.5	13.5

New Jersey Department of Transportation HAPS Program, March 1, 2013

**Roadway Level of Services (LOS) Analysis (For Affected Segments)**

Segment ID:	Segment Limits:	LOS C/D	MSV	AADT	Res'vd	Project	Available
US Hwy 441	SR 235 – NW 126 Ave	D	35,500	17,495	1,331	83	16,591

Segment ID:	Segment Limits:	LOS C/D	MSV	AADT	Res'vd	Project	Available
CR 2054 East	East of SR 235	D	14,580	2,042	418	83	12,037

**Trip Distribution**

Segment	% Distribution of ADT (PePeDogs)	Trips (ADT)
US Hwy 441 (CR 235 – NW 126 <sup>th</sup> Ave)	75%	62
CR 2054 East (East of SR 235)	25%	21





## Statement of Proposed Uses

The proposed Site Plan includes a proposed 6,720 square foot building to serve as a kennel and dog training facility to be located on an approximately 11 acre property located at the northeastern corner of the intersection of US Highway 441 and CR 2054 (tax parcels 03962-000-000 and 03203-000-000).

## Comprehensive Plan Consistency

### **Future Land Use Element**

*Goal 1: Future Land Use Map: The City of Alachua shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens.*

*Objective 1.3      Commercial: The City of Alachua shall establish three commercial districts: Community Commercial, Commercial and Central Business District. These districts shall provide a broad range of retail sales and services, as well as office uses, in order to provide for the availability of goods and services, both to the citizens of Alachua and to the citizens of the North Central Florida region.*

**Consistency:**      **The proposed commercial use is consistent with the types of uses contemplated by the Commercial FLU category. The use is of a size and scale to appropriately located on the project site, abutting a state and county road.**

*Policy 1.3.b      Commercial: The Commercial land use category is established to provide for general commercial uses, as well as more intense commercial and highway commercial uses. This is the land use category in which large-scale, regional commercial uses could locate. The following uses are allowed within the Commercial land use category:*

1. Retail sales and services;
2. Personal services;
3. Financial Institutions;
4. Outdoor recreation and entertainment;
5. Tourist-related uses;
6. Hotels, motels;
7. Commercial shopping centers;
8. Auto-oriented uses;
9. Traditional Mixed-use Neighborhood Planned Developments;
10. Employment Center Planned Developments;
11. Commercial recreation centers;
12. Office/business parks;
13. Limited industrial services;
14. Eating Establishments

**Consistency:**        **The proposed kennel and dog training facility is a personal service, which is listed as a permitted use above.**

*Policy 1.3.d        Design and performance standards*

*The following criteria shall apply when evaluating commercial development proposals:*

1.     *Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;*

**Consistency:**        **Site access is limited to one appropriately designed access to CR 2054, which will receive a County driveway connection permit as part of the project approval.**

2.     *Buffering from adjacent existing/potential uses;*

**Consistency:**        **The proposed Landscape Plan provides the required buffering outlined in the City LDR, including the buffering requirements found in Sec. 6.2.2. All required landscape plantings, buffers and tree replacement standards are proposed as part of this project. Please note that the attached Landscape Plan makes specific reference to the applicable landscape planting, buffers and tree replacement requirements and how the project meets each applicable criterion.**

3.     *Open space provisions and balance of proportion between gross floor area and site size;*

**Consistency:**        **The proposed Site Plan provides the required open space outlined in the City LDR. Specifically, the project includes open space that far**

exceeds the 10% minimum area as required in Sec. 6.7.3(A) of the LDR.

4. *Adequacy of pervious surface area in terms of drainage requirements;*

**Consistency:** The proposed Site Plan provides the appropriate pervious surface area to accommodate the site. This is demonstrated through the provision of an approved SRWMD permit, which shall be issued to the project.

5. *Placement of signage;*

**Consistency:** Proposed signage shall be permitted by the City and will be designed in compliance with the City LDR. Specifically, proposed signage will be permitted as a building permit and will be designed according to the standards outlined in Sec. 6.5.

6. *Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;*

**Consistency:** The proposed Site Plan provides a lighting plan that is designed to comply with the requirements outlined in the City LDR Sec. 6.4.

7. *Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;*

**Consistency:** The proposed Site Plan has been designed to provide safe site access. The proposed driveway shall access a county road (CR 2054), which accesses US 441 from an existing median opening.

8. *Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;*

**Consistency:** The proposed Landscape Plan provides the required landscaping and buffering as outlined in the City LDR, including the buffering requirements found in Sec. 6.2.2. All required landscape plantings, buffers and tree replacement requirements are proposed as part of this project. Please note that the attached Landscape Plan makes specific reference to the applicable landscape planting, buffer and tree replacement requirements and how the project meets each applicable criterion.

9. *Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and*



**Consistency:** There are no unique features or resources to the site that constrain site development.

10. *Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.*

**Consistency:** The project is in compliance with the applicable performance based criteria found in the LDR. The following indicates how the project complies with the criteria for a kennel:

4.3.3(C)(4) Kennel / Outdoor

*Kennel/outdoor. An outdoor kennel shall comply with the following standards:*

- (a) *Buildings and open runs. Not locate open runs or buildings used for housing of animals within 125 feet of any lot line.*

**Consistency:** The Site Plan limits buildings and open runs to areas outside of the required 125 foot setback.

- (b) *Adjacent to single-family detached development or in residential district.*
- (i) *If adjacent to single-family residential districts (RSF-1, RSF-3, RSF-4 and RSF-6), not exceed two stories or 130 percent of the maximum height allowed for single-family detached dwellings in the district.*

**Consistency:** N/A.

- (ii) *If adjacent to existing single-family detached development, not allow the height of buildings to exceed two stories or 180 percent of the average height of the adjacent single-family development.*

**Consistency:** The proposed building is one story in height.

- (c) *Landscaped buffer adjacent to single-family detached development. If adjacent to existing single-family detached development, provide a landscaped buffer, a minimum of 15 feet in width along the yard which the single-family detached development abuts.*

**Consistency:** The required buffer is provided along the common lot line abutting the existing single family residence.

- (d) *Accessory uses. Accessory uses to an outdoor kennel may include retail sales, veterinary service, and grooming services, as long as the accessory uses do not include more than 25 percent of the total gross floor area.*

**Consistency:** So noted. Any accessory uses shall comply with the criteria listed above.

11. Commercial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.

**Consistency:** The proposed commercial FAR (shown on cover sheet), is below the maximum FAR and therefore is consistent with this policy.

### **Traffic Circulation Element**

*The GOAL 1: Provide for a traffic analysis submitted circulation system, which serves existing and future land uses.*

*Objective 1.1: Level of Service The City shall establish a safe, convenient and efficient level of service standard for all motorized and non-motorized transportation systems.*

**Consistency:** As demonstrated in the Concurrency Impact Analysis illustrates section of this report, the developments compliance impacts associated with this element the proposed project will comply with the adopted level of service standards by the City of Alachua.

### **Community Facilities and Natural Groundwater Aquifer Recharge Element**

*This development shall connect to an on-site septic system and centralized potable water system provided by the City of Alachua. Policy 1.2.a: The City shall establish a Community Wastewater Service Area, which includes all areas where wastewater service is available. Wastewater service shall be deemed available if:*

*3. A gravity wastewater system, wastewater pumping station, or force main exists within ¼ mile of the property line of any residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and the gravity wastewater system, wastewater pumping station, or force main can be accessed through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.*

**Consistency:** As confirmed by the City Public Services Director, the project site is not located within the Community Wastewater Service area as it is located over ¼ mile from a gravity wastewater system, wastewater pumping station or force main. Therefore, an on-site septic system is proposed to serve the facility.

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GOAL 2: Solid Waste The City of Alachua will provide for solid waste disposal service in a sanitary, economic, and environmentally safe manner.

Objective 2.1: Continue to ensure satisfactory and economical solid waste service for all City residents, with an emphasis on reuse and recycling.

Policy 2.1.a: The City hereby establishes the following level of service standards for solid waste disposal facilities:

<u>FACILITY TYPE</u>	<u>LEVEL OF SERVICE STANDARD</u>
<u>Solid Waste Landfill</u>	<u>.73 tons per capita per year</u>

**Consistency:** Solid waste shall be disposed of in an on-site dumpster serviced by a private or public refuse service. As demonstrated in the Concurrency Impact Analysis section of this report, the solid waste impacts associated with the proposed project will comply with the adopted level of service standards by the City of Alachua.

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GOAL 3: Stormwater Develop and maintain a stormwater management system that minimizes flooding, protects, preserves and enhances desirable water quality conditions, and, where possible, preserves and utilizes existing natural features.

Objective 3.1: Ensure provision of drainage and stormwater retention through level of service standards and design requirements to minimize flooding and to protect and improve water quality.

**Consistency:** The site shall retain all storm water on-site with a system designed to meet the required design standards. The as indicated in the Comp Plan and Land Development Code. In addition, the site shall implement strict erosion control measures, as specified in the plans, during all construction activities.

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GOAL 4: Potable water Provide an adequate supply of high quality potable water to customers throughout the water service area.

Objective 4.1: Achieve and maintain acceptable levels of service for potable water quantity and quality.

**Consistency:** This development shall connect to the centralized potable water system provided by the City of Alachua. As demonstrated in the Concurrency Impact Analysis section of this report, the potable water impacts associated with the proposed project will comply with the adopted level of service standards by the City of Alachua.



**Conservation and Open Space Element**

*GOAL 1: To conserve, protect, manage and restore the natural and environmental resources of the City by emphasizing stewardship and understanding that environmental issues transcend political and geographical boundaries.*

**Consistency:** The site is designed to preserve a large number of native trees and undergrowth vegetation along the perimeter of the property, thus meeting the intent of this Goal.

*OBJECTIVE 1.11: Open and Green Space The City shall work to preserve native ecosystems and the natural aesthetic beauty and charm of Alachua by ensuring the provision of open spaces and green linkages throughout the City, designed for the enjoyment of the citizenry.*

*Policy 1.11.a: The City shall consider offering incentives to developers to include open green spaces beyond the minimum amount required in new developments.*

**Consistency:** The open space provided by the development substantially exceeds the amount required. The landscaping materials proposed are approved native species and meet or exceed the code requirements as well.

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## Memorandum

To: Brandon DATE: 3/5/2015  
FROM: Brenna French  
SUBJECT: Neighborhood Meeting, PepeDogs Facility

### PUBLIC NOTICE

A neighborhood workshop will be held to discuss a site plan for a proposed dog training facility and kennel on approximately 8.46 acres located at 10908 Rachael Blvd in Alachua (parcel numbers 03962-000-000 and 03203-000-000). This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the proposed development and to seek their comments.

The meeting will be held on Wednesday, March 25, 2015 at 6:00 p.m. in Meeting Room A at the Alachua Branch of the Alachua County Library, at 14913 NW 140<sup>th</sup> Street.



Contact: Clay Sweger, AICP, LEED AP  
eda engineers – surveyors – planners, inc.  
(352) 373-3541

## ACLU challenges use of prisoners in the drawing of political districts

The Associated Press

TALLAHASSEE — The American Civil Liberties Union of Florida is challenging a new Florida county use of prisoners in the drawing of political districts.

The civil rights group on Thursday filed a lawsuit in federal court in Tallahassee alleging that Jefferson County's 2013

districting plan amounted to "prison gerrymandering."

The lawsuit alleges that by counting the more than 1,100 inmates at the Jefferson Correctional Institution as residents for redistricting purposes, Jefferson County watered down the voting strength of other districts.

The lawsuit also says

that under the maps for County Commission and School Board, prisoners make up more than 60 percent of the population in the county's District 3, given that district an unfair advantage compared with the county's four other districts.

Convicted felons lose their voting rights in Florida.

## STATE

### Merlin in Orlando looking to hire 300

ORLANDO — Merlin Entertainment is looking for a few good men and women.

The entertainment company says that it's holding a job fair to find 300 workers for its new entertainment complex.

The complex includes a Madame Tussauds wax museum, an aquarium and the Orlando Eye, a 400-foot observation wheel.

The complex is located on International Drive in the heart of Orlando's tourist district.

It is scheduled to open in May.

### Ex-cop says bank trespassed on home

ESTERO — A retired police officer is suing a bank, saying its contractor

broke into his southwest Florida house and falsely told people he was in

there.

The News-Press of Fort Myers reports that former Chicago police officer Mike Tomaszewski filed a lawsuit against Fifth Third Bank, which holds his

mortgage.

According to the lawsuit, two men working for the bank's contractor broke into the house in November and posted a sign saying it was found to be unsecured or vacant. He says they falsely told neighbors he is in foreclosure.

Tomaszewski's lawyer says even if the house had been in foreclosure, the bank would have needed a warrant to enter.

The lawsuit accuses the bank of trespassing, invasion of privacy and defamation.

The bank declined comment.

### Sept. 11 agent dies from WTC exposure

ORLANDO — An ATF agent has died from cancer linked to his time working as a first responder to the collapsed World Trade Center after the Sept. 11, 2001, terrorist attacks.

ATF Special Agent William Sheldon was to be buried Monday in a suburb of Orlando.

Officials with the Bureau of Alcohol, Tobacco, Firearms and Explosives say Sheldon had been battling cancer for a year.

Sheldon was part of a five-man ATF team that responded to the World Trade Center site in New York in 2001.

Three members from the team developed cancer, which doctors linked to exposure to debris from the towers.

— Compiled from The Associated Press

## OBITUARIES

### Today's Services

### Funeral Notices

### Funeral Notices

**McPEAK** — Dorothy Charlotte Whitman, 82, of Ft. Pierce, died on March 9, 2015, at 11:00 a.m. at the Miami Funeral Home Chapel, 311 S. Main St., Gainesville, FL 32601 (352) 376-6361. The family will receive friends at 30 a.m.

**METZLER, ROBERT E.**, 87, St. Petersburg, died peacefully March 7, 2015, at Suncoast Hospice Care Center. He was a lifelong resident of St. Petersburg.

**BLAKEMAN, MARY F.**, 88, passed peacefully into heaven in the early morning of March 12 after a long and wonderful life.

### Death Notices

**BRINN, MARGIE O.**, 93, Keystone Heights, died on March 9, 2015, at 11:00 a.m. at the Miami Funeral Home Chapel, 311 S. Main St., Gainesville, FL 32601 (352) 376-6361. The family will receive friends at 30 a.m.

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**CAIN, ETTIE L.**, 92, Abcoha, died on March 9, 2015, at 11:00 a.m. at the Miami Funeral Home Chapel, 311 S. Main St., Gainesville, FL 32601 (352) 376-6361. The family will receive friends at 30 a.m.

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**GRAHAM JR., ANDREW, 62**, Miami, died on March 9, 2015, at 11:00 a.m. at the Miami Funeral Home Chapel, 311 S. Main St., Gainesville, FL 32601 (352) 376-6361. The family will receive friends at 30 a.m.

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**GRAHAM, MARGARET W.**, 83, Miami, died on March 9, 2015, at 11:00 a.m. at the Miami Funeral Home Chapel, 311 S. Main St., Gainesville, FL 32601 (352) 376-6361. The family will receive friends at 30 a.m.

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## Senators clash over exemptions from child-care licensing in Fla.

The News Service of Florida

TALLAHASSEE — A bill that would make clear national groups like the Boys and Girls Clubs are exempt from Florida's licensing requirements is stalled in a legislative committee, sparking a clash between two state senators.

Sen. Eleanor Sobel, chairwoman of the Senate Children, Families and Elder Affairs Committee, says in a letter to the bill (SB 250) by her fellow Broward County Democrat, Chris Smith, would be postponed "because we don't have enough information."

"As senators, we are to the children of this state, the state to look into who is licensed, who is not licensed and why," Sobel, D-Hollywood, said after the meeting.

Smith's bill would change current law governing child-care facilities to specify that national organizations with their own standards for local clubs — such as Big Brothers Big Sisters, the Boys and Girls Clubs and the Girl Scouts and Boy Scouts — would not be subject to licensure by the state.

The meeting last week was the second time Sobel's committee had postponed the bill. The first was on Feb. 19, and Smith said that since then, he'd met with committee members to discuss their concerns.

"I had the votes, but Sen. Sobel just can't comprehend or understand the law," Sen. Dan Claitor, D-Fort Lauderdale, said. "That seems to be a problem."

The bill is intended to clarify a situation decades in the making. Every child-care facility in Florida is required to have a license and renew it annually through the Department of Children and Families. In 1997, certain religious and national groups were

exempted from their requirements, but since then questions have arisen.

The Boys and Girls Clubs — generally known for after-school sports — have been exempt, along with similar clubs. Smith said that's because the clubs don't provide child care, which is considered in law to be for younger children, but after-school care for older children. Other officials, however, contend that since the exemption was passed, the Boys and Girls Clubs have been serving children as young as 3.

"There is some confusion, I think, in the current statute," said Department of Children and Families Secretary Mike Carroll. "From the department's standpoint, I would like clarification as to whether the current law within the state to exempt national organizations who do provide child care ... Or if we don't, then we have to be clear, because we're not in the middle of that right now."

Former Lt. Gov. Jeff Kotlikowski, who lobbies for the Boys and Girls Clubs, said the bill was prompted by the fact that "the agencies that we deal with don't accept one another's background screenings."

Kotlikowski pointed to the Boys and Girls Clubs' use of what's known as "level-two screening," which is a finger print-based, national criminal history search. "The Boys and Girls Clubs far exceed what anyone else is doing," he told the committee last month.

But Janet Mabry, a lobbyist who represents private child-care providers, spoke against the bill that day. She said that while the Boys and Girls Clubs provide background screening, their exemption from licensure means they aren't subject to the same health and safety standards of child-care facilities.

"The question is, as they (lawmakers) expand their definition to national organizations providing after-school care, what happens, and do we want people to be exempt from health and safety and minimum requirements?" Mabry asked. Smith and Kotlikowski contend the clubs also comply with other local standards, such as fire safety.

they've been doing that for years," Smith said. Mabry

Sobel, meanwhile, said the licensing information on how the national groups maintain health and safety standards at their local clubs.

"I worked with the Boys and Girls Clubs in Broward County, and I saw how ground-level support definitely supported their change to Level 2," she wrote in an email. "But when it came to licensing ... the national organization never voluntarily responded to Senate and House staff inquiries as of this email about national standards for health and safety. The Boys and Girls Clubs, I believe, rely on the national standards for their chapters (and) as one has seen a copy."

Sobel also pointed to a 2006 opinion on the matter by then-Attorney General Bob Butterworth. "Given the clear legislative intent to establish statewide minimum standards for the care and protection of children in child-care facilities, it is not infringing on religious rights or parental programs or regulating the activity programs of membership organizations that do not provide child care, in my opinion that programs operated by membership organizations that fall within the definition of 'child care program' are not exempt from licensure by the Department of Children and Families," Butterworth wrote.

Passions are running high. Sobel said she received 45 texts to her personal cell phone with the message, "As a resident of Broward County, we only you to support SB 250 which we understood you were supporting with the changes you requested up until today."

Smith said he had nothing to do with the texts and was sorry they had become part of the discussion.

He also said he's not sure whether he'll bring the bill back before Sobel's committee or try another legislative tactic.

"After 14 years in the Legislature, I know how to get things through," he said. "This is an important issue that affects hundreds of thousands of kids around the state, and we've got plenty of time to get it done."

Give the family a personal message of condolence by posting to our guest books online.

Gainesville.com/vobits

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**Obituary Information**

All obituaries are paid notices and are placed by the funeral home or interment facility handling the arrangements at a service to the family.

