

FOR PLANNING USE ONLY Case #:
Application Fee: \$
Filing Date:
Acceptance Date:
Review Type: P&Z

THE GOOD LIFE COMMUNITY

## Site Plan Application

### Reference City of Alachua Land Development Regulations Article 2.4.9

#### Α. PROJECT

- 1. Project Name: Transmission Doctor
- 2. Address of Subject Property: 14616 NW 128th Terrace, Alachua, FL 32615
- 3. Parcel ID Number(s): 03212-010-000
- 4. Existing Use of Property: automotive repair service shop
- 5. Future Land Use Map Designation : Industrial
- 6. Zoning Designation: Industrial General (IG)
- 7. Acreage: 1.040

#### APPLICANT R

- Owner (title holder) Applicant's Status Agent 1.
- Name of Applicant(s) or Contact Person(s): <u>Sergio</u> Reyes, PE Title: President 2. Company (if applicable): eda engineers-surveyors-planners, inc. Mailing address: 2404 NW 43rd Street

Mailing address.		
<sub>City:</sub> Gainesville	State: FL	ZIP: 32606
Telephone: 352-373-3541	FAX: 352-373-7249	e-mail: sreyes@edafl.com

- 3. If the applicant is agent for the property owner\*:
  - Name of Owner (title holder): M&M Properties of Alachua, LLC

Mailing Address: 14616 NW 128th Terrace

City: Alachua \_\_\_\_ State: FL \_\_\_\_\_ ZIP: 32615

\* Must provide executed Property Owner Affidavit authorizing the agent to act on behalf of the property owner.

#### C. ADDITIONAL INFORMATION

Is there any additional contact for sale of, or options to purchase, the subject property? □ Yes No

If yes, list names of all parties involved:

If yes, is the contract/option contingent or absolute? 

Contingent □ Absolute

#### **ATTACHMENTS** D.

- 1. Site Plan including but not limited to:
  - a. Name, location, owner, and designer of the proposed development.

  - c. Vicinity map indicating general location of the site and all abutting streets and properties.
    d. Complete legal description.
    e. Statement of Proposed Uses.
    f. Location of the site in relation to adjacent and Location of the site in relation to adjacent properties, including the means of ingress and egress to such properties and any screening or buffers along adjacent properties.
  - Date, north arrow, and graphic scale (not to exceed one (1) inch equal to fifty (50) feet.) g.
  - h. Area and dimensions of site.
  - i. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
  - Access and points of connection to utilities (electric, potable water, sanitary sewer, gas, etc.) j.
  - k. Location and dimensions of all existing and proposed parking areas and loading areas.
  - Location, size, and design of proposed landscaped areas (including existing trees and required Ι. landscaped buffer areas) with detail illustrating compliance with Section 6.2.2 of the Land Development Regulations.

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- m. Location and size of any lakes, ponds, canals, or other waters and waterways.
- n. Structures and major features fully dimensioned including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and floor area ratio.
- o. Location of waste receptacles and detail of waste receptacle screening.
- p. For development consisting of a nonresidential use, except for single tenant retail sales and services uses greater than or equal to 20,000 square feet in area and except for use types within the industrial services, manufacturing and production, warehouse freight and movement, wasterelated services, and wholesale sales use categories:
  - i. Architectural plans and dimension plans which demonstrate compliance with the design standards for business uses as provided in Section 6.8.2 of the LDRs, including:
    - (a) Calculation of glazing of the front façade.
    - (b) Calculation of the area of ground floor façades subject to glazing.
    - (c) Detail on the architectural plans and dimension plans depicting façade massing and/or alternatives to required façade massing.
    - (d) Sufficient plan detail and calculations of each material utilized in each facade.
- q. For development consisting of a nonresidential use where a single tenant is greater than or equal to 20.000 square feet in area:
  - i. Architectural plans and dimension plans which demonstrate compliance with the design standards for single tenant retail sales and service uses greater than or equal to 20,000 square feet in area as provided in Section 6.8.3 of the LDRs, including:
    - (a) Calculation of glazing of the façades facing streets, residential uses, and vacant residential/agricultural land.
    - (b) Calculation of the area of ground floor façades subject to glazing.
    - (c) If glazing alternatives are used, calculation of area of alternative materials used.
    - (d) Detail on the architectural plans and dimension plans depicting façade massing and/or alternatives to required façade massing.
    - (e) Color architectural plans depicting the color of all materials used in the façade.
- For development consisting of one or more of the following: Multi-family residential; Hotel; or Mobile r. Home Park:
  - i. Tabulation of gross acreage.
  - ii. Tabulation of density.
  - iii. Number of dwelling units proposed.
  - iv. Location and percent of total open space and recreation areas.
  - v. Floor area of dwelling units.
  - vi. Number of proposed parking spaces.
  - vii. Street layout.
  - viii. Layout of mobile home stands (for mobile home parks only).
  - ix. City of Alachua Public School Student Generation Form.

#### Sheet Size: 24" X 36" with 3" left margin and ½" top, bottom, and right margins

- 2. Stormwater management plan including the following:
  - a. Existing contours at one (1) foot intervals based on U.S. Coastal and Geodetic Datum.
  - b. Proposed finished floor elevation of each building site.
  - c. Existing and proposed stormwater management facilities with size and grades.d. Proposed orderly disposal of surface water runoff.