For more information, call 312-634-01 or 374-5017 or visit [obits@gainesville.com](http://obits@gainesville.com) or call (352) 338-3131

**PUBLIC NOTICE**

A neighborhood workshop will be held in person at a site plan for a proposed new housing facility and located at approximately 840 West Lake Street, Suite 200, in Gainesville, Florida. The purpose of this meeting is to inform neighborhood property owners of the proposed development and to take their comments.

The meeting will be held on Wednesday, March 23, 2016 at 8:00 a.m. in Meeting Room 2 of the Alachua County Library at 1418 NW 14th Street.

Contact: City Manager, ACP 1418 NW 14th Street, Gainesville, FL 32601 (352) 378-1541

Gainesville as you imagine it

[gainesville.com](http://gainesville.com)



03203-000-000  
RBC BANK  
C/O 945000197  
C/O 945000197  
SHAWNEE MISSION, KS 66225

03199-001-000  
INA M TRUSTEE SUMMERS  
PO BOX 2211  
ALACHUA, FL 32616-2211

03199-002-000  
CARL EUGENE SUMMERS  
11212 SAGE BLVD  
ALACHUA, FL 32615-5733

03200-000-000  
DANIELLE THOMAS  
16424 NE 2ND ST  
Gainesville, FL 32609

03202-002-001  
PATRICK E RHODES  
14303 NW 107TH TER  
ALACHUA, FL 32615

03202-010-004  
MARGARET M STACK  
11010 SAGE BLVD  
ALACHUA, FL 32615-5732

03202-010-008  
DAVID C DURKEE  
PO BOX 1543  
ALACHUA, FL 32616-1543

03953-000-000  
ALACHUA SELF STORAGE INC  
14024 NW US HWY 441  
ALACHUA, FL 32615

03953-001-001  
ALACHUA SELF STORAGE INC  
PO BOX 1857  
ALACHUA, FL 32616

03957-000-000  
LIABILITY LIMITED PARTNERSHIP  
MCCALL FAMILY LIMITED  
5045 WESTSHORE DR  
5045 WESTSHORE DR  
NEW PORT RICHEY, FL 34652

03958-000-000  
ALACHUA SELF STORAGE INC  
PO BOX 1857  
ALACHUA, FL 32616

03962-000-000  
RBC BANK  
C/O 945000197  
C/O 945000197  
SHAWNEE MISSION, KS 66225

03962-001-000  
HILLMAN SUPPLY COMPANY INC  
PO BOX 2339  
ALACHUA, FL 32616-2339

05848-000-000  
JOHN C HIPPI CONST EQUIP CO  
PO BOX 1000  
ALACHUA, FL 32616-1000

05949-004-000  
ALACHUA INC CALVARY BAPTIST  
CHURCH OF  
PO BOX 1227  
ALACHUA, FL 32616-1227

05971-000-000  
CHAMBERS & STEELE  
1225 NW FRONTIER DR  
LAKE CITY, FL 32055

92060-517-900  
CSX TRANSPORTATION INC  
500 WATER ST  
500 WATER ST  
JACKSONVILLE, FL 32202-4423



Antoinette Endelicato  
5562 NW 93<sup>rd</sup> Avenue  
Gainesville, FL 32653

Dan Rhine  
288 Turkey Creek  
Alachua, FL 32615

Bill Atwater  
6017 NW 115<sup>th</sup> Place  
Alachua, FL 32615

Tom Gorman  
9210 NW 59<sup>th</sup> Street  
Alachua, FL 32653

Richard Gorman  
5716 NW 93<sup>rd</sup> Avenue  
Alachua, FL 32653

Peggy Arnold  
410 Turkey Creek  
Alachua, FL 32615

David Forest  
23 Turkey Creek  
Alachua, FL 32615

John Tingle  
333 Turkey Creek  
Alachua, FL 32615

TCMOA  
President  
1000 Turkey Creek  
Alachua, FL 32615

Linda Dixon, AICP  
Assistant Director Planning  
PO Box 115050  
Gainesville, FL 32611

Craig Parenteau  
FL Dept of Environmental Protection  
4801 Camp Ranch Road  
Gainesville, FL 32641

Laura Williams  
12416 NW 148<sup>th</sup> Avenue  
Alachua, FL 32615

Jeanette Hinsdale  
PO Box 1156  
Alachua, FL 32616

Lynn Coullias  
7406 NW 126<sup>th</sup> Avenue  
Alachua, FL 32615

Lynda Coon  
7216 NW 126<sup>th</sup> Avenue  
Alachua, FL 32615

# Neighborhood Workshop Notice

Date: March 25, 2015  
Time: 6:00 p.m.  
Place: Alachua Library Branch, Meeting Room A  
14913 NW 140<sup>th</sup> Street, Alachua, FL 32615  
Contact: **eda** engineers – surveyors – planners, inc. at (352) 373-3541

A neighborhood workshop will be held to discuss a site plan for a proposed dog training facility and kennel on approximately 8.46 acres located at 10908 Rachael Blvd in Alachua (parcel numbers 03962-000-000 and 03203-000-000). This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the proposed project and to seek their comments.



## PePeDogs Neighborhood Meeting Sign-In Sheet

Wednesday, March 25, 2015; 6:00 pm

[illegible]





engineers • surveyors • planners, inc.

## Neighborhood Meeting Minutes

**Location:**

Alachua Library Branch  
14913 NW 140<sup>th</sup> Street  
Alachua, Florida 32615

**Meeting Date & Time:**

March 25, 2015 at 6:00 pm

**Community Participants:**

2

**Attendees:**

Margaret Stack      11010 Sage Rd.  
Glo Durkee      10826 Sage Rd.

**Project Representatives:**

**Agent:**

Clay Sweger (eda)

**Owner Representatives:**

Jose Peruyero, Jr. & Jose Peruyero, Sr.

**Meeting Minutes:**

Mr. Sweger gave a brief presentation regarding the proposed Site Plan, which will include a dog training facility and kennel. The two meeting attendees asked the following questions:

Q: What is the use?

A: A dog training facility and kennel. The business name is Pepedogs and they currently have a facility in High Springs. They have been serving the county for many years.

Q: Will the kennels be outdoors? Will there be outdoor dog runs?

A: No to both questions. The only outdoor area will be for training and walking the animals during select daytime hours.

Q: How close will the use be to our property?

A: Your property is located north of the 200-foot wide railroad right of way. The project is located south of the railroad area and will be recessed at least 125 feet from the northern property line (per City Code). In addition, we are keeping a vast majority of the vegetation in place in the 125 foot setback area.

Q: Will there be a fence?

A: Yes, there will be a perimeter fence along the property line and another fence around the outdoor training areas.



Q: Will where the access be to the site?

A: The proposed driveway will be along Rachael Blvd., not US 441.

Q: What will be the plan for waste pick-up?

A: The employees will pick up after the animals.

Q: How many kennel spaces?

A: There will be a maximum of 100 indoor spaces.

Q: We have a concern about dogs getting loose?

A: We have not had any problems at the existing facility and the two fence systems will ensure that no dogs will get loose.

Q: How will noise be mitigated?

A: Noise will be mitigated by using exclusively indoor kennels and limited outdoor activities to daytime hours. No dogs will be outdoors at night. In addition, the large setback and buffer area from all property lines will greatly reduce any impacts.

After the meeting attendees asked all their questions, they expressed appreciation for the information and wished the operators luck with their project and hoped that they will all be good neighbors.



engineers • surveyors • planners, inc.

#### **LEGAL DESCRIPTION (PARCELS 03203-000-000& 03962-000-000)**

---

**PARCEL 6:**

THAT PORTION OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER (NE  $\frac{1}{4}$  OF NE  $\frac{1}{4}$ ) OF SECTION 24, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, THAT LIES SOUTHERLY OF THE FORMER ATLANTIC COASTLINE RAILROAD RIGHT OF WAY AND NORTHERLY OF OLD STATE ROAD NO. 2 AND NORTHERLY OF STATE ROAD NO. 25 (U.S. HIGHWAY NO. 441); LESS THE FOLLOWING DESCRIBED LAND:

COMMENCE AT THE NORTHEAST CORNER OF SAID SECTION 24 FOR A POINT OF REFERENCE, THENCE RUN SOUTH 00 DEG. 52 MIN. 29 SEC. EAST ALONG THE EAST LINE OF SAID SECTION 24, A DISTANCE OF 26.60 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF THE FORMER SEABOARD COASTLINE RAILROAD FOR THE POINT OF BEGINNING; FROM THE SAID POINT OF BEGINNING CONTINUE SOUTH 00 DEG. 52 MIN. 29 SEC. EAST ALONG THE SAID EAST LINE OF SECTION 24, A DISTANCE OF 649.06 FEET TO AN IRON PIPE ON THE NORTHERLY RIGHT OF WAY LINE OF U.S. HIGHWAY NO. 441; THENCE RUN NORTH 61 DEG. 54 MIN. 59 SEC. WEST ALONG THE SAID NORTHERLY RIGHT OF WAY LINE, A DISTANCE OF 228.63 FEET TO AN IRON PIPE; THENCE RUN NORTH 00 DEG. 52 MIN. 29 SEC. WEST PARALLEL TO AND 200.0 FEET PERPENDICULAR TO THE SAID EAST LINE OF SECTION 24, A DISTANCE OF 571.08 FEET TO AN INTERSECTION WITH THE SAID SOUTHERLY RIGHT OF WAY LINE OF SAID SEABOARD COASTLINE RAILROAD; THENCE RUN SOUTH 81 DEG. 35 MIN. 15 SEC. EAST, ALONG THE SAID SOUTHERLY RIGHT OF WAY LINE, A DISTANCE OF 202.70 FEET TO THE SAID IRON PIPE ON THE EAST LINE OF SECTION 24 AND THE POINT OF BEGINNING. CONTAINING 2.80 ACRES, MORE OR LESS.

**PARCEL 7:**

A TRACT OF LAND SITUATED IN THE SE  $\frac{1}{4}$  OF SE  $\frac{1}{4}$  OF SECTION 13, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA. SAID TRACT OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SOUTHEAST CORNER OF THE AFOREMENTIONED SECTION 13, TOWNSHIP 8 SOUTH, RANGE 18 EAST, FOR A POINT OF REFERENCE; THENCE RUN NORTH 89 DEG. 06 MIN. 22 SEC. WEST ALONG THE SOUTH LINE OF SAID SECTION 13, A DISTANCE OF 509.72 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE NORTH 89 DEG. 06 MIN. 22 SEC. WEST ALONG SAID SOUTH LINE OF SECTION 13 A DISTANCE OF 648.75 FEET TO THE NORTHEASTERLY RIGHT-OF-WAY LINE OF STATE ROAD NO. S-340A (66 FOOT RIGHT-OF-WAY); THENCE RUN NORTH 8 DEG. 00 MIN. 00 SEC. EAST A DISTANCE OF 125.33 FEET TO THE SOUTH LINE OF SEABOARD COAST LINE RAILROAD; THENCE RUN SOUTHEASTERLY ALONG SAID SOUTH LINE TO THE POINT OF BEGINNING.

IN THE CIRCUIT COURT OF THE  
EIGHTH JUDICIAL CIRCUIT IN AND  
FOR ALACHUA COUNTY, FLORIDA

CIVIL DIVISION

CASE NO.: 2010 CA 007119

Division: MG

RBC BANK (USA),

Plaintiff,

VS.

JUSTALEX, INC., a Florida corporation,  
JOHN M. PLA, ALEXANDRA K. PLA,  
and GARLAND PLA,

Defendants.

RECORDED IN OFFICIAL RECORDS  
INSTRUMENT # 2650970 2 PGS  
June 16, 2011 09:06:22 AM  
Book 4038 Page 604  
J K IRBY Clerk of Circuit Court  
ALACHUA COUNTY, Florida

Doc Stamp-Deed: \$0.70



CERTIFICATE OF TITLE

The undersigned Clerk of the Court certifies that he/she executed and filed a Certificate of Sale in this action on June 3, 2011, for the property described herein, and that no objections to the sale have been filed within the time allowed for filing objections.

The following property in Alachua County, Florida:

See attached Exhibit A

was sold to RBC Bank (USA), whose address is 4221 W. Boy Scout Blvd., Suite 100, Tampa, Florida 33607.

Witness my hand and seal of this Court on June 14, 2011

*J.K. Irby*  
Clerk of the Circuit Court  
Alachua County



cc:

Leslie Joughin III, Esquire  
Jose I. Moreno, Esquire

Case: 2010 CA 007119



00026009858  
DK CE10-R

FILED  
CK 46

2011 JUN 14 AM 9:37

J.K. IRBY  
CLERK OF COURTS  
ALACHUA COUNTY, FL

By: *[Signature]*

(Seal)

2



**EXHIBIT A****LEGAL DESCRIPTION****PARCEL 6:**

THAT PORTION OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER (NE 1/4 OF NE 1/4) OF SECTION 24, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, THAT LIES SOUTHERLY OF THE FORMER ATLANTIC COASTLINE RAILROAD RIGHT OF WAY AND NORTHERLY OF OLD STATE ROAD NO. 2 AND NORTHERLY OF STATE ROAD NO. 25 (U. S. HIGHWAY NO. 441); LESS THE FOLLOWING DESCRIBED LAND:

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## 2014 Roll Details — Real Estate Account #03203 000 000

Real Estate Account #03203 000 000

[Parcel details](#)

[Latest bill](#)

[Full bill history](#)

**2014**

Paid

**2013**

Paid

**2012**

Paid

**2011**

Paid

...

**2002**

Paid

[Get Bills by Email](#)

Owner: RBC BANK  
4221 W BOY SCOUT BLVD STE 100  
TAMPA, FL 33607  
Situs: (unknown)

Account number: **03203 000 000**

Alternate Key: 1012210

Millage code: 1700

Millage rate: 24.8319

Assessed value: 3,500

School assessed value: 3,500

*Location is not guaranteed to be accurate.*

[Property  
Appraiser](#)

- GIS

### 2014 annual bill

Ad valorem:  
Non-ad valorem:  
Total Discountable:  
No Discount NAVA:  
Total tax:

[View](#)

**\$86.92**  
**\$0.00**  
**86.92**  
**0.00**

### Legal description

COM SE COR SEC RUN W 509.72 FT TO  
POB CONT W 648.75 FT TO NELY R/W SR-  
S 340-A N 8 DEG E 125.33 FT TO S R/W  
OF SCL RR SELY ALONG SAID S R/W TO  
POB OR 4038/0604

### Location



Book, page, item: --  
Geo number: 13-08-18-  
03203000000  
Range: 18  
Township: 08  
Section: 13

**Paid 2014-11-25 \$83.44**  
**Receipt #14-0030749**



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## 2014 Roll Details — Real Estate Account At 10908 RACHAEL BLVD, ALACHUA 32615

Real Estate Account #03962 000 000

[Parcel details](#)

[Latest bill](#)

[Full bill history](#)

**2014**

Paid

**2013**

Paid

**2012**

Paid

**2011**

Paid

...

**2002**

Paid

[Get Bills by Email](#)

Owner: RBC BANK  
4221 WBOY SCOUT BLVD STE 100  
TAMPA, FL 33607  
Situs: 10908 RACHAEL BLVD  
ALACHUA 32615

Account number: **03962 000 000**

Alternate Key: 1014414

Millage code: 1700

Millage rate: 24.8319

Assessed value: 33,100

School assessed value: 33,100

*Location is not guaranteed to be accurate.*

[Property  
Appraiser](#)

- GIS

### 2014 annual bill

Ad valorem:	<b>\$821.94</b>
Non-ad valorem:	<b>\$37.90</b>
Total Discountable:	<b>859.84</b>
No Discount NAVA:	<b>0.00</b>
Total tax:	

**Paid 2014-11-26 \$825.45**  
**Receipt #14-0033584**

[View](#)

### Legal description

THAT PART OF NE1/4 OF NE1/4 S OF ACL  
RR & N OF PUB RD (LESS OR 797/272)  
OR 4038/0604

### Location

Book, page, item: --  
Geo number: 24-08-18-  
03962000000  
Range: 18  
Township: 08  
Section: 24





## City of Alachua

TRACI L. CAIN  
CITY MANAGER

PUBLIC SERVICES DEPARTMENT  
MARCUS COLLINS - PUBLIC SERVICES DIRECTOR

January 28<sup>th</sup>, 2015

PNC Bank, NA successor by merger to RBC Bank  
201 E. Pine Street, Ste. 100  
Orlando, FL 32801

Dear Sir/Madam:

This letter is to inform you that regarding your inquiry for connection to the City of Alachua's wastewater gravity system on parcel #03962-000-000.

In reference to the City of Alachua Comprehensive Plan Policy 1.2.a. (3)

The City shall establish a Community Wastewater Service area, which includes all areas where wastewater service is available. Wastewater service shall be deemed available if:

**3. A gravity wastewater system, wastewater pumping station, or force main exists within ¼ mile of the property line of any residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and the gravity wastewater system, wastewater pumping station, or force main can be accessed through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.**

Since the review of your request to connect to the parcel is greater than 1,320 ft for commercial development, you are not required to connect.

If you have any questions, please do not hesitate to contact the Public Services Department at 386-418-6140.

Sincerely,

Marcus Collins  
Public Services Director

Cc: Amber Roberts-Crawford  
BROKER/ OWNER



engineers • surveyors • planners, inc.

## DRAINAGE DESIGN NOTES

PePeDogs

10908 Rachael Blvd  
City of Alachua, Florida

March 31, 2015





engineers • surveyors • planners, inc.

## DRAINAGE DESIGN NOTES

PePeDogs

10908 Rachael Blvd  
City of Alachua, Florida

March 31, 2015

Professional Engineer of Record:	
Date:	
<u>Christopher A. Gmuer, PE</u> Engineer	<u>71599</u> Cert. No.

## Table of Contents

Drainage Design Notes

Attachment A                      Geotechnical Report by Universal Engineering Sciences

Attachment B                      Pre-Development and Post-Development Drainage Maps

Attachment C                      Pre-Development Time of Concentration WinTR-55 Model  
Pre-Development and Post-Development Conditions  
PONDS Model

Attachment D                      Aerial Photograph

Attachment E                      Soils Map

Attachment F                      Quad Map

Attachment G                      FEMA Map

## DRAINAGE DESIGN NOTES

I. **PROJECT NAME** PePeDogs  
Alachua, FL

II. **PROJECT LOCATION**  
City: Alachua  
County: Alachua  
Sections: 13 & 24 Township: 8S Range: 18E  
General Location: 10908 RACHAEL BLVD  
Alachua, FL  
Watershed: Santa Fe River (Sub-DA: Cellon Creek)

III. **GENERAL PROJECT INFORMATION**

1. The proposed development consists of the construction of a 6,720-SF building with associated parking, utilities, one(1) treatment swale along the proposed driveway and one(1) stormwater management facility.

IV. **DRAINAGE AND DESIGN CRITERIA**

1. **Stormwater Management Facility**  
Suwannee River Water  
Management District (SRWMD) Meet requirements of 40C-42.  
City of Alachua
2. **Treatment Swale**  
Suwannee River Water Management District (SRWMD)  
5.5.1: Dry System  
5.5.2: Treat Specified WQTV or 80% of the runoff resulting from a 3YR-1HR design storm, SC  
SCS Type II Distribution  
1:3 Side slopes max

V. **SITE SOILS INFORMATION**

Universal Engineering Sciences conducted a subsurface investigation on the site and summarized their findings in the report No. 1205846, dated March 6, 2015. A copy of the report is provided in Attachment A.

VI. **EXISTING SITE CONDITIONS**

The existing conditions of the site is vacant. There is an open field and an area of trees. Up until approximately 2007, the site was a farm.

**VII. DRAINAGE DESCRIPTION**

**1. Pre-development Conditions**

In the Pre-Development condition, the project site elevation is approximately 91 in the northwestern corner. Stormwater flows southeasterly across the site to collect at a low spot at elevation 75. Offsite areas discharge to the project site.

**2. Post-development Conditions**

- a) The proposed stormwater conveyance system consists of one(1) stormwater management system and one(1) swale system that will provide water quality treatment , recovery, and attenuation as required by the Suwannee River Water Management District and the City of Alachua, Florida. The treatment swale is designed to treat the water quality treatment volume for the proposed roadway.
- b) Pre and Post-development conditions have been met as required by the Suwannee River Water Management District and the City of Alachua, Florida, refer to the post drainage exhibit.



## VIII. DRAINAGE DESIGN

### 1/ DRAINAGE AREAS

#### PRE-Development

PRE-DEVELOPMENT DA-1			
	Area (sf)	Area (Acres)	Curve CN
Impervious Area	0	0.000	98.0
Open Area (Fair, 'A') - Fenced Area	0	0.000	49.0
Open Area (Good, 'A')	49,642	1.140	39.0
SMF	0	0.000	100.0
<b>TOTALS</b>	<b>49,642</b>	<b>1.140</b>	<b>39</b>

#### POST-Development

POST-DEVELOPMENT DA-Swale			
	Area (sf)	Area (Acres)	Curve CN
Impervious - Driveway	4,956	0.114	98.0
Impervious - Building	0	0.000	98.0
Impervious - Parking Lot	0	0.000	98.0
Open Area (Fair, 'A') - Fenced Area	0	0.000	49.0
Open Area (Good, 'A')	0	0.000	39.0
Swale	1,179	0.027	100.0
<b>TOTALS</b>	<b>6,134</b>	<b>0.141</b>	<b>98</b>

POST-DEVELOPMENT DA-1			
	Area (sf)	Area (Acres)	Curve CN
Impervious Area	26,460	0.607	98.0
Open Area (Fair, 'A') - Fenced Area	3,960	0.091	49.0
Open Area (Good, 'A')	6,156	0.141	39.0
SMF	13,065	0.300	100.0
<b>TOTALS</b>	<b>49,642</b>	<b>1.140</b>	<b>87</b>

## 2/ BASIN STORAGE DATA

Swale				
Stage (EL)	Area (SF)	Area (ac)	Volume (CF)	Volume (AC-FT)
86.50	565	0.013	0	0.000
87.50	1,179	0.027	872	0.020

SMF-1				
Stage (EL)	Area (SF)	Area (ac)	Volume (CF)	Volume (AC-FT)
78.25	6,859	0.157	0	0.000
79.00	7,987	0.183	5,567	0.128
80.00	9,579	0.220	14,350	0.329
81.00	11,272	0.259	24,776	0.569
82.00	13,065	0.300	36,944	0.848

## 3/ WATER QUALITY TREATMENT VOLUME

The SMFs provide water quality treatment volume per SRWMD and COA criteria for dry retention basins. This criteria includes two thresholds, whichever of the two is greater:

$$\text{Volume V1} = 1.00 \text{ inches over the total area}$$

DESCRIPTION	WQTV (CF)	Stage (FT)
Swale	511	87.09
SMF-1	4,137	78.82

## 4/ BASIN GEOMETRY & DETAILS

DESCRIPTION	SMF-1	Swale
Perimeter (FT)	461	209
Volume (CF)	36,944	872
Height (FT)	3.75	1.00
Effective basin width (ft)	57	9
Effective basin length (ft)	174	96
Maximum unsaturated area	11,272	1,179

## 5/ SUBSURFACE INVESTIGATION INFORMATION

Based on the Soils Report No. 1205846, dated March 6, 2015 prepared by Universal Engineering Sciences, the recommendations of the soil characteristics are summarized below:

	SMF-1	Swale
Soil Boring	B-1/B-2/B-3/B-4	B-1/B-2/B-3/B-4
Average Ground El.	79.00	87.00
Depth Confined layer (ft)	4.50	4.50
Confined layer El.	74.50	82.50
Depth of SHWT (ft)	4.00	4.00
SHWT EL.	75.00	83.00
Vertical Infiltration Rate (ft/d)	9.00	9.00
Safety factor	2.00	2.00
Vertical Infiltration rate (ft/d)	4.50	4.50
Horizontal Infiltration Rate(ft/d)	18.00	18.00
Safety factor	2.00	2.00
Horizontal Infiltration rate(ft/d)	9.00	9.00
Fillable porosity (%)	25.00	25.00

## 6/ STORM ROUTING RESULTS

The computer program POND3 3.3 was used to route the storms through the proposed retention system. The design storm used for design was 100-yr critical storms and the mean annual, and 25YR-96 HR storm events. The input data and storm routing results can be seen in the Attachment C.

SMF-1					
Storm	Stage	PRE	POST	PRE	POST
Event	(ft)	Rate (cfs)	Rate (cfs)	Volume (CF)	Volume (CF)
100YR-01HR	79.66	0.22	0.00	397	0
100YR-02HR	79.89	0.27	0.00	1,196	0
100YR-04HR	80.21	0.57	0.00	2,784	0
100YR-08HR	80.24	0.62	0.00	4,803	0
100YR-24HR	80.47	0.32	0.19	11,032	10,478
100YR-72HR	80.69	0.44	0.28	17,965	16,902
100YR-168HR	80.80	0.37	0.31	24,119	20,397
100YR-240HR	80.84	0.47	0.31	30,087	19,164
WQTV	78.82	-	-	-	-

## 7/ FREEBOARD

SRWMD 5.1.2: 1-ft of freeboard required

Storm	SMF-1
Event	Freeboard (ft)
100YR-01HR	2.34
100YR-02HR	2.11
100YR-04HR	1.79
100YR-08HR	1.76
100YR-24HR	1.53
100YR-72HR	1.31
100YR-168HR	1.20
100YR-240HR	1.16
Treatment	3.18

## 8/ RECOVERY

### Water Quality Treatment Volume

The criteria for the recovery of the system is the recovery of the required water quality volume within 72 hours following the storm event.

DESCRIPTION	Swale	SMF-1
Treatm. Vol	511	4,137
Time (hrs)	24.00	24.00

### Storm Events

Storage volumes designed into retention or detention systems must be available as follows:

1. One-half of the total volume within seven days following the end of the design storm event, and
2. The total volume within 30 days following the end of the design storm event.

Storm	Recovery	Recovery
Event	1/2 Volume (day)	Full Volume (day)
100YR-01HR	↓ 1 day	1 day
100YR-02HR	↓ 1 day	2 days
100YR-04HR	1 day	6 days
100YR-08HR	1 day	10 days
100YR-24HR	↓ 1 day	30 days
100YR-72HR	↓ 1 day	30 days
100YR-168HR	5 days	30 days
100YR-240HR	↓ 1 day	30 days

## 9/ MAINTENANCE

Drainage maintenance is described in the Drainage Details and Notes sheet. Maintenance will consist of mowing, weeding, pruning, and tree trimming that will be and estimated \$3,000 per year.

## Attachment A

Geotechnical Report by Universal Engineering Sciences





# UNIVERSAL ENGINEERING SCIENCES

Consultants in: Geotechnical Engineering • Environmental Engineering  
Construction Materials Testing • Threshold Inspection • Private Provider Inspection

March 6, 2015

Scentworx  
P.O. Box 769  
High Springs, FL 32655

Attention: Mr. Jose Peruyero

Reference: **Report of Geotechnical Consulting Services**  
Scentworx - Stormwater Management System  
10908 Rachael Boulevard (Tax Parcel: 03962-0)  
Alachua, Alachua County, Florida  
UES Project No. 0230.1500024.0000

UES Report No. 1205846

LOCATIONS:  
Atlanta R 18 15  
Daytona Beach P  
Fort Myers  
Fort Pierce  
Gainesville  
Jacksonville  
Kissimmee  
Leesburg  
Miami  
Ocala  
Orlando (Headquarters)  
Palm Coast  
Panama City  
Pensacola  
Rockledge  
Sarasota  
Tampa  
West Palm Beach

Dear Mr. Peruyero:

Universal Engineering Sciences, Inc. (UES) has completed geotechnical engineering services for the stormwater management area at the subject project in Alachua, Alachua County, Florida, as authorized in Proposal 1204428, dated February 25, 2015. This Report presents the results of our subsurface field exploration, laboratory soil testing programs, and recommendations for the proposed stormwater management system.

## Objectives

The objectives of our geotechnical consulting services on this portion of the project have been summarized as follows:

- Explore the subsurface conditions within the proposed stormwater management area,
- Perform a series of laboratory tests on selected subsurface soil specimens to assist with engineering soil classifications and to establish the relevant soil composition and permeability characteristics,
- Classify and stratify the various soil strata encountered in the soil test borings,
- Evaluate the groundwater level in the area of exploration and make appropriate recommendations,
- Recommend appropriate aquifer design parameter values for design of the on-site stormwater management system.

## Project Information

The subject parcel is located at 10908 Rachael Boulevard in Alachua, Alachua County, Florida. Current site development plans include construction of a new stormwater management facility. Our office was provided a copy of the overall site which shows the layout of the site, as well as the locations of the borings.



By contract, our exploration was confined to the zone of soil likely to be stressed by the proposed construction. Our work did not address the potential for surface expression of deep geological conditions, such as sinkholes. This evaluation requires a more extensive range of field services than performed in this study. We will be pleased to conduct an exploration to evaluate the probable effect of the regional geology upon the proposed construction, if you desire.

### **Site Conditions**

UES personnel visited the project parcel during the performance of the field portion of this geotechnical study. Our on-site observations have been summarized as follows. At the time of our exploration, the project parcel was undeveloped and partially wooded. Surface organic soils, surface debris, were not observed on the project site.

### **Local Geology**

The general geology of central Alachua County is characterized by a surface veneer of Pleistocene and Pliocene sands and sandy clays overlying the Miocene age Hawthorn Group, a highly variable mixture of interbedded quartz sands, clays, carbonates, pebbles and grains occurring in thickness of up to 150 feet. Underlying the Hawthorn Group is the upper Eocene age Ocala Formation, occurring as a uniform limestone, which is approximately 200 feet thick and overlies the Eocene age Avon Park Formation, which can be up to 500 feet thick. Both the Ocala and Hawthorn Formations dip to the northeast by approximately one degree.

The general hydrogeology of Alachua County consists of three aquifer systems: a surficial aquifer, and intermediate aquifer, and the Floridan aquifer system. The surficial aquifer exists as an unconfined water table situated over the impermeable Hawthorn Group and is usually a subdued reflection of surface topography. The intermediate aquifer system includes all rocks that collectively retard the exchange of water between the overlying surficial aquifer system and the underlying Floridan aquifer system. Water in this system is contained under confined conditions. The Floridan aquifer system is a thick carbonate sequence that functions regionally as a water-yielding hydraulic unit. The direction of shallow groundwater flow is generally toward surface water bodies. The surface of the upper Floridan Aquifer in the general project site area is estimated in the elevation range of +30 to +40 feet NGVD.

### **General Area Soils Information**

The United States Department of Agriculture (USDA) *Soil Survey of Alachua County, Florida* describes the near-surface soil profile in the project parcel as Arredondo soil. This soil is nearly level to gently sloping and well drained. The seasonal high water table is at a depth of more than 72 inches of the surface. Engineering properties for Arredondo soil are summarized below in Table 1.

<b>Table 1 – Relevant Engineering Index Properties of Arredondo soil (3B)</b>						
<b>Depth, Inches</b>	<b>Texture</b>	<b>Classification</b>	<b>% Passing #200 Sieve</b>	<b>Plasticity Index</b>	<b>Shrink-swell Potential</b>	<b>Permeability</b>
0-49	Fine sand	SP-SM, SM	5-15	NP	Low	6.0-20 in/hr
49-54	Loamy sand, loamy fine sand, sandy loam	SM, SM-SC	13-25	NP-7	Low	2.0-6.0 in/hr
54-86	Sandy loam, fine sandy loam, sandy clay loam	SM-SC, SC	20-40	NP-20	Low	0.2-2.0 in/hr



## **Subsurface Exploration**

The field geotechnical testing activities were started and completed on March 3, 2015. Field tests for this portion of the geotechnical study included four (4) soil test borings to a depth of 15 feet, performed at the locations shown on the attached Boring Location Plan. The actual test locations shown were approximate, and were staked in the field by UES personnel using existing landmarks and site features. The boreholes were backfilled to grade upon field work completion.

**Standard Penetration Test (SPT) Borings:** Penetration tests were performed in accordance with ASTM Procedure D-1586, *Penetration Test and Split-Barrel Sampling of Soils*. This test procedure generally involves driving a 1.4-inch I.D. split-tube sampler into the soil profile in six inch increments for a minimum distance of 18 inches using a 140-pound hammer free-falling 30 inches. The total number of blows required to drive the sampler the second and third 6-inch increments is designated as the N-value, and provides an indication of in-place soil strength, density and consistency.

The results of the classification and stratification have been shown on the attached Boring Logs. It should be noted that soil conditions might vary between soil test boring locations, and between the subsurface soil strata interfaces which have been shown on the Boring Logs. The soil test boring data reflect information from the specific test locations only. This report presents an evaluation of site conditions on the basis of traditional geotechnical procedures for site characterization. The recovered samples were not examined, either visually or analytically, for chemical composition or environmental hazards.

## **Subsurface Findings**

The field exploration performed for this project disclosed subsurface conditions that are consistent with the local geology and general area soils information described above. The subsurface conditions found in the soil test borings have been summarized in the attached Boring Logs and described below.

Generally, soil test borings encountered sand with silt [SP/SM] to depths of 3.5 to 5.5 feet, followed by clayey sand to sandy clay [SC/CH] to the maximum boring termination depths of 15 feet below ground surface.

The groundwater level was not generally encountered below existing site grades at the boring locations, at the time of our exploration. However, soil test boring B-3 did encounter a perched groundwater table of 5.5 feet below ground surface. It should be noted that the groundwater level may not have been fully stabilized in the boreholes when the readings were taken upon boring work completion. Fluctuations of groundwater level conditions on this project parcel should be expected to occur seasonally as a result of rainfall, surface runoff, and nearby construction activities.

## **Laboratory Soil Tests**

The soil samples recovered from the field exploration program were placed in containers and returned to our soils laboratory, where the Geotechnical Engineer visually classified the samples. Laboratory soil tests are performed to aid in the classification of the soils, and to help in the evaluation of engineering characteristics of the soils. Representative soil samples were selected for percent fines determination and permeability tests. The test results have been presented on the attached Boring Logs and summarized in Table 2.



Table 2 - Laboratory Soil Test Results				
Test Location	Sample Depth	Type of Test	Results	Soil Description
B-1	4 feet	% Finer #200	8 %	Sand with silt
		Moisture Content	6 %	
		Permeability	14 feet/day	
B-2	4 feet	% Finer #200	9 %	Sand with silt
		Moisture Content	6 %	
		Permeability	13 feet/day	
B-4	2.5 feet	% Finer #200	7 %	Sand with silt
		Moisture Content	7 %	
		Permeability	15 feet/day	

**Percent Passing No. 200 Sieve:** Certain recovered soil samples were selected to determine the percentage of fines. In these tests the soil samples were dried and washed over a No. 200 mesh sieve. The percent of soil by weight passing the sieve was the percentage of fines or portion of the sample in the silt and clay size range. This test was conducted in accordance with ASTM Procedure D-1140, *Amount of Material in Soils Finer Than the #200 Sieve*.

**Permeability:** Representative soil samples were selected to determine the permeability rate of the soil. Constant head permeability tests were performed on remolded representative samples of the near surface soils from the proposed stormwater management area. These tests were conducted following the concepts outlined in ASTM D-2434, *Standard Test Method for Permeability of Granular Soils (Constant Head and Falling Head)*.

**Moisture Content:** Certain recovered soil samples were selected to determine their moisture content. The moisture content is the ratio expressed as a percentage of the weight of water in a given mass of soil to the weight of the solid particles. This test was conducted in accordance with ASTM Procedure D-2216, *Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock*.

### **Stormwater Management System**

The laboratory test data indicates that the sandy soils within the existing and proposed stormwater management areas for this project generally have permeability rates of 13 to 15 feet per day at the boring locations. Based upon the above findings, we recommend that you consider the soil parameters presented in Table 3 for design of the stormwater management system on the subject project site. It should be noted that the above referenced values are measured values and do not incorporate factor of safety.



<b>Table 3.-Stormwater Management System Soil Design Parameters</b>	
Corresponding Soil Boring Test Locations	B-1/B-2/B-3/B-4
Average Depth to Confining/Restrictive Layer, feet	4.5
Average Unsaturated Vertical Infiltration rate, feet per day	9
Average Saturated Vertical Infiltration Rate, feet per day	14
Average Estimated Horizontal Hydraulic Conductivity, feet per day	18
Estimated Fillable Porosity, percentage	25
Estimated Depth of Seasonal High Water Table feet	4

### **Stormwater Management System Fill Suitability**

The recovered soil samples were classified using visual and textural means, and limited laboratory testing. We offer the following ***preliminary guidelines*** for the use of on-site soils, such as those excavated from the proposed shallow retention areas, as fill material for the project.

Soil materials excavated and classified as fine sands to sand with silts and sand with clay (SP, SP-SM, SP-SC), with typically 12% fines or less (silt/clay fraction), may be considered suitable for use as utility trench backfill, as well as building pad and pavement subgrade structural fill, provided said materials are properly dried, placed, and compacted.

Soil materials excavated and classified as silty fine sands [SM], with typically 12% to 25% fines, may also be considered suitable for use as utility trench backfill, as well as building pad and pavement subgrade structural fill, after significant drying and some mixing with the fine sand material described above. Proper placement, proof rolling and compaction must also be performed.

Soil materials excavated and classified as clayey sand, silt or clay (SC, ML, MH, CL, and CH) and any organic-laden soils (5% or greater organics by weight) should not be reused as fill beneath buildings or pavement sections. These materials could be used in green areas, if applicable and in non-structural applications where excessive ground subsidence will not create functional or aesthetic problems. It should be noted that silt and clay materials will retain water and if used may become saturated and soft for a significant period of time following a rain event.

Soil borings for a typical geotechnical report are widely spaced and generally not sufficient for reliably detecting the presence of isolated, anomalous surface or subsurface conditions, or reliably estimating unsuitable or suitable material quantities. Accordingly, UES does not recommend relying on our boring information to negate presence of anomalous materials or for estimation of material quantities unless our contracted services ***specifically*** include sufficient exploration for such purpose(s) and within the report we so state that the level of exploration provided should be sufficient to detect such anomalous conditions or estimate such quantities. Therefore, UES will not be responsible for any extrapolation or use of our data by others beyond the purpose(s) for which it is applicable or intended.



### **Report Limitations**

This Report has been prepared for the exclusive use of Scentworx, and members of the Design/Construction Team for the specific project discussed in this Report. This Report has been prepared in accordance with generally accepted local geotechnical engineering practices; no other warranty is expressed or implied. If any changes in the design or location of the project elements as outlined in this Report are planned, the conclusions and recommendations contained in this Report shall not be considered valid unless the changes are reviewed and the conclusions modified or approved, in writing, by UES.

UES performs hydraulic conductivity tests, including the two most common, i.e., DRI and remolded laboratory permeability testing, using generally accepted practices of the local engineering community. These common tests are the quickest and most economical for stormwater management system design. However, the user of this information is cautioned that the potential variability of results and reproducibility associated with these types of tests can be significant. It is important to note that there are many factors influencing the permeability of a soil. These factors include, but are not limited to, soil grain size, soil particle arrangement and structure, dispersion of soil fines, density, and degree of saturation, soil heterogeneity, and soil anisotropy. Also, the permeability measured by such tests may not be representative of that of the total effective aquifer thickness.

Factors of safety can compensate for part of the inherent test limitations but the Designer must exercise judgment regarding final selection and applicability of provided soil design input parameters. Should the modeling analysis indicate marginally acceptable compliance with Water Management District design criteria, it may be advisable to perform more extensive and representative in-situ permeability testing by collecting "undisturbed" horizontal and vertical soil samples and/or installing grouted piezometers or wells for slug testing. UES can perform these field tests if desired.

Additionally, the actual exfiltration rates from the pond may be influenced by pond geometry, natural soil variability, in-situ depositional characteristics and soil density, retention volume, and groundwater mounding effects. Also, it is important to note that the upper in-situ soil zone is usually altered during the excavation and grading operations by heavy, vibrating earthwork equipment. Due to these numerous factors cited above, published literature suggests that the permeability of a soil can only be estimated to within an order of magnitude. Therefore, appropriate factors of safety should be incorporated into the design process.

**Closure**

We have enjoyed being a part of the engineering team on this project, and appreciate the opportunity to have assisted you towards its successful completion. Please contact our office if you have any questions or need further assistance.

Respectfully submitted,  
UNIVERSAL ENGINEERING SCIENCES, INC.  
Certificate of Authorization Number 549



Timothy Kwiatkowski, EI  
Staff Geotechnical Engineer



Eduardo Suarez, P.E.  
Senior Geotechnical Engineer  
Florida P.E. No. 60272  
Date: 3.13.15

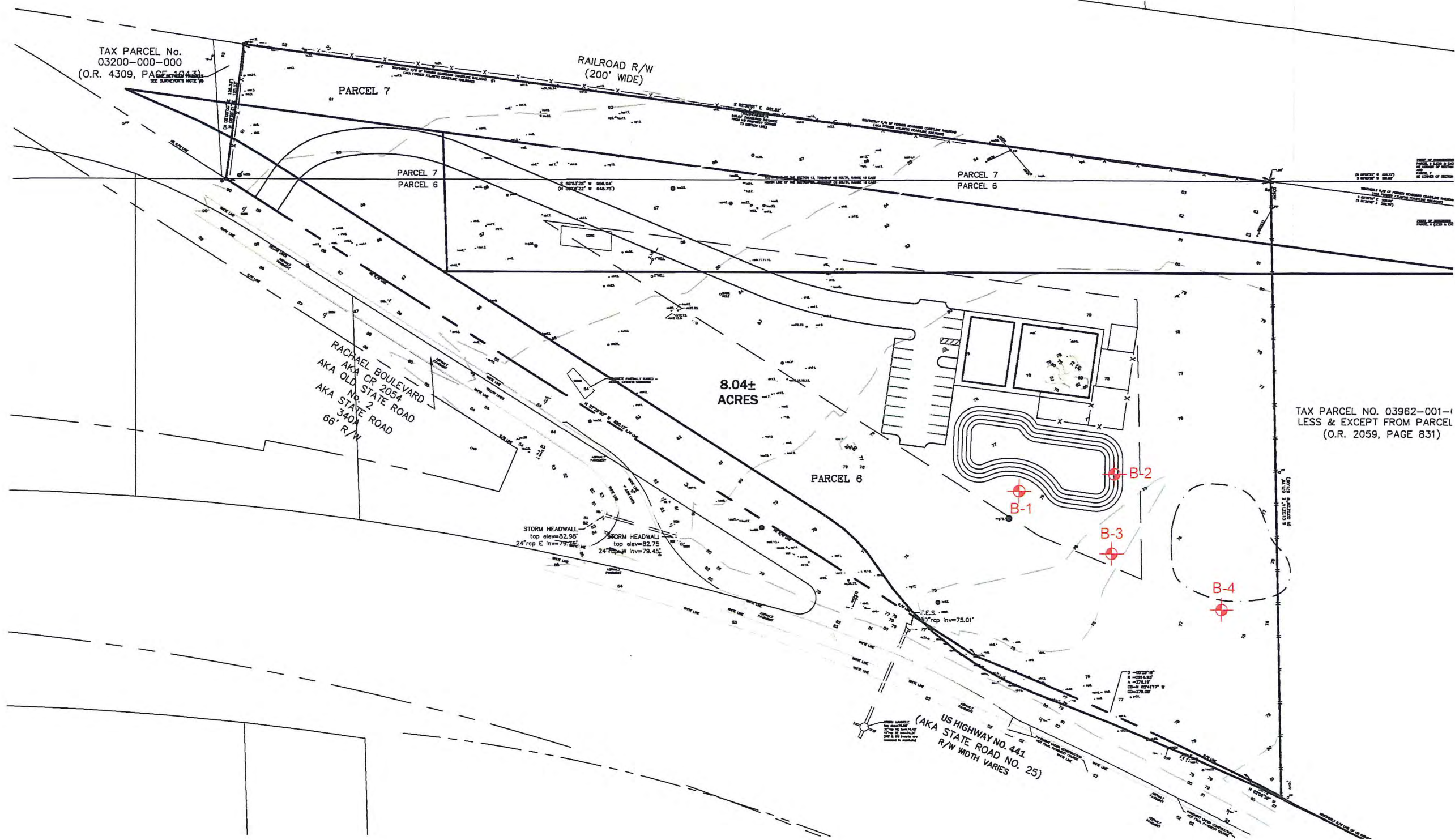


ES/JSP: es (1)

Attachments: Boring Location Plan, Boring Logs, Key to Boring Log

cc: EDA Engineer-Surveyors Planers, Inc





LEGEND

STANDARD PENETRATION TEST BORING LOCATION

NOTE: ALL SOIL TEST BORING LOCATIONS SHOWN ARE APPROXIMATE.



CLIENT: JOSE PERUYERO	
DRAWN BY: KD	DATE: 3/13/15
CHECKED BY: ES	DATE: 3/13/15
SCALE: 1"=100'	ACADFILE: 0230.1500024-A
PROJECT NO: 0230.1500024.0000	REPORT NO: 1205846

SCENTWORX  
10908 RACHEAL BOULEVARD  
ALACHUA, ALACHUA COUNTY, FLORIDA  
BORING LOCATION PLAN







# UNIVERSAL ENGINEERING SCIENCES BORING LOG

PROJECT NO.: 0230.1500024.0000

REPORT NO.: 1205846

PAGE: A-2

PROJECT: SCENTWORX SMA  
10908 RACHAEL BOULEVARD  
ALACHUA, ALACHUA COUNTY, FLORIDA

BORING NO: **B-1** SHEET: **1 of 1**

SECTION: 24 TOWNSHIP: 8 S RANGE: 18 E

CLIENT: JOSE PERUYERO  
LOCATION: SEE BORING LOCATION PLAN

GS ELEVATION(ft): DATE STARTED: 3/3/15  
WATER TABLE (ft): NE DATE FINISHED: 3/3/15  
DATE OF READING: NA DRILLED BY: R. WOODARD

REMARKS:

EST. WSWT (ft): TYPE OF SAMPLING: ASTM D-1586

DEPTH (FT.)	S A M P L E	BLOWS PER 6" INCREMENT	N VALUE	W.T.	S Y M B O L	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS		K (FT./ DAY)	ORG. CONT. (%)
									LL	PI		
0						Very loose light brown to tan SAND, with silt [SP-SM]						
1												
2		1-1-2	3									
3												
4		1-1-2	3				8	6			14	
5		2-2-3	5			Loose to medium dense gray and orange clayey SAND, with lenses of clay [SC]						
6												
7		4-4-6	10									
8		7-7-10	17									
9												
10		6-7-9	16									
11												
12												
13												
14												
15		3-3-5	8			Boring Terminated at 15'						



# UNIVERSAL ENGINEERING SCIENCES BORING LOG

PROJECT NO.: 0230.1500024.0000

REPORT NO.: 1205846

PAGE: A-3

PROJECT: SCENTWORX SMA  
10908 RACHAEL BOULEVARD  
ALACHUA, ALACHUA COUNTY, FLORIDA

BORING NO: **B-2** SHEET: **1 of 1**

CLIENT: JOSE PERUYERO  
LOCATION: SEE BORING LOCATION PLAN  
REMARKS:

SECTION: 24 TOWNSHIP: 8 S RANGE: 18 E  
GS ELEVATION(ft): DATE STARTED: 3/3/15  
WATER TABLE (ft): NE DATE FINISHED: 3/3/15  
DATE OF READING: NA DRILLED BY: R. WOODARD  
EST. WSWT (ft): TYPE OF SAMPLING: ASTM D-1586

DEPTH (FT.)	S A M P L E	BLOWS PER 6" INCREMENT	N VALUE	W.T.	S Y M B O L	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS		K (FT./ DAY)	ORG. CONT. (%)
									LL	PI		
0						Very loose brown and tan SAND, with silt [SP-SM]						
1												
2		1-1-1/12"	1									
3												
4		1-2	3				9	6			13	
5		3-3-4	7			Loose to medium dense light gray and orange clayey SAND [SC]						
6												
7		6-6-7	13									
8		5-6-8	14									
9												
10		8-7-8	15									
11												
12												
13												
14												
15		4-4-5	9			Boring Terminated at 15'						



# UNIVERSAL ENGINEERING SCIENCES BORING LOG

PROJECT NO.: 0230.1500024.0000

REPORT NO.: 1205846

PAGE: A-4

PROJECT: SCENTWORX SMA  
10908 RACHAEL BOULEVARD  
ALACHUA, ALACHUA COUNTY, FLORIDA

BORING NO: **B-3** SHEET: **1 of 1**

CLIENT: JOSE PERUYERO  
LOCATION: SEE BORING LOCATION PLAN  
REMARKS: PERCHED WATER TABLE

SECTION: 24 TOWNSHIP: 8 S RANGE: 18 E  
GS ELEVATION(ft): DATE STARTED: 3/3/15  
WATER TABLE (ft): 5.5 DATE FINISHED: 3/3/15  
DATE OF READING: 3-3-15 DRILLED BY: R. WOODARD  
EST. WSWT (ft): TYPE OF SAMPLING: ASTM D-1586

DEPTH (FT.)	S A M P L E	BLOWS PER 6" INCREMENT	N VALUE	W.T.	S Y M B O L	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS		K (FT./ DAY)	ORG. CONT. (%)
									LL	PI		
0						Very loose light brown to tan SAND, with silt [SP-SM]						
1												
2		1/18"	1									
3												
4		WOH-WOH-1	1									
5												
6		1-1-1	2			Medium dense gray and orange clayey SAND [SC]						
7		3-4-6	10									
8		6-7-6	13									
9												
10		5-5-6	11									
11												
12						Medium dense green and orange clayey SAND to sandy CLAY [SC/CH]						
13												
14												
15		6-8-9	17			Boring Terminated at 15'						





# UNIVERSAL ENGINEERING SCIENCES BORING LOG

PROJECT NO.: 0230.1500024.0000

REPORT NO.: 1205846

PAGE: A-5

PROJECT: SCENTWORX SMA  
10908 RACHAEL BOULEVARD  
ALACHUA, ALACHUA COUNTY, FLORIDA

BORING NO: **B-4** SHEET: **1 of 1**

CLIENT: JOSE PERUYERO  
LOCATION: SEE BORING LOCATION PLAN  
REMARKS:

SECTION: 24 TOWNSHIP: 8 S RANGE: 18 E  
GS ELEVATION(ft): DATE STARTED: 3/3/15  
WATER TABLE (ft): NE DATE FINISHED: 3/3/15  
DATE OF READING: NA DRILLED BY: R. WOODARD  
EST. WSWT (ft): TYPE OF SAMPLING: ASTM D-1586

DEPTH (FT.)	S A M P L E	BLOWS PER 6" INCREMENT	N VALUE	W.T.	S Y M B O L	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS		K (FT./ DAY)	ORG. CONT. (%)
									LL	PI		
0						Very loose light brown to tan SAND, with silt [SP-SM]						
1												
2		1-1-1	2				7	7			15	
3												
4		1/12" - 1	1			Very loose brown clayey SAND [SC]						
5		2-4-5	9									
6												
7		3-4-6	10									
8		3-4-7	11			Medium dense brown, orange and gray clayey SAND to sandy CLAY [SC/CH]						
9												
10		4-7-8	15									
11												
12												
13												
14												
15		4-4-5	9			Boring Terminated at 15'						





## KEY TO BORING LOGS

### SYMBOLS

22	Number of Blows of a 140-lb Weight Falling 30 in. Required to Drive Standard Spoon One Foot
WOR	Weight of Drill Rods
S	Thin-Wall Shelby Tube Undisturbed Sampler Used
90% Rec.	Percent Core Recovery from Rock Core-Drilling Operations
	Sample Taken at this Level
	Sample Not Taken at this Level
	Change in Soil Strata
	Free Ground Water Level
	Seasonal High Ground Water Level

### RELATIVE DENSITY (sand-silt)

Very loose - Less Than 4 Blows/Ft.  
 Loose - 4 to 10 Blows/Ft.  
 Medium Dense - 10 to 30 Blows/Ft.  
 Dense - 30 to 50 Blows/Ft.  
 Very Dense - More Than 50 Blows/Ft.

### CONSISTANCY (clay)

Very Soft - Less Than 2 Blows/Ft.  
 Soft - 2 to 4 Blows/Ft.  
 Firm - 4 to 8 Blows/Ft.  
 Stiff - 8 to 15 Blows/Ft.  
 Very Stiff - 15 to 30 Blows/Ft.  
 Hard - More Than 30 Blows/Ft.

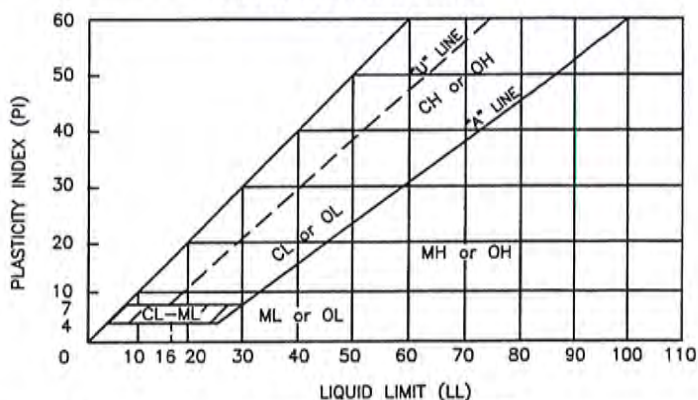
Based on Safety Hammer N-Values

### UNIFIED CLASSIFICATION SYSTEM

MAJOR DIVISIONS			GROUP SYMBOLS	TYPICAL NAMES
COARSE-GRAINED SOILS More than 50% retained on No. 200 sieve*	GRAVELS 50% or more of coarse fraction retained on No. 200 sieve	CLEAN GRAVELS	GW	Well-graded gravels and gravel-sand mixtures, little or no fines
			GP	Poorly graded gravels and gravel-sand mixtures, little or no fines
		GRAVELS WITH FINES	GM	Silty gravels, gravel-sand-silt mixtures
			GC	Clayey gravels, gravel-sand-clay mixtures
	SANDS More than 50% of coarse fraction passes No. 4 sieve	CLEAN SANDS	SW	Well-graded sands and gravelly sands, little or no fines
			SP	Poorly graded sands and gravelly sands, little or no fines
		SANDS WITH FINES	SM	Silty sands, sand-silt mixtures
			SC	Clayey sands, sand-clay mixtures
FINE-GRAINED SOILS 50% or more passes No. 200 sieve*	SILTS AND CLAYS Liquid limit 50% or less	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands	
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays silty clays, lean clays	
		OL	Organic silts and organic silty clays of low plasticity	
	SILTS AND CLAYS Liquid limit greater than 50%	MH	Inorganic silts, micaceous or diatomaceous fine sands or silts, elastic silts	
		CH	Inorganic clays or high plasticity, fat clays	
		OH	Organic clays of medium to high plasticity	
	Highly organic Soils		PT	Peat, muck and other highly organic soils

\* Based on the material passing the 3-in. (75mm) sieve.

### PLASTICITY CHART

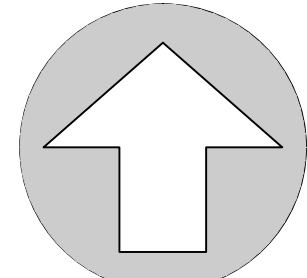
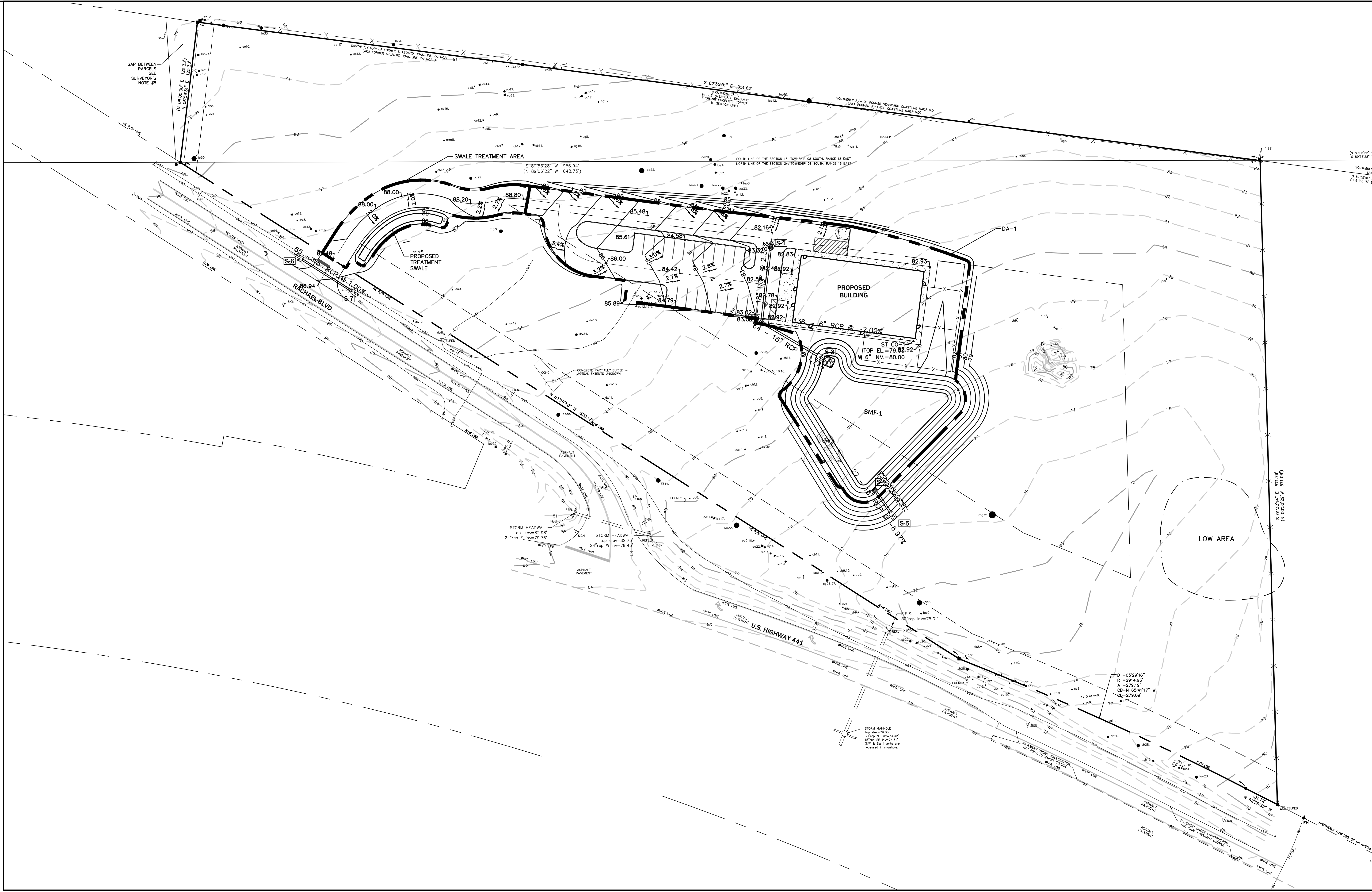


## Attachment B

### Pre-Development and Post-Development Drainage Maps

<div>PRE</div>	Project phase:		CITY SUBMITTAL		Project:	<div><div><div></div></div></div> <div>NORTH</div> <div>SCALE: 1" = 40'</div> <div><div>0204080</div><div></div></div> <div>GRAPHIC SCALE</div>	No.	Date	Comment
	Designed: CAG		Drawn: JAB						
	Project No.: 15-141		Date: 3/31/2015						
	Professional Engineer of Record:								
	Christopher A. Omner Engineer								
Sheet No.:					PEPEDOGS				
					PRE-DEVELOPMENT				
					Sheet title:				
					71599 Certificate No.				





NORTH

SCALE: 1" = 40'



GRAPHIC SCALE



engineers • surveyors • planners  
EB 2389  
104 N.W. 43rd ST., GAINESVILLE, FLORIDA 32606-6602

PEPEDOGS

PAVING, GRADING AND DRAINAGE PLAN

**Project:**

CITY SUBMITTAL.

CAG	Drawn: JAB
-----	------------

15-141	Date: 3/31/2015
--------	-----------------

Christopher A. Gmuer  
Engineer

71399  
Certificate No.

Sheet No.:

## POST

## Attachment C

Pre-Development Time of Concentration WinTR-55 Model  
Pre-Development and Post-Development Conditions PONDs Model

C. Drane

PEPE DOGS  
PRE  
Alachua County, Florida

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
-----							
PRE							
SHEET	100	0.0258	0.410				0.278
SHALLOW	191	0.0258	0.050				0.020
Time of Concentration							.298
							=====



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**Project Data**

Project Name: PePeDogs - SMF-1  
Simulation Description: 100 Year Critical, WQTV  
Project Number: P15141  
Engineer : C. Gmuer, PE  
Supervising Engineer: C. Drane, EI  
Date: 05-27-2015

**Aquifer Data**

Base Of Aquifer Elevation, [B] (ft datum): 74.50  
Water Table Elevation, [WT] (ft datum): 75.00  
Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day): 9.00  
Fillable Porosity, [n] (%): 25.00  
Unsaturated Vertical Infiltration Rate, [Iv] (ft/day): 4.5  
Maximum Area For Unsaturated Infiltration, [Av] (ft²): 11272.0

**Geometry Data**

Equivalent Pond Length, [L] (ft): 174.0  
Equivalent Pond Width, [W] (ft): 57.0  
Ground water mound is expected to intersect the pond bottom

**Stage vs Area Data**

Stage (ft datum)	Area (ft²)
78.25	6859.0
79.00	7987.0
80.00	9579.0
81.00	11272.0
82.00	13065.0

**Discharge Structures**

**Discharge Structure #1 is active as orifice**

**Structure Parameters**

Description: 3.75" Diameter  
Orifice elevation, (ft datum): 80.25  
Orifice coefficient: 4.9  
Orifice area, (ft²): 0.076699039  
Orifice exponent: 0.5

Tailwater - disabled, free discharge

**Discharge Structures (cont'd.)**

Discharge Structure #2 is inactive

Discharge Structure #3 is inactive

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**Scenario Input Data**

*Scenario 1 :: 100YR-001HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     4.4  
Design Rainfall Duration (hours)    1.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 1 Hour

Initial ground water level (ft datum)   75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 2 :: 100YR-002HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     5.4  
Design Rainfall Duration (hours)    2.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 2 Hour

Initial ground water level (ft datum)   75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	



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**Scenario Input Data (cont'd.)**

*Scenario 3 :: 100YR-004HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     6.7  
Design Rainfall Duration (hours)     4.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 4 Hour

Initial ground water level (ft datum)     75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 4 :: 100YR-008HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     8.0  
Design Rainfall Duration (hours)     8.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 8 Hour

Initial ground water level (ft datum)     75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

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**Scenario Input Data (cont'd.)**

*Scenario 5 :: 100YR-024HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     11.0  
Design Rainfall Duration (hours)    24.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 24 Hour

Initial ground water level (ft datum)   75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 6 :: 100YR-072HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     13.8  
Design Rainfall Duration (hours)    72.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 72 Hour

Initial ground water level (ft datum)   75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

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**Scenario Input Data (cont'd.)**

*Scenario 7 :: 100YR-168HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     16.0  
Design Rainfall Duration (hours)    168.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 168 Hour

Initial ground water level (ft datum)   75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 8 :: 100YR-240HR POST*

Hydrograph Type:     Inline SCS  
Modflow Routing:     Routed with infiltration  
Repetitions:         1

Basin Area (acres)         1.140  
Time Of Concentration (minutes)     6.0  
DCIA (%)                 0.0  
Curve Number             87  
Design Rainfall Depth (inches)     18.0  
Design Rainfall Duration (hours)    240.0  
Shape Factor             UHG 484  
Rainfall Distribution         FDOT 240 Hour

Initial ground water level (ft datum)   75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 9 :: WQTV = 4,137CF*

Hydrograph Type:     Slug Load  
Modflow Routing:     Routed with infiltration

Treatment Volume (ft<sup>3</sup>)             4137

Initial ground water level (ft datum)   75.00 (default)



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**Scenario Input Data (cont'd.)**

*Scenario 9 (cont'd.) :: Slug Load :: WQTV = 4,137CF*

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 10 :: 100YR-001HR PRE*

Hydrograph Type: Inline SCS  
**• Modflow Routing: Not routed**  
 Repetitions: 1

Basin Area (acres) 1.140  
 Time Of Concentration (minutes) 18.0  
 DCIA (%) 0.0  
 Curve Number 39  
 Design Rainfall Depth (inches) 4.4  
 Design Rainfall Duration (hours) 1.0  
 Shape Factor UHG 323  
 Rainfall Distribution FDOT 1 Hour

Initial ground water level (ft datum) 75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 11 :: 100YR-002HR PRE*

Hydrograph Type: Inline SCS  
**• Modflow Routing: Not routed**  
 Repetitions: 1

Basin Area (acres) 1.140  
 Time Of Concentration (minutes) 18.0  
 DCIA (%) 0.0  
 Curve Number 39  
 Design Rainfall Depth (inches) 5.4  
 Design Rainfall Duration (hours) 2.0  
 Shape Factor UHG 323  
 Rainfall Distribution FDOT 2 Hour

Initial ground water level (ft datum) 75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

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**Scenario Input Data (cont'd.)**

*Scenario 12 :: 100YR-004HR PRE*

Hydrograph Type:      Inline SCS  
• **Modflow Routing:**    **Not routed**  
Repetitions:            1

Basin Area (acres)            1.140  
Time Of Concentration (minutes)    18.0  
DCIA (%)                      0.0  
Curve Number                39  
Design Rainfall Depth (inches)    6.7  
Design Rainfall Duration (hours)    4.0  
Shape Factor                UHG 323  
Rainfall Distribution            FDOT 4 Hour

Initial ground water level (ft datum)    75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 13 :: 100YR-008HR PRE*

Hydrograph Type:      Inline SCS  
• **Modflow Routing:**    **Not routed**  
Repetitions:            1

Basin Area (acres)            1.140  
Time Of Concentration (minutes)    18.0  
DCIA (%)                      0.0  
Curve Number                39  
Design Rainfall Depth (inches)    8.0  
Design Rainfall Duration (hours)    8.0  
Shape Factor                UHG 323  
Rainfall Distribution            FDOT 8 Hour

Initial ground water level (ft datum)    75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

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**Scenario Input Data (cont'd.)**

*Scenario 14 :: 100YR-024HR PRE*

Hydrograph Type:      Inline SCS  
**• Modflow Routing:**    **Not routed**  
Repetitions:            1

Basin Area (acres)            1.140  
Time Of Concentration (minutes)    18.0  
DCIA (%)                        0.0  
Curve Number                 39  
Design Rainfall Depth (inches)    11.0  
Design Rainfall Duration (hours)    24.0  
Shape Factor                  UHG 323  
Rainfall Distribution            FDOT 24 Hour

Initial ground water level (ft datum)    75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 15 :: 100YR-072HR PRE*

Hydrograph Type:      Inline SCS  
**• Modflow Routing:**    **Not routed**  
Repetitions:            1

Basin Area (acres)            1.140  
Time Of Concentration (minutes)    18.0  
DCIA (%)                        0.0  
Curve Number                 39  
Design Rainfall Depth (inches)    13.8  
Design Rainfall Duration (hours)    72.0  
Shape Factor                  UHG 323  
Rainfall Distribution            FDOT 72 Hour

Initial ground water level (ft datum)    75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

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**Scenario Input Data (cont'd.)**

*Scenario 16 :: 100YR-168HR PRE*

Hydrograph Type:      Inline SCS  
• **Modflow Routing:**    **Not routed**  
Repetitions:            1

Basin Area (acres)            1.140  
Time Of Concentration (minutes)    18.0  
DCIA (%)                      0.0  
Curve Number                39  
Design Rainfall Depth (inches)    16.0  
Design Rainfall Duration (hours)   168.0  
Shape Factor                 UHG 323  
Rainfall Distribution           FDOT 168 Hour

Initial ground water level (ft datum)    75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

*Scenario 17 :: 100YR-240HR PRE*

Hydrograph Type:      Inline SCS  
• **Modflow Routing:**    **Not routed**  
Repetitions:            1

Basin Area (acres)            1.140  
Time Of Concentration (minutes)    18.0  
DCIA (%)                      0.0  
Curve Number                39  
Design Rainfall Depth (inches)    18.0  
Design Rainfall Duration (hours)   240.0  
Shape Factor                 UHG 323  
Rainfall Distribution           FDOT 240 Hour

Initial ground water level (ft datum)    75.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	



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**Sort-By-Category Report**

Scenarios Considered: 1 to 17

**Stage - Maximum**

Rank	Scenario Number	Maximum Stage (ft datum)	Time (hours)	Description
1	8	80.919	184.080	100YR-240HR POST
2	7	80.917	160.067	100YR-168HR POST
3	6	80.790	64.053	100YR-072HR POST
4	5	80.513	21.093	100YR-024HR POST
5	4	80.238	6.040	100YR-008HR POST
6	3	80.207	3.573	100YR-004HR POST
7	2	79.887	2.040	100YR-002HR POST
8	1	79.663	0.973	100YR-001HR POST
9	9	78.817	0.002	WQTV = 4,137CF
10	10	Not Available	Not Available	100YR-001HR PRE
11	11	Not Available	Not Available	100YR-002HR PRE
12	12	Not Available	Not Available	100YR-004HR PRE
13	13	Not Available	Not Available	100YR-008HR PRE
14	14	Not Available	Not Available	100YR-024HR PRE
15	15	Not Available	Not Available	100YR-072HR PRE
16	16	Not Available	Not Available	100YR-168HR PRE
17	17	Not Available	Not Available	100YR-240HR PRE

**Discharge - Rate - Maximum Positive**

Rank	Scenario Number	Maximum Positive Discharge Rate (ft <sup>3</sup> /s)	Time (hours)	Description
1	13	0.6204	4.120	100YR-008HR PRE
2	12	0.5729	3.040	100YR-004HR PRE
3	17	0.4715	184.000	100YR-240HR PRE
4	15	0.4448	60.000	100YR-072HR PRE
5	16	0.3684	160.000	100YR-168HR PRE
6	14	0.3206	12.080	100YR-024HR PRE
7	8	0.3075	184.080	100YR-240HR POST
8	7	0.3068	160.067	100YR-168HR POST
9	6	0.2761	64.053	100YR-072HR POST
10	11	0.2658	1.520	100YR-002HR PRE
11	10	0.2198	0.920	100YR-001HR PRE
12	5	0.1927	21.093	100YR-024HR POST
13	1	None	N.A.	100YR-001HR POST
14	2	None	N.A.	100YR-002HR POST
15	3	None	N.A.	100YR-004HR POST
16	4	None	N.A.	100YR-008HR POST
17	9	None	N.A.	WQTV = 4,137CF

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**Sort-By-Category Report (cont'd.)**

**Discharge - Cumulative Volume - Maximum Positive**

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft <sup>3</sup> )	Time (hours)	Description
1	17	30086.5	241.520	100YR-240HR PRE
2	16	24119.4	169.560	100YR-168HR PRE
3	7	20396.9	192.347	100YR-168HR POST
4	8	19163.6	240.173	100YR-240HR POST
5	15	17965.3	73.520	100YR-072HR PRE
6	6	16902.1	96.347	100YR-072HR POST
7	14	11031.5	25.560	100YR-024HR PRE
8	5	10478.0	48.347	100YR-024HR POST
9	13	4802.6	9.560	100YR-008HR PRE
10	12	2784.2	5.560	100YR-004HR PRE
11	11	1195.9	3.560	100YR-002HR PRE
12	10	397.0	2.560	100YR-001HR PRE
13	1	None	N.A.	100YR-001HR POST
14	2	None	N.A.	100YR-002HR POST
15	3	None	N.A.	100YR-004HR POST
16	4	None	N.A.	100YR-008HR POST
17	9	None	N.A.	WQTV = 4,137CF

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**Detailed Results (cont,d.)**    :: Scenario 1 :: 100YR-001HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.067	0.1493	0.0000	79.656	0.47055	0.00000	12423.5	1270.4	0.0	U/P
1.080	0.1097	0.0000	79.655	0.47039	0.00000	12429.7	1293.0	0.0	U/P
1.093	0.0773	0.0000	79.653	0.47022	0.00000	12434.2	1315.6	0.0	U/P
1.107	0.0543	0.0000	79.650	0.47004	0.00000	12437.4	1338.1	0.0	U/P
1.120	0.0385	0.0000	79.648	0.46985	0.00000	12439.6	1360.7	0.0	U/P
1.133	0.0277	0.0000	79.646	0.46965	0.00000	12441.2	1383.2	0.0	U/P
1.147	0.0198	0.0000	79.644	0.46945	0.00000	12442.3	1405.8	0.0	U/P
1.160	0.0142	0.0000	79.641	0.46925	0.00000	12443.1	1428.3	0.0	U/P
1.173	0.0100	0.0000	79.639	0.46905	0.00000	12443.7	1450.8	0.0	U/P
1.187	0.0071	0.0000	79.636	0.46885	0.00000	12444.1	1473.3	0.0	U/P
1.200	0.0050	0.0000	79.634	0.46864	0.00000	12444.4	1495.8	0.0	U/P
1.213	0.0034	0.0000	79.631	0.46844	0.00000	12444.6	1518.3	0.0	U/P
1.227	0.0024	0.0000	79.629	0.46823	0.00000	12444.8	1540.8	0.0	U/P
1.240	0.0016	0.0000	79.626	0.46802	0.00000	12444.9	1563.3	0.0	U/P
1.253	0.0011	0.0000	79.624	0.46782	0.00000	12444.9	1585.7	0.0	U/P
1.267	0.0007	0.0000	79.621	0.46761	0.00000	12445.0	1608.2	0.0	U/P
1.280	0.0004	0.0000	79.619	0.46740	0.00000	12445.0	1630.6	0.0	U/P
1.293	0.0002	0.0000	79.616	0.46719	0.00000	12445.0	1653.1	0.0	U/P
1.307	0.0001	0.0000	79.614	0.46699	0.00000	12445.0	1675.5	0.0	U/P
1.320	0.0000	0.0000	79.611	0.46678	0.00000	12445.0	1697.9	0.0	U/P
1.333	0.0000	0.0000	79.609	0.46657	0.00000	12445.0	1720.3	0.0	U/P
1.347	0.0000	0.0000	79.606	0.46628	0.00000	12445.0	1742.7	0.0	U/P
25.347	0.0000	0.0000	77.747	0.06193	0.00000	12445.0	12445.0	0.0	U/S
49.347	0.0000	0.0000	77.350	0.00000	0.00000	12445.0	12445.0	0.0	S
73.347	0.0000	0.0000	77.117	0.00000	0.00000	12445.0	12445.0	0.0	S
97.347	0.0000	0.0000	76.954	0.00000	0.00000	12445.0	12445.0	0.0	S
121.347	0.0000	0.0000	76.829	0.00000	0.00000	12445.0	12445.0	0.0	S
145.347	0.0000	0.0000	76.729	0.00000	0.00000	12445.0	12445.0	0.0	S
169.347	0.0000	0.0000	76.646	0.00000	0.00000	12445.0	12445.0	0.0	S
193.347	0.0000	0.0000	76.576	0.00000	0.00000	12445.0	12445.0	0.0	S
217.347	0.0000	0.0000	76.514	0.00000	0.00000	12445.0	12445.0	0.0	S
241.347	0.0000	0.0000	76.460	0.00000	0.00000	12445.0	12445.0	0.0	S
265.347	0.0000	0.0000	76.412	0.00000	0.00000	12445.0	12445.0	0.0	S
289.347	0.0000	0.0000	76.369	0.00000	0.00000	12445.0	12445.0	0.0	S
313.347	0.0000	0.0000	76.330	0.00000	0.00000	12445.0	12445.0	0.0	S
337.347	0.0000	0.0000	76.294	0.00000	0.00000	12445.0	12445.0	0.0	S
361.347	0.0000	0.0000	76.261	0.00000	0.00000	12445.0	12445.0	0.0	S
721.347	0.0000	0.0000	76.002	----	----	12445.0	12445.0	0.0	N.A.

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**Detailed Results (cont,d.)**    :: Scenario 2 :: 100YR-002HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
2.133	0.0644	0.0000	79.878	0.48887	0.00000	16325.6	3135.9	0.0	U/P
2.147	0.0462	0.0000	79.875	0.48868	0.00000	16328.2	3159.4	0.0	U/P
2.160	0.0332	0.0000	79.873	0.48849	0.00000	16330.1	3182.9	0.0	U/P
2.173	0.0237	0.0000	79.871	0.48830	0.00000	16331.5	3206.3	0.0	U/P
2.187	0.0169	0.0000	79.868	0.48810	0.00000	16332.5	3229.7	0.0	U/P
2.200	0.0121	0.0000	79.866	0.48789	0.00000	16333.2	3253.2	0.0	U/P
2.213	0.0085	0.0000	79.863	0.48769	0.00000	16333.7	3276.6	0.0	U/P
2.227	0.0060	0.0000	79.861	0.48748	0.00000	16334.0	3300.0	0.0	U/P
2.240	0.0041	0.0000	79.859	0.48728	0.00000	16334.3	3323.4	0.0	U/P
2.253	0.0028	0.0000	79.856	0.48707	0.00000	16334.4	3346.8	0.0	U/P
2.267	0.0018	0.0000	79.854	0.48687	0.00000	16334.5	3370.1	0.0	U/P
2.280	0.0011	0.0000	79.851	0.48666	0.00000	16334.6	3393.5	0.0	U/P
2.293	0.0005	0.0000	79.849	0.48645	0.00000	16334.7	3416.8	0.0	U/P
2.307	0.0002	0.0000	79.846	0.48625	0.00000	16334.7	3440.2	0.0	U/P
2.320	0.0000	0.0000	79.844	0.48604	0.00000	16334.7	3463.5	0.0	U/P
2.333	0.0000	0.0000	79.841	0.48583	0.00000	16334.7	3486.8	0.0	U/P
2.347	0.0000	0.0000	79.839	0.48553	0.00000	16334.7	3510.2	0.0	U/P
26.347	0.0000	0.0000	78.461	0.07422	0.00000	16334.7	14854.6	0.0	U/S
50.347	0.0000	0.0000	78.171	0.00857	0.00000	16334.7	16334.7	0.0	S
74.347	0.0000	0.0000	77.819	0.00000	0.00000	16334.7	16334.7	0.0	S
98.347	0.0000	0.0000	77.582	0.00000	0.00000	16334.7	16334.7	0.0	S
122.347	0.0000	0.0000	77.404	0.00000	0.00000	16334.7	16334.7	0.0	S
146.347	0.0000	0.0000	77.264	0.00000	0.00000	16334.7	16334.7	0.0	S
170.347	0.0000	0.0000	77.149	0.00000	0.00000	16334.7	16334.7	0.0	S
194.347	0.0000	0.0000	77.051	0.00000	0.00000	16334.7	16334.7	0.0	S
218.347	0.0000	0.0000	76.968	0.00000	0.00000	16334.7	16334.7	0.0	S
242.347	0.0000	0.0000	76.894	0.00000	0.00000	16334.7	16334.7	0.0	S
266.347	0.0000	0.0000	76.829	0.00000	0.00000	16334.7	16334.7	0.0	S
290.347	0.0000	0.0000	76.770	0.00000	0.00000	16334.7	16334.7	0.0	S
314.347	0.0000	0.0000	76.718	0.00000	0.00000	16334.7	16334.7	0.0	S
338.347	0.0000	0.0000	76.670	0.00000	0.00000	16334.7	16334.7	0.0	S
362.347	0.0000	0.0000	76.626	0.00000	0.00000	16334.7	16334.7	0.0	S
722.347	0.0000	0.0000	76.286	----	----	16334.7	16334.7	0.0	N.A.



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**Detailed Results (cont,d.)**    :: Scenario 3 :: 100YR-004HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
4.267	0.0009	0.0000	80.135	0.51093	0.00000	21565.9	5906.3	0.0	U/P
4.280	0.0005	0.0000	80.133	0.51071	0.00000	21565.9	5930.9	0.0	U/P
4.293	0.0003	0.0000	80.130	0.51049	0.00000	21565.9	5955.4	0.0	U/P
4.307	0.0001	0.0000	80.128	0.51027	0.00000	21565.9	5979.9	0.0	U/P
4.320	0.0000	0.0000	80.125	0.51005	0.00000	21565.9	6004.3	0.0	U/P
4.333	0.0000	0.0000	80.123	0.50982	0.00000	21565.9	6028.8	0.0	U/P
4.347	0.0000	0.0000	80.120	0.50949	0.00000	21565.9	6053.3	0.0	U/P
28.347	0.0000	0.0000	79.019	0.06926	0.00000	21565.9	15849.5	0.0	U/S
52.347	0.0000	0.0000	78.740	0.02101	0.00000	21565.9	18021.4	0.0	S
76.347	0.0000	0.0000	78.545	0.01489	0.00000	21565.9	19479.7	0.0	S
100.347	0.0000	0.0000	78.390	0.01169	0.00000	21565.9	20594.5	0.0	S
124.347	0.0000	0.0000	78.260	0.00562	0.00000	21565.9	21500.1	0.0	S
148.347	0.0000	0.0000	78.034	0.00038	0.00000	21565.9	21565.9	0.0	S
172.347	0.0000	0.0000	77.851	0.00000	0.00000	21565.9	21565.9	0.0	S
196.347	0.0000	0.0000	77.702	0.00000	0.00000	21565.9	21565.9	0.0	S
220.347	0.0000	0.0000	77.578	0.00000	0.00000	21565.9	21565.9	0.0	S
244.347	0.0000	0.0000	77.471	0.00000	0.00000	21565.9	21565.9	0.0	S
268.347	0.0000	0.0000	77.378	0.00000	0.00000	21565.9	21565.9	0.0	S
292.347	0.0000	0.0000	77.295	0.00000	0.00000	21565.9	21565.9	0.0	S
316.347	0.0000	0.0000	77.221	0.00000	0.00000	21565.9	21565.9	0.0	S
340.347	0.0000	0.0000	77.154	0.00000	0.00000	21565.9	21565.9	0.0	S
364.347	0.0000	0.0000	77.093	0.00000	0.00000	21565.9	21565.9	0.0	S
724.347	0.0000	0.0000	76.640	----	----	21565.9	21565.9	0.0	N.A.

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**Detailed Results (cont,d.)**    :: Scenario 4 :: 100YR-008HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
7.467	0.2685	0.0000	80.177	0.51467	0.00000	26110.2	10039.5	0.0	U/P
7.480	0.2685	0.0000	80.176	0.51456	0.00000	26123.1	10064.2	0.0	U/P
7.493	0.2685	0.0000	80.174	0.51446	0.00000	26136.0	10088.9	0.0	U/P
7.507	0.2685	0.0000	80.173	0.51435	0.00000	26148.9	10113.5	0.0	U/P
7.520	0.2685	0.0000	80.172	0.51425	0.00000	26161.8	10138.2	0.0	U/P
7.533	0.2685	0.0000	80.171	0.51414	0.00000	26174.7	10162.9	0.0	U/P
7.547	0.2685	0.0000	80.170	0.51404	0.00000	26187.6	10187.6	0.0	U/P
7.560	0.2685	0.0000	80.168	0.51393	0.00000	26200.5	10212.3	0.0	U/P
7.573	0.2685	0.0000	80.167	0.51383	0.00000	26213.4	10236.9	0.0	U/P
7.587	0.2685	0.0000	80.166	0.51372	0.00000	26226.2	10261.6	0.0	U/P
7.600	0.2685	0.0000	80.165	0.51362	0.00000	26239.1	10286.2	0.0	U/P
7.613	0.2685	0.0000	80.164	0.51351	0.00000	26252.0	10310.9	0.0	U/P
7.627	0.2685	0.0000	80.163	0.51340	0.00000	26264.9	10335.5	0.0	U/P
7.640	0.2686	0.0000	80.161	0.51330	0.00000	26277.8	10360.2	0.0	U/P
7.653	0.2686	0.0000	80.160	0.51319	0.00000	26290.7	10384.8	0.0	U/P
7.667	0.2686	0.0000	80.159	0.51309	0.00000	26303.6	10409.4	0.0	U/P
7.680	0.2686	0.0000	80.158	0.51298	0.00000	26316.5	10434.1	0.0	U/P
7.693	0.2686	0.0000	80.157	0.51288	0.00000	26329.4	10458.7	0.0	U/P
7.707	0.2686	0.0000	80.155	0.51277	0.00000	26342.3	10483.3	0.0	U/P
7.720	0.2686	0.0000	80.154	0.51267	0.00000	26355.1	10507.9	0.0	U/P
7.733	0.2686	0.0000	80.153	0.51256	0.00000	26368.0	10532.5	0.0	U/P
7.747	0.2686	0.0000	80.152	0.51246	0.00000	26380.9	10557.1	0.0	U/P
7.760	0.2686	0.0000	80.151	0.51235	0.00000	26393.8	10581.7	0.0	U/P
7.773	0.2686	0.0000	80.149	0.51225	0.00000	26406.7	10606.3	0.0	U/P
7.787	0.2686	0.0000	80.148	0.51214	0.00000	26419.6	10630.9	0.0	U/P
7.800	0.2686	0.0000	80.147	0.51204	0.00000	26432.5	10655.5	0.0	U/P
7.813	0.2686	0.0000	80.146	0.51193	0.00000	26445.4	10680.1	0.0	U/P
7.827	0.2686	0.0000	80.145	0.41778	0.00000	26458.3	10704.6	0.0	U/P
7.840	0.2686	0.0000	80.144	0.22981	0.00000	26471.2	10720.2	0.0	U/S
7.853	0.2686	0.0000	80.145	0.13913	0.00000	26484.1	10726.7	0.0	S
7.867	0.2686	0.0000	80.146	0.14561	0.00000	26497.0	10733.5	0.0	S
7.880	0.2686	0.0000	80.146	0.15227	0.00000	26509.9	10740.7	0.0	S
7.893	0.2686	0.0000	80.147	0.15911	0.00000	26522.8	10748.1	0.0	S
7.907	0.2687	0.0000	80.147	0.16612	0.00000	26535.7	10755.9	0.0	S
7.920	0.2687	0.0000	80.148	0.17330	0.00000	26548.6	10764.1	0.0	S
7.933	0.2687	0.0000	80.148	0.18064	0.00000	26561.5	10772.6	0.0	S
7.947	0.2687	0.0000	80.149	0.18812	0.00000	26574.3	10781.4	0.0	S
7.960	0.2687	0.0000	80.149	0.19574	0.00000	26587.2	10790.6	0.0	S
7.973	0.2687	0.0000	80.149	0.20347	0.00000	26600.1	10800.2	0.0	S
7.987	0.2687	0.0000	80.150	0.21131	0.00000	26613.0	10810.2	0.0	S
8.000	0.2687	0.0000	80.150	0.21924	0.00000	26625.9	10820.5	0.0	S
8.013	0.2647	0.0000	80.150	0.22724	0.00000	26638.7	10831.2	0.0	S
8.027	0.2522	0.0000	80.150	0.23528	0.00000	26651.1	10842.3	0.0	S
8.040	0.2256	0.0000	80.150	0.24335	0.00000	26662.6	10853.8	0.0	S
8.053	0.1881	0.0000	80.150	0.25142	0.00000	26672.5	10865.7	0.0	S
8.067	0.1478	0.0000	80.150	0.25945	0.00000	26680.6	10877.9	0.0	S
8.080	0.1104	0.0000	80.149	0.26743	0.00000	26686.8	10890.6	0.0	S
8.093	0.0790	0.0000	80.148	0.27532	0.00000	26691.3	10903.6	0.0	S
8.107	0.0564	0.0000	80.147	0.28311	0.00000	26694.6	10917.0	0.0	S
8.120	0.0407	0.0000	80.146	0.29076	0.00000	26696.9	10930.8	0.0	S
8.133	0.0294	0.0000	80.145	0.29826	0.00000	26698.6	10944.9	0.0	S
8.147	0.0211	0.0000	80.143	0.30557	0.00000	26699.8	10959.4	0.0	S
8.160	0.0151	0.0000	80.142	0.31266	0.00000	26700.7	10974.3	0.0	S
8.173	0.0108	0.0000	80.140	0.31950	0.00000	26701.3	10989.4	0.0	S
8.187	0.0077	0.0000	80.139	0.32608	0.00000	26701.8	11004.9	0.0	S
8.200	0.0055	0.0000	80.137	0.33237	0.00000	26702.1	11020.7	0.0	S
8.213	0.0039	0.0000	80.136	0.33833	0.00000	26702.3	11036.8	0.0	S
8.227	0.0027	0.0000	80.134	0.34395	0.00000	26702.5	11053.2	0.0	S
8.240	0.0019	0.0000	80.132	0.34919	0.00000	26702.6	11069.9	0.0	S
8.253	0.0013	0.0000	80.131	0.35406	0.00000	26702.6	11086.7	0.0	S
8.267	0.0008	0.0000	80.129	0.35851	0.00000	26702.7	11103.8	0.0	S
8.280	0.0005	0.0000	80.127	0.36255	0.00000	26702.7	11121.2	0.0	S
8.293	0.0002	0.0000	80.125	0.36614	0.00000	26702.7	11138.7	0.0	S
8.307	0.0001	0.0000	80.124	0.36929	0.00000	26702.7	11156.3	0.0	S
8.320	0.0000	0.0000	80.122	0.37198	0.00000	26702.8	11174.1	0.0	S
8.333	0.0000	0.0000	80.120	0.37422	0.00000	26702.8	11192.0	0.0	S
8.347	0.0000	0.0000	80.118	0.37505	0.00000	26702.8	11210.0	0.0	S
32.347	0.0000	0.0000	79.511	0.04745	0.00000	26702.8	16842.9	0.0	S
56.347	0.0000	0.0000	79.212	0.02493	0.00000	26702.8	19409.3	0.0	S
80.347	0.0000	0.0000	78.998	0.01781	0.00000	26702.8	21150.1	0.0	S
104.347	0.0000	0.0000	78.828	0.01404	0.00000	26702.8	22487.1	0.0	S
128.347	0.0000	0.0000	78.685	0.01163	0.00000	26702.8	23576.9	0.0	S
152.347	0.0000	0.0000	78.561	0.00993	0.00000	26702.8	24497.2	0.0	S
176.347	0.0000	0.0000	78.451	0.00865	0.00000	26702.8	25292.7	0.0	S
200.347	0.0000	0.0000	78.352	0.00765	0.00000	26702.8	25992.2	0.0	S
224.347	0.0000	0.0000	78.263	0.00411	0.00000	26702.8	26615.2	0.0	S
248.347	0.0000	0.0000	78.104	0.00051	0.00000	26702.8	26702.8	0.0	S
272.347	0.0000	0.0000	77.960	0.00000	0.00000	26702.8	26702.8	0.0	S
296.347	0.0000	0.0000	77.839	0.00000	0.00000	26702.8	26702.8	0.0	S
320.347	0.0000	0.0000	77.735	0.00000	0.00000	26702.8	26702.8	0.0	S

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**Detailed Results (cont,d.)**    :: Scenario 5 :: 100YR-024HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
360.347	0.0000	0.0000	78.379	0.00547	0.00000	39039.0	27665.7	10478.0	S
384.347	0.0000	0.0000	78.314	0.00496	0.00000	39039.0	28120.9	10478.0	S
744.347	0.0000	0.0000	77.502	----	----	39039.0	28561.1	10478.0	N.A.

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**Detailed Results (cont,d.)**    :: Scenario 6 :: 100YR-072HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
408.347	0.0000	0.0000	78.567	0.00576	0.00000	50326.8	31174.3	16902.1	S
432.347	0.0000	0.0000	78.501	0.00530	0.00000	50326.8	31654.9	16902.1	S
792.347	0.0000	0.0000	77.752	----	----	50326.8	33424.7	16902.1	N.A.



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**Detailed Results (cont,d.)**    :: Scenario 7 :: 100YR-168HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
504.347	0.0000	0.0000	78.828	0.00607	0.00000	59356.2	34746.6	20396.9	S
528.347	0.0000	0.0000	78.761	0.00569	0.00000	59356.2	35254.4	20396.9	S
888.347	0.0000	0.0000	78.108	----	----	59356.2	38959.4	20396.9	N.A.

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**Detailed Results (cont,d.)**    :: Scenario 8 :: 100YR-240HR POST

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
240.000	0.0360	0.0000	80.251	0.02453	0.01146	67570.2	31604.5	19158.4	S
240.013	0.0355	0.0000	80.251	0.02451	0.01146	67571.9	31605.7	19158.9	S
240.027	0.0338	0.0000	80.251	0.02444	0.01142	67573.6	31606.9	19159.5	S
240.040	0.0303	0.0000	80.251	0.02439	0.01131	67575.1	31608.1	19160.0	S
240.053	0.0252	0.0000	80.251	0.02433	0.01107	67576.4	31609.2	19160.6	S
240.067	0.0198	0.0000	80.251	0.02423	0.01068	67577.5	31610.4	19161.1	S
240.080	0.0148	0.0000	80.251	0.02413	0.01012	67578.3	31611.6	19161.6	S
240.093	0.0106	0.0000	80.251	0.02402	0.00938	67579.0	31612.7	19162.0	S
240.107	0.0076	0.0000	80.251	0.02390	0.00848	67579.4	31613.9	19162.5	S
240.120	0.0055	0.0000	80.250	0.02377	0.00740	67579.7	31615.0	19162.9	S
240.133	0.0039	0.0000	80.250	0.02360	0.00611	67579.9	31616.1	19163.2	S
240.147	0.0028	0.0000	80.250	0.02325	0.00448	67580.1	31617.3	19163.4	S
240.160	0.0020	0.0000	80.250	0.02318	0.00187	67580.2	31618.4	19163.6	S
240.173	0.0015	0.0000	80.250	0.02383	0.00000	67580.3	31619.5	19163.6	S
240.187	0.0010	0.0000	80.250	0.02428	0.00000	67580.3	31620.7	19163.6	S
240.200	0.0007	0.0000	80.250	0.02426	0.00000	67580.4	31621.8	19163.6	S
240.213	0.0005	0.0000	80.250	0.02424	0.00000	67580.4	31623.0	19163.6	S
240.227	0.0004	0.0000	80.249	0.02422	0.00000	67580.4	31624.2	19163.6	S
240.240	0.0003	0.0000	80.249	0.02420	0.00000	67580.5	31625.3	19163.6	S
240.253	0.0002	0.0000	80.249	0.02418	0.00000	67580.5	31626.5	19163.6	S
240.267	0.0001	0.0000	80.249	0.02417	0.00000	67580.5	31627.6	19163.6	S
240.280	0.0001	0.0000	80.249	0.02415	0.00000	67580.5	31628.8	19163.6	S
240.293	0.0000	0.0000	80.249	0.02413	0.00000	67580.5	31630.0	19163.6	S
240.307	0.0000	0.0000	80.249	0.02412	0.00000	67580.5	31631.1	19163.6	S
240.320	0.0000	0.0000	80.249	0.02410	0.00000	67580.5	31632.3	19163.6	S
240.333	0.0000	0.0000	80.249	0.02409	0.00000	67580.5	31633.4	19163.6	S
240.347	0.0000	0.0000	80.248	0.02408	0.00000	67580.5	31634.6	19163.6	S
264.347	0.0000	0.0000	80.080	0.01786	0.00000	67580.5	33297.9	19163.6	S
288.347	0.0000	0.0000	79.931	0.01547	0.00000	67580.5	34721.3	19163.6	S
312.347	0.0000	0.0000	79.798	0.01368	0.00000	67580.5	35971.0	19163.6	S
336.347	0.0000	0.0000	79.676	0.01226	0.00000	67580.5	37085.4	19163.6	S
360.347	0.0000	0.0000	79.564	0.01110	0.00000	67580.5	38090.0	19163.6	S
384.347	0.0000	0.0000	79.460	0.01012	0.00000	67580.5	39003.3	19163.6	S
408.347	0.0000	0.0000	79.364	0.00929	0.00000	67580.5	39839.1	19163.6	S
432.347	0.0000	0.0000	79.273	0.00857	0.00000	67580.5	40608.5	19163.6	S
456.347	0.0000	0.0000	79.188	0.00794	0.00000	67580.5	41320.1	19163.6	S
480.347	0.0000	0.0000	79.108	0.00739	0.00000	67580.5	41980.9	19163.6	S
504.347	0.0000	0.0000	79.032	0.00690	0.00000	67580.5	42596.9	19163.6	S
528.347	0.0000	0.0000	78.959	0.00646	0.00000	67580.5	43172.8	19163.6	S
552.347	0.0000	0.0000	78.891	0.00607	0.00000	67580.5	43713.1	19163.6	S
576.347	0.0000	0.0000	78.825	0.00571	0.00000	67580.5	44221.1	19163.6	S
600.347	0.0000	0.0000	78.763	0.00537	0.00000	67580.5	44699.9	19163.6	S
960.347	0.0000	0.0000	78.141	----	----	67580.5	48416.9	19163.6	N.A.

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**Detailed Results**    :: Scenario 9 :: WQTV = 4,137CF

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	689.5000	0.0000	75.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.002	689.5000	0.0000	78.817	0.40169	0.00000	4137.0	2.4	0.0	U/P
24.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
48.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
72.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
96.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
120.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
144.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
168.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
192.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
216.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
240.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
264.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
288.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
312.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
336.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
360.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry
720.000	0.0000	0.0000	----	----	----	4137.0	4137.0	0.0	dry

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**Project Data**

Project Name: PePeDogs - Swale WQTV  
Simulation Description: WQTV  
Project Number: P15141  
Engineer : C. Gmuer, PE  
Supervising Engineer: C. Drane, EI  
Date: 03-27-2015

**Aquifer Data**

Base Of Aquifer Elevation, [B] (ft datum): 82.50  
Water Table Elevation, [WT] (ft datum): 83.00  
Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day): 9.00  
Fillable Porosity, [n] (%): 25.00  
Unsaturated Vertical Infiltration Rate, [Iv] (ft/day): 4.5  
Maximum Area For Unsaturated Infiltration, [Av] (ft²): 1179.0

**Geometry Data**

Equivalent Pond Length, [L] (ft): 96.0  
Equivalent Pond Width, [W] (ft): 9.0  
Ground water mound is expected to intersect the pond bottom

**Stage vs Area Data**

Stage (ft datum)	Area (ft²)
86.00	565.0
87.00	1179.0

**Discharge Structures**

**Discharge Structure #1 is inactive**  
**Discharge Structure #2 is inactive**  
**Discharge Structure #3 is inactive**



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**Scenario Input Data**

*Scenario 1 :: WQTV = 519CF*

Hydrograph Type: Slug Load  
Modflow Routing: Routed with infiltration

Treatment Volume (ft<sup>3</sup>) 519

Initial ground water level (ft datum) 83.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
1.000	6.000	11.000	30.000
2.000	7.000	12.000	
3.000	8.000	13.000	
4.000	9.000	14.000	
5.000	10.000	15.000	

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**Sort-By-Category Report**

Scenarios Considered: 1

**Stage - Maximum**

Rank	Scenario Number	Maximum Stage (ft datum)	Time (hours)	Description
1	1	86.672	0.002	WQTV = 519CF

**Discharge - Rate - Maximum Positive**

Rank	Scenario Number	Maximum Positive Discharge Rate (ft <sup>3</sup> /s)	Time (hours)	Description
1	1	None	N.A.	WQTV = 519CF

**Discharge - Cumulative Volume - Maximum Positive**

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft <sup>3</sup> )	Time (hours)	Description
1	1	None	N.A.	WQTV = 519CF

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**Detailed Results**    :: Scenario 1 :: WQTV = 519CF

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	86.5000	0.0000	83.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.002	86.5000	0.0000	86.672	0.05094	0.00000	519.0	0.3	0.0	U/P
24.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
48.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
72.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
96.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
120.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
144.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
168.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
192.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
216.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
240.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
264.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
288.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
312.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
336.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
360.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry
720.000	0.0000	0.0000	----	----	----	519.0	519.0	0.0	dry

## Attachment D

Aerial Photograph





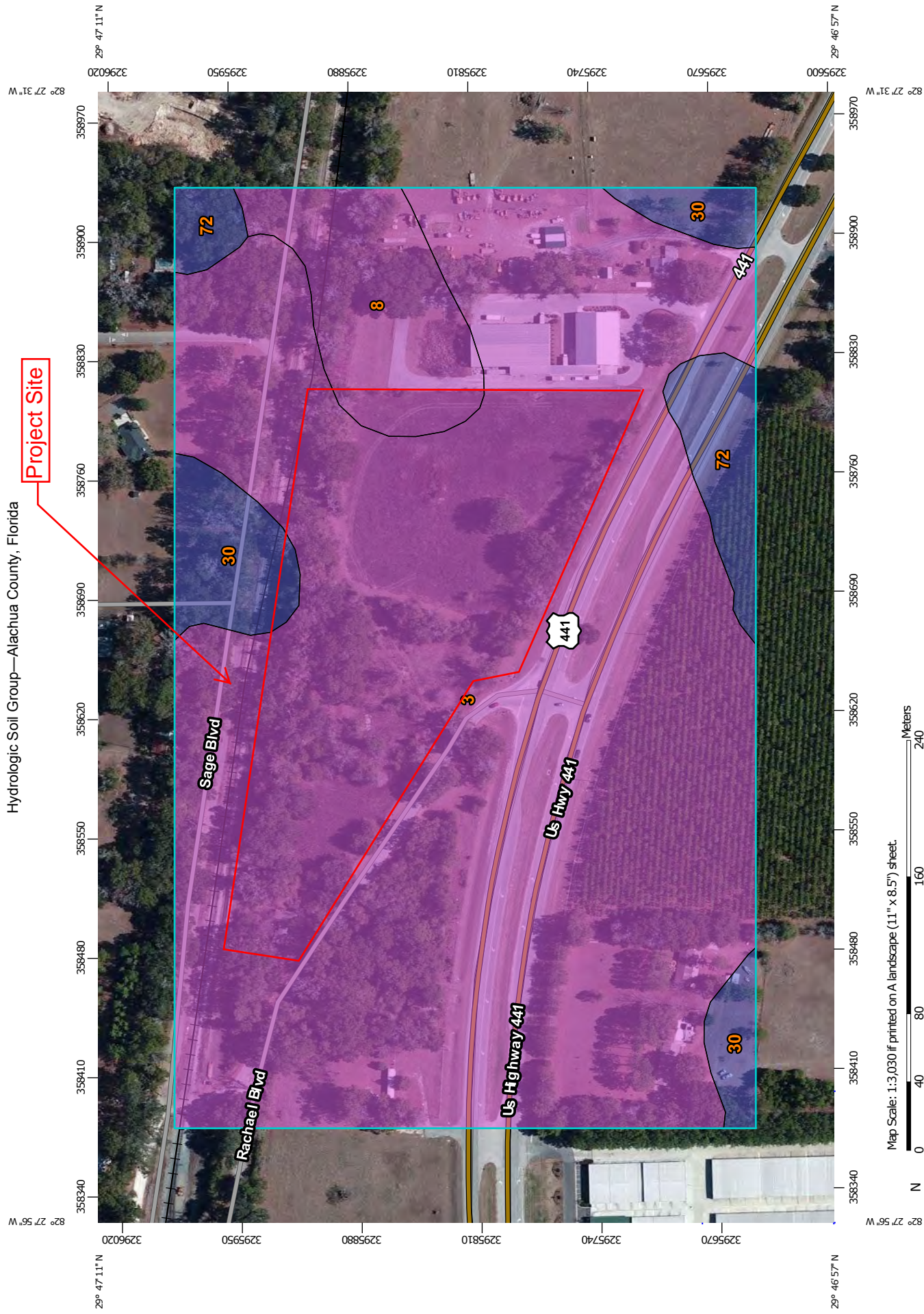
Google earth



## Attachment E

### Soils Map





MAP LEGEND

**Area of Interest (AOI)**

Area of Interest (AOI)

**Soils**

**Soil Rating Polygons**

A

A/D

B

B/D

C

C/D

D

Not rated or not available

**Soil Rating Lines**

A

A/D

B

B/D

C

C/D

D

Not rated or not available

**Soil Rating Points**

A

A/D

B

B/D

**Water Features**

Streams and Canals

**Transportation**

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

**Background**

Aerial Photography

C

C/D

D

Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL: <http://websoilsurvey.nrcs.usda.gov>  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Alachua County, Florida  
Survey Area Data: Version 14, Sep 18, 2014

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 29, 2010—Jan 22, 2011

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Alachua County, Florida (FL001)				
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Arredondo fine sand, 0 to 5 percent slopes	A	39.1	84.1%
8	Millhopper sand, 0 to 5 percent slopes	A	3.1	6.7%
30	Kendrick sand, 2 to 5 percent slopes	B	2.6	5.5%
72	Lochloosa fine sand, 5 to 8 percent slopes	B	1.7	3.7%
<b>Totals for Area of Interest</b>			<b>46.5</b>	<b>100.0%</b>

## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



engineers • surveyors • planners, inc.

April 6, 2015

Brandon Stubbs  
Planner  
City of Alachua  
15100 NW 142<sup>nd</sup> Terrace  
Alachua, FL 32615

Re: PePeDogs – Site Plan Attachment #11 SRWMD ERP

Dear Mr. Stubbs:

For the PePeDogs Project, we plan to submit a 10-2 Self-Certification application with Suwannee River Water Management District. This will be submitted within 30 days after construction begins. The requirements for a 10-2 Self-Certification can be found in Florida Statue 403.841(12) are as follows. The way in which we comply with each requirement is in bold.

(a) The total project area involves less than 10 acres and less than 2 acres of impervious surface.

**Comply: The total project area is 8.04-ac. The total amount of impervious area is 0.72-ac.**

(b) No activities will impact wetlands or other surface waters.

**Comply: There are no wetlands or other surface waters on the project site.**

(c) No activities are conducted in, on, or over wetlands or other surface waters.

**Comply: There are no wetlands or other surface waters on the project site.**

(d) Drainage facilities will not include pipes having diameters greater than 24 inches, or the hydraulic equivalent, and will not use pumps in any manner.

**Comply: The proposed drainage facilities do not include any pipes larger than 18" and does not include any pumps.**

(e) The project is not part of a larger common plan, development, or sale.

**Comply: The project is not part of a larger common plan, development, or sale.**

(f) The project does not:

1. Cause adverse water quantity or flooding impacts to receiving water and adjacent lands;

**Comply: The proposed Stormwater management facility provides for water quality and attenuation of the 100-year storm events.**

2. Cause adverse impacts to existing surface water storage and conveyance capabilities;

**Comply:** There is no existing surface water storage on the project site. The proposed project does not cause adverse impacts to the existing drainage patterns because a bypass berm is provided north of the project to direct existing drainage around the project site to maintain existing drainage patterns.

3. Cause a violation of state water quality standards; or

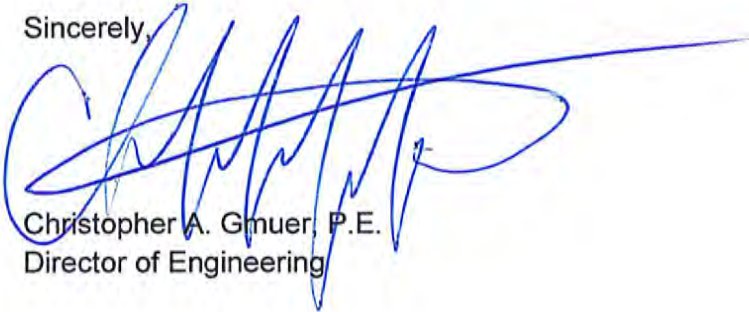
**Comply:** The proposed Stormwater management facility provides for water quality.

4. Cause an adverse impact to the maintenance of surface or ground water levels or surface water flows established pursuant to s. 373.042 or a work of the district established pursuant to s. 373.086.

**Comply:** The proposed Stormwater management facility does not cause an adverse impact to the maintenance of surface or ground water levels or surface water flows. There is no surface water stored on site.

If you have any questions or requests for additional information, please contact our office.

Sincerely,

A handwritten signature in blue ink, appearing to read 'C. Gmuer', with a long horizontal flourish extending to the right.

Christopher A. Gmuer, P.E.  
Director of Engineering



Alachua County Board of County Commissioners  
Public Works Department  
Customer Name & Address:

ScentworX, Inc  
PO Box 769  
High Springs, FL 32655

CODE	DESCRIPTION	AMOUNT
RB	SFH Water Charges Receivable	
DS	SFH Water Deposit	
RF	SFH Reconnection Fee	
RH	WC Cart Exchange	
RI	WC Additional Refuse Fee	
L3	RCC Permit Fees	
W1	Kanapaha Rental Fee	
WO4	Kanapaha Rental Fee for Lights	
W5	Kanapaha Rent – Inspection Rate	
WO9	Kanapaha Application Fee	
W4	Kanapaha Rent – Concession Rate	
W3	Kanapaha Rent – NFP Daily Rental Fee	
W2	Kanapaha Damage Deposit	
RK	AC Fairgrounds Rental Fee	
RM	AC Fairgrounds Fire Inspection Fee	
RL	AC Fairgrounds Damage Deposit	
W00	Poe Springs Lodge Rental Fee	
W0	Poe Springs – Damage Deposit	

TOTAL AMOUNT RECEIVED: 222.00

Receipt # PW

M081201

Date: 4-6-15

Cash / Check # 1206

Visa MC Discover

FEES & CHARGES FOR:

CODE	DESCRIPTION	AMOUNT
W02	Poe Springs After Hours	
WB	Curb & Culvert Inspections (Driveway)	
WO2	Final Site Inspection Fee	
WA	Paving/Draining Inspect Fee (Subdivision)	
WO1	Right of Way Utilization Fee	222.00
WX	Record Plat Review Fee	
WV	Plat & Road Vacating	
LT	Franchise Application Fee	
WJ	Copying and Records Research	
FL	Flood Zone Information	
WK	Scrap Metal Sales PW	
WG	Traffic Sign Fees	
WQ	A/R Invoiced: Fleet Charges	
	Cell Phone Use	
ST	FL Sales Tax Collected	
MR*	Misc. Revenue	

RECEIVED BY: C. Smith





# UNIVERSAL ENGINEERING SCIENCES

Consultants in: Geotechnical Engineering • Environmental Engineering  
Construction Materials Testing • Threshold Inspection • Private Provider Inspection

May 6, 2015

LOCATIONS:  
Atlanta  
Daytona Beach  
Fort Myers  
Fort Pierce  
Gainesville  
Jacksonville  
Kissimmee  
Leesburg  
Miami  
Ocala  
Orlando (Headquarters)  
Palm Coast  
Panama City  
Pensacola  
Rockledge  
Sarasota  
Tampa  
West Palm Beach

Scentworx  
P.O. Box 769  
High Springs, FL 32655

Attention: Mr. Jose Peruyero

Reference: **Report of Geotechnical Observations**  
Scentworx - Stormwater Management System  
10908 Rachael Boulevard (Tax Parcel: 03962-0)  
Alachua, Alachua County, Florida  
UES Project No. 0230.1500024.0000 UES Report No. 1226147

Dear Mr. Peruyero:

As requested, representatives of Universal Engineering Sciences, Inc. (UES) visited the subject site on April 29, 2015 to visually observe if the two holes (pits) with hills next to them were sinkholes. Please find attached survey provided by EDA with the location of the "holes". UES has previously prepared a Report of Geotechnical Consulting Services for the subject site UES Report 1205846, dated March 6, 2015, providing geotechnical recommendations for the proposed stormwater management system.

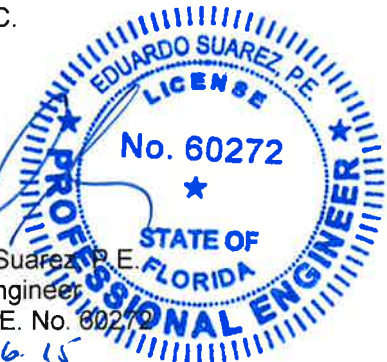
During our site visit we observed that the two areas had been excavated to depths of approximately 2 to 3 feet below the surrounding grades and the excavated material stockpiled just outside the excavated holes. We probed the bottom of the pits and did not find indication of open chimneys. It should be noted that metal pieces were observed within the vicinity of the excavated areas. Based solely on our visual observations, our knowledge of the local geology, the location and nature of the pits observed, we believe the two holes/pits in question do not appear to be karst features.

It should be noted that there was no requirement for UES to attempt to locate any manmade buried objects or identify any potential hazardous conditions. UES cannot be responsible for any buried man-made objects or environmental hazards which may be encountered during construction. Further, as indicated in our previous report, our reports do not address the potential for surface expression of deep geotechnical conditions of the subject parcel. We appreciate this opportunity to provide geotechnical consulting services for you on this project. If you should have any questions, or if we can be of further assistance, please contact our office.

Respectfully submitted,  
UNIVERSAL ENGINEERING SCIENCES, INC.  
Certificate of Authorization Number 549

Timothy Kwiatkowski, E.I  
Geotechnical Staff Engineer

Eduardo Suarez, P.E.  
Project Engineer  
Florida P.E. No. 60272  
Date: 5.6.15



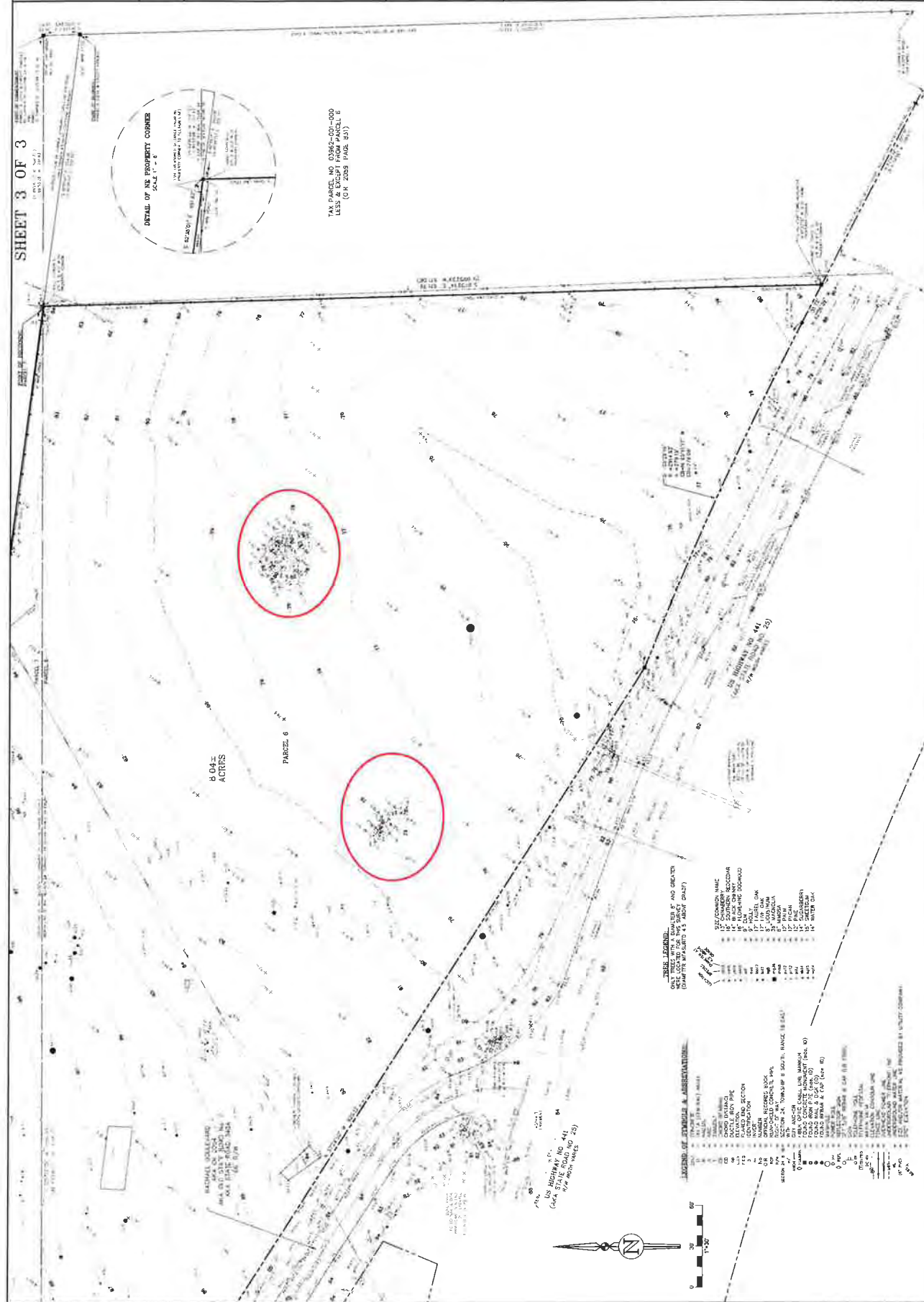
Attachments: EDA Survey

[illegible]

**CPA**

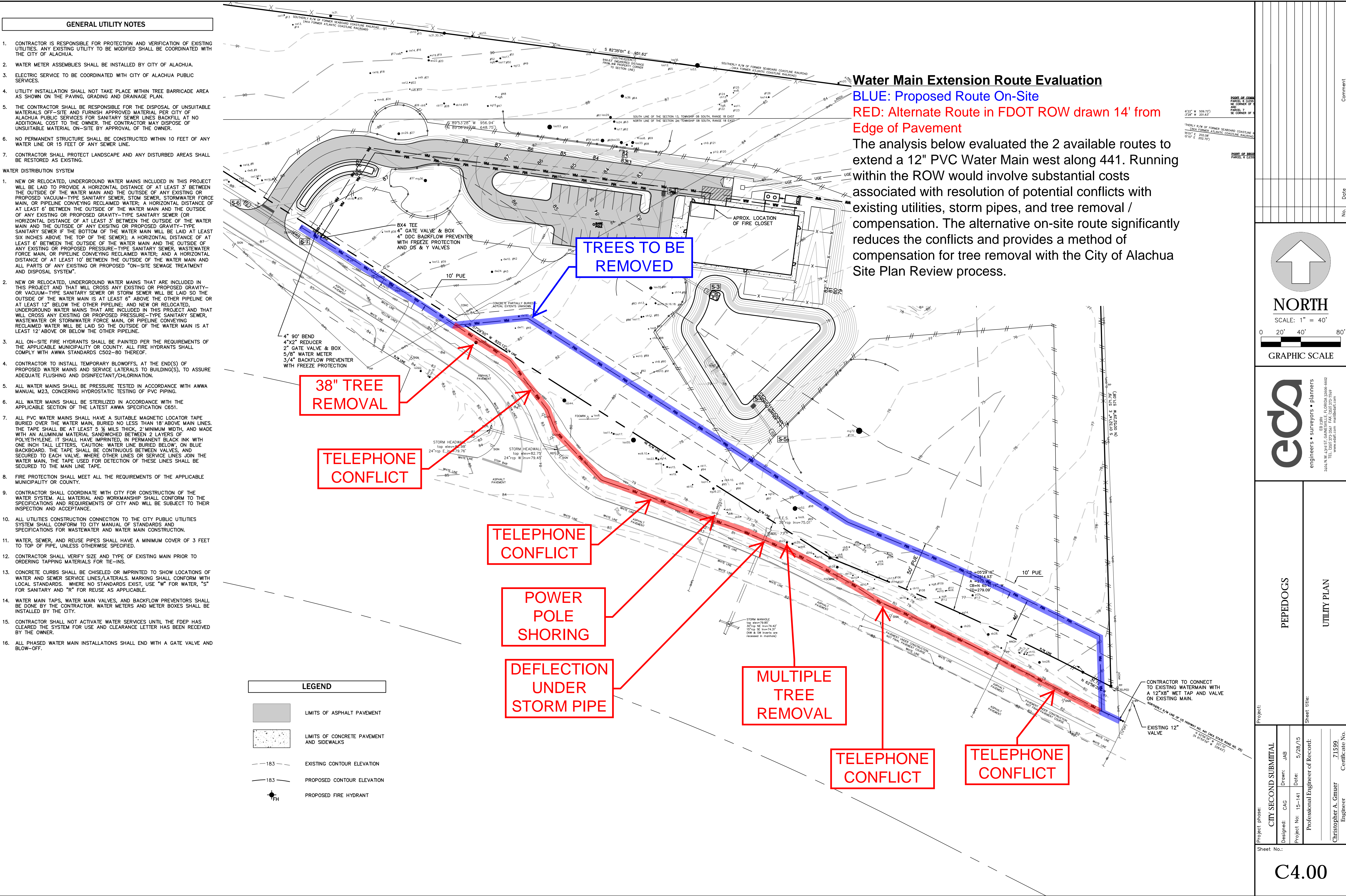
**FLOOD INSURANCE RATE MAP STATEMENT**

SHEET 3 OF 3





Plotted May 27, 2015 — 10:43:32 — engineer6





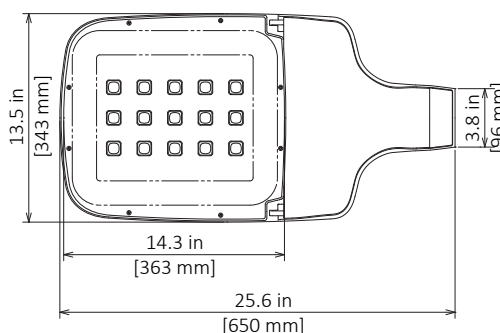
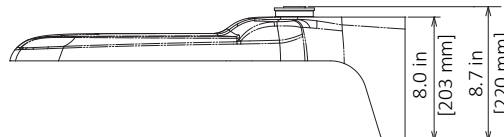
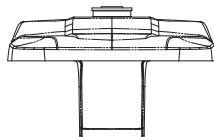
# ARIETA™13 LED Area Luminaire

## AR13

### Luminaire Data

**Weight** 15.4 lbs [7 kg]

**EPA** 0.47 ft<sup>2</sup>



### Ordering Information

Sample Catalog No. AR13 6M MV NW 5 BK 700 MSL3

Product	LED No. & Type	Voltage	Nominal Color Temperature <sup>2</sup>	Distribution	Finish <sup>3</sup>	Drive Current <sup>4</sup>	Options
<b>AR13</b>	<b>4M</b> <b>6M</b> <b>10M</b> <b>15M</b> <b>18M</b> <b>20M<sup>1</sup></b>	<b>MV</b> 120-277V <b>HV</b> 347-480V	<b>NW</b> 4000K <b>CW</b> 5000K	<b>2</b> Type 2 <b>3</b> <b>Type 3</b> <b>4</b> <b>Type 4</b> <b>5</b> Type 5	<b>BK</b> Black <b>DB</b> Dark Bronze <b>WH</b> White	<b>350</b> 350mA <b>530</b> 530mA <b>700</b> 700mA	<b>BSK</b> Bird Spider Kit <b>RPA</b> Round Pole Adaptor <b>HSS<sup>5</sup></b> House Side Shield <b>FDC<sup>6</sup></b> Fixed Drive Current <b>PCR</b> NEMA Photocontrol Receptacle <b>PCR5</b> ANSI 5-wire Photocontrol Receptacle <b>PCR7</b> ANSI 7-wire Photocontrol Receptacle <b>SC</b> PCR Shorting Cap <b>MSL7<sup>7</sup></b> Motion Sensor with L7 Lens <b>MSL3<sup>7</sup></b> Motion Sensor with L3 Lens <b>FSIR100</b> Motion Sensor Configuration Tool <b>PPS<sup>8</sup></b> Programmable Power Supply <b>ORR</b> Optics Rotated Right <b>ORL</b> Optics Rotated Left <b>WL</b> Utility Wattage Label

#### Notes:

- 20M available with MV only
- 4000K AND 5000K standard, consult factory for other color temperatures.
- Black, Dark Bronze or White standard, consult factory for other finishes.
- Factory set drive current, field adjustable standard. Refer to performance data on page 3.
- Flush mounted shield factory installed, also available for field installation. House Side Shield cuts light off at 1/2 mounting height behind luminaire.
- Non-field adjustable drive current. Specify 350mA, 530mA or 700mA setting.
- Motion Sensor available with MV only. Motion Sensor default setting dims luminaire to 50% when no motion detected for 5 minutes. Field adjustable using FSIR100 for alternate settings. See L7 or L3 Lens coverage details on page 4. Consult factory for MS specified with ANSI 5-wire or 7-wire Photocontrol Receptacle. Luminaire warranty is limited to 5 years with a Motion Sensor. PCR option is required for On/Off control using light detection.
- Consult factory for programming.



# ARIETA™13 LED Area Luminaire

## AR13

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### Luminaire Specifications

#### Housing

Die cast aluminum housing with universal mounting design allows for attachment to existing pole without redrilling for retrofit applications. Square pole mounting standard, round pole adapter option available. Meets ANSI C136.31-2001 Normal Application Vibration Standards. All hardware is stainless steel. Electrical components are accessed without tools and are mounted on removable power door. Power door features quick electrical disconnects to terminal block and LED board.

#### Light Emitting Diodes

Hi-flux/Hi-power white LEDs produce a minimum of 95% of initial intensity at 100,000 hours of life. LEDs are tested in accordance with IES LM-80 testing procedures. Mean correlated color temperature of 4000K (standard) and 70 minimum CRI. LEDs are 100% mercury and lead free.

#### Optical Systems

Micro-lens systems produce IES Type 2, Type 3, Type 4 or Type 5 distributions. Luminaire produces 0% total lumens above 90° (BUG Rating, U=0).

#### Electrical

Power Supply drive current is field adjustable (350mA, 530mA or 700mA). 0-10V dimmable power supply is standard. Power supply features a minimum power factor of .90 and <20% Total Harmonic Distortion (THD). EMC meets or exceeds FCC CFR Part 15. Transient voltage complies with ANSI C62.41 Cat. A. Integral surge protector is tested per ANSI/IEEE C62.45 procedures based on ANSI/IEEE C62.41.2 definitions for standard and optional waveforms for Location Category C High. Photocontrol receptacle or ANSI C136.41 5-wire or 7-wire dimming receptacle is optional. Photocontrol or control module is provided by others.

#### Finish

Housing receives a fade and abrasion resistant, epoxy polyester powder coat.

#### Listings/Ratings/Labels

Luminaires are UL listed for use in wet locations in the United States and Canada. Optical systems maintain an IP66 rating. Design Lights Consortium qualified 4000K product. Assembled in the United States. Wattage label complies to ANSI C136.15-2011.

#### Photometry

Luminaires are photometrically tested by certified independent testing laboratories in accordance with IES LM-79 testing procedures.

#### Warranty

10-year limited warranty is standard on all luminaires and components. 5-year limited warranty on luminaires with a motion sensor.

### Performance Data

See next page

# ARIETA™13 LED Area Luminaire

## AR13

### Performance Data

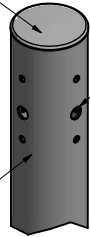
All data nominal, consult factory for IES files or LM-79 reports.

No. of LEDs & Type	Drive Current (mA)	System Wattage (W)	Type 5		
			Delivered Lumens (Lm)	Efficacy (Lm/W)	BUG Rating
4M	350*	18	2170	121	B2 U0 G1
	530	28	3080	110	B2 U0 G1
	700	37	3850	104	B2 U0 G1
6M	350	26	3120	120	B2 U0 G1
	530	39	4430	114	B2 U0 G1
	700	52	5530	106	B3 U0 G1
10M	350	43	5190	121	B3 U0 G1
	530	65	7380	114	B3 U0 G1
	700	86	9210	107	B3 U0 G2
15M	350	65	7790	120	B3 U0 G1
	530	97	11080	114	B3 U0 G2
	700	129	13818	107	B4 U0 G2
18M	350	78	9340	120	B3 U0 G2
	530	116	13290	115	B4 U0 G2
	700	155	16580	107	B4 U0 G2
20M	350*	87	10380	119	B3 U0 G2
	530	129	14770	114	B4 U0 G2
	700	172	18420	107	B4 U0 G2

Note:

\* DLC qualification at 350mA limited to 120-240V

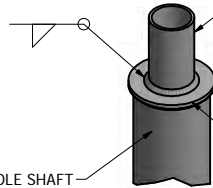
REMOVABLE CAP



DRILLED PER FIXTURE REQUIREMENTS:  
D1- DRILLED FOR 1 FIXTURE  
D2- DRILLED FOR 2 FIXTURES AT 90° OR 180°  
D3- DRILLED FOR 3 FIXTURES AT 90° OR 120°  
D4- DRILLED FOR 4 FIXTURES

POLE SHAFT

## DRILLED MOUNTING OPTIONS

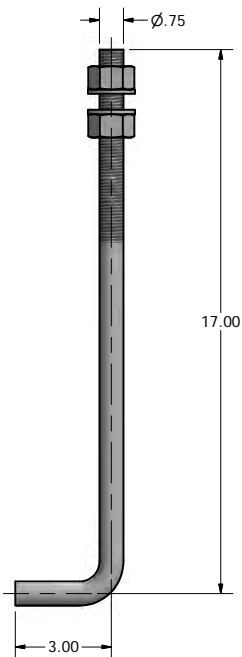


TENON MOUNT OPTIONS:  
T2- Ø2.38 OD X 4.00 LG  
T3- Ø3.00 OD X 5.00 LG  
T4- Ø4.00 OD X 6.00 LG

POLE SHAFT

.25 THK. TENON MOUNT

## TENON MOUNT OPTIONS



Ø.75 X 20.00 ANCHOR BOLT

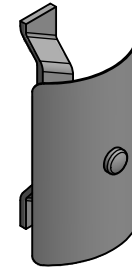
POLE HGT.  
(FT.)2.00 X 4.00  
HAND HOLE  
W/ COVER

14.00

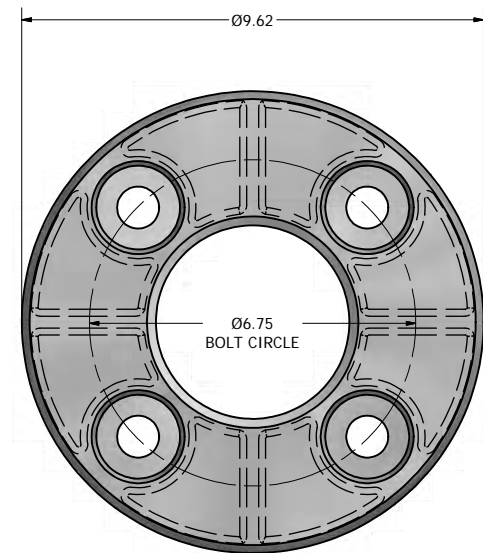
## POLE DETAIL

POLE SPECIFICATIONS							
NO.	COMPONENT	MAT'L DESIGNATION					
1.	POLE SHAFT	6063-T6					
2.	BASE PLATE	A356-T6					
3.	ANCHOR BOLTS	F1554 GR. 55					
4.	GALVANIZED HARDWARE	A153					
FINISH SPECIFICATIONS							
POLES SHALL HAVE A POLYESTER POWDER COAT FINISH IN A STANDARD COLOR.							
POLE DIMENSIONS							
POLE HGT (FT.)	TOP DIA. (IN.)	BOTTOM DIA. (IN.)	GAGE	MTG. HGT. (FT.)			
14'	3.00	4.00	.125	14'			
BASE PLATE DIMENSIONS							
BOLT CIRCLE (IN.)	BASE PLATE DIM. (IN.)	BOLT HOLE (IN.)	PLATE THK. (IN.)				
6.75	9.62	.88	.75				
ANCHOR BOLT DIMENSIONS							
ANCHOR BOLT DIA. (IN.)		ANCHOR BOLT LENGTH (IN.)					
.75		20.00					
ALLOWABLE WIND LOADING (SQ. FT.)							
WIND*	120 MPH	130 MPH	140 MPH	150 MPH	160 MPH	170 MPH	180 MPH
EPA	6.4	5.2	4.3	3.6	3.0	2.5	2.0

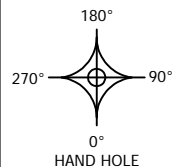
\*2010 FBC / ASCE 7 (3 SEC. GUST FACTOR)



## 2.00 X 4.00 HAND HOLE COVER



## Ø9.62 X 1.88 THK. BASE CASTING



DRAWN: L. GRUNIS	3/19/2015
CHECKED	
REVISION:	DATE:
APPROVED:	
QUOTE:	
S.O.#	
REF:	SCALE: NONE

UNITED  
LIGHTING  
STANDARDS  
A DWH COMPANY

23171 Groesbeck Hwy.  
Warren, MI 48089  
P: (586) 774-5650 | F: (586) 774-5706  
www.unitedlightingstandards.com

SOME GEOGRAPHICAL AREAS HAVE SPECIAL WIND CONDITIONS THAT CAN CREATE WIND INDUCED VIBRATIONS CAUSING A FATIGUE PROBLEM. NO METHOD HAS YET BEEN FOUND FOR PREDICTING DESTRUCTIVE LIGHTING POLE VIBRATION. THESE CONDITIONS ARE UNIQUE AND CANNOT BE GUARANTEED AGAINST, AND ARE THE RESPONSIBILITY OF A LOCAL SITE ENGINEER.		
TITLE:		
CATALOG:		
DWG NO: RTA-43141	SIZE: C	SHEET 1 OF 1

# **Holly Electric Inc.**

EC 13005429

## **J&K Academy Electrical Load**

**General Lighting 6600 sq. X 125 % X 3.5 VA = 28,875 VA**

**Unknown Receptacles 6600 X 1 VA = 6600 VA**

**Signage 1200va X 125% = 1500 VA**

**A/c 6 x 50a X 230volt = 6900 VA**

**Largest Motor 1150va X 25% = 287.5 VA**

**Total VA 44,162.5va**

**44,162.5 VA / 230volts = 192.1 VA**

**250 amps X .80% = 200 VA**

**250 amps is not a common breaker there I recommend 400 amp Service.**

**400 amp commercial Meter can with Bypass handle**

**2- 200 amp panel boards with min. 42 circuit spaces**

**Feeder sizes will 3/0 copper for the lines, and 3/0 copper for neutral because when parallel service neutral cannot be derated.**

**My apologies for taken so long I wanted to make sure I was Right.**

**Thank-you**

**Donald Hollingsworth**





STATE OF FLORIDA  
DEPARTMENT OF HEALTH  
ONSITE SEWAGE TREATMENT AND DISPOSAL  
SYSTEM  
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. \_\_\_\_\_  
DATE PAID: \_\_\_\_\_  
FEE PAID: \_\_\_\_\_  
RECEIPT #: \_\_\_\_\_

APPLICATION FOR:

[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative  
[ ] Repair [ ] Abandonment [ ] Temporary [ ] \_\_\_\_\_

APPLICANT: J & K Canine Academy, Inc.

AGENT: eda engineers~surveyors~planners, inc. TELEPHONE: (352)373-3541

MAILING ADDRESS: 2404 NW 43rd St. Gainesville, FL 32606

=====

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3) (m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

=====

PROPERTY INFORMATION

LOT: N/A BLOCK: N/A SUBDIVISION: N/A PLATTED: N/A

PROPERTY ID #: 03203-000-000, 03962-001-000 ZONING: C-1 I/M OR EQUIVALENT: [Y] / N ]

PROPERTY SIZE: 8.04 ACRES WATER SUPPLY: [ ] PRIVATE PUBLIC [ ] <=2000GPD [X] >2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [ Y / N ] DISTANCE TO SEWER: >1,320 FT

PROPERTY ADDRESS: 10908 Rachael Blvd. Alachua, FL

DIRECTIONS TO PROPERTY: Going West on US 441, turn right on Rachael Blvd (CR 2054), the project site is on the right.

BUILDING INFORMATION

[ ] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	<u>Dog Boarding</u>	<u>N/A</u>	<u>6,600</u>	<u>Commercial - Veterinary Clinic</u>
2	_____	_____	_____	_____
3	_____	_____	_____	_____
4	_____	_____	_____	_____

[ ] Floor/Equipment Drains [ ] Other (Specify) \_\_\_\_\_

SIGNATURE: [Signature] DATE: \_\_\_\_\_