  - Centerline elevations along adjacent streets. e.
  - f. Water Management District surfacewater management Statement of proposed uses on the site plan
- 3. Fire Department Access and Water Supply: The design criteria shall be Chapter 18 of the Florida Fire Prevention Code. Plans must be on separate sealed sheets and must be prepared by a professional Fire engineer licensed in the State of Florida. Fire flow calculations must be provided for each newly constructed building. When required, fire flow calculations shall be in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office (ISO) and /or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater. All calculations must be demonstrated and provided. All calculations and specifications must be on the plans and not on separate sheets. All fire protection plans are reviewed and approved by the Alachua County Fire Marshal.
- 4. Concurrency Impact Analysis showing the impact on public facilities, including potable water, sanitary sewer, transportation, solid waste, recreation, stormwater, and public schools in accordance with Article 2.4.14 of the Land Development Regulations.
- 5. Analysis of Consistency with the City of Alachua Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies and describe in detail how the application complies with the noted Goal, Objective, or Policy.)

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a. In addition to submitting specific written information regarding your **commercial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your commercial development will comply with these standards.

Policy 1.3.d Design and performance standards

The following criteria shall apply when evaluating commercial development proposals:

- 1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
- 2. Buffering from adjacent existing/potential uses;
- 3. Open space provisions and balance of proportion between gross floor area and site size;
- 4. Adequacy of pervious surface area in terms of drainage requirements;
- 5. Placement of signage;
- 6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
- 7. Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;
- 8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
- 9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
- 10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
- 11. Commercial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.

#### For industrial project Applications:

1.

b. In addition to submitting specific written information regarding your **industrial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your industrial development will comply with these standards.

Policy 1.5.d

The City shall develop performance standards for industrial uses in order to address the following:

- Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
- 2. Buffering from adjacent existing/potential uses;
- 3. Open space provisions and balance of proportion between gross floor area and site size;
- 4. Adequacy of pervious surface area in terms of drainage requirements;
- 5. Placement of signage;
- 6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
- 7. Safety of on-site circulation patterns (patron, employee and delivery vehicles, trucks), including parking layout and drive aisles, and points of conflict;
- 8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
- 9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
- 10. Performance based zoning requirements that may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
- 11. Industrial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres by 5 acres or greater, .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.

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- 6. <u>For Site Plans for Buildings Less than 80,000 Square Feet in Area</u>: One (1) set of labels for all property owners within 400 feet of the subject property boundaries even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) and all persons/organizations registered to receive notice of development applications. <u>For Site Plans for Buildings Greater than or Equal to 80,000 Square Feet in Area</u>: Two (2) sets of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of city limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.
- 7. Neighborhood Meeting Materials, including:
  - i. Copy of the required published notice (advertisement) must be published a newspaper of general circulation, as defined in Article 10 of the City's Land Development Regulations
  - ii. Copy of written notice (letter) sent to all property owners within 400 feet and to all persons/organizations registered with the City to receive notice, and mailing labels or list of those who received written notice
  - iii. Written summary of meeting must include (1) those in attendance; (2) a summary of the issues related to the development proposal discussed; (3) comments by those in attendance about the development proposal; and, (4) any other information deemed appropriate.
- 8. Legal description with tax parcel number, separate from all other documentation on 8.5" x 11" paper.
- 9. Proof of ownership (i.e., copy of deed.)
- 10. Proof of payment of taxes.
- Environmental Resource Permit (or Letter of Exemption) from the Suwannee River Water Management District or Self-Certification for a Stormwater Management System in Uplands Serving Less than 10 Acres of Total Project Area and Less than 2 Acres of Impervious Surfaces from the Florida Department of Environmental Protection pursuant to Section 403.814(12), Florida Statutes.
- 12. If access is from a County Road, access management permit from Alachua County Public Works (or documentation providing evidence that a permit application has been submitted).
- If access is from a State Road, access management permit from Florida Department of Transportation (or documentation providing evidence that a permit application has been submitted).
- 14. Fee. Please see fee schedule for fee determination. No application shall be accepted for processing until the required application fee is paid in full by the applicant. Any necessary technical review or additional reviews of the application beyond the initial engineering review fee will be billed to the applicant at the rate of the reviewing entity. The invoice shall be paid in full prior to any legislative and/or quasi-judicial action of any kind on the petition, appeal, or development application.

<u>All 14 attachments are required for a complete application.</u> A completeness review of the application will be conducted within five (5) business days of receipt. If the application is determined to be incomplete, the application will be returned to the applicant.

I/We certify and acknowledge that the information contained herein is true and correct to the best of my/our knowledge.

Signature of Applicant		Signature of Co-applicant
SOPLAID ROUPS, RE		
Typed or printed name and title of applicant		Typed or printed name of co-applicant
State of Florido	County of	Alachua
, who is/are personally kn		1 <sup>24</sup> day of <u>March</u> , 2017 by <u>Sergio Reyes</u> who has/have produced
as identification.		Ochina Libroom
DEBBIE WALLEN Notary Public - State of Florida		Signature of Notary Public, State of Florida.
My Comm. Expires Jul 22, 2018 Commission # FF-2265 Alachua	Planning ar	nd Community Development Department ua, FL 32616 + (386) 418-6121
FUB	ox 9 + Alachi	Ja, FL 32616 + (386) 418-6121 Page



THE GOOD LIFE COMMUNITY

# Authorized Agent Affidavit

Number(s):         03212-011           1.04         PROVIDING AGENT           mes A. Markle         (if applicable):         Transidress:           (if applicable):         4616 NW 128th         128th           ::         386-462-7775         22D AGENT         1300 Reves, PE           (if applicable):         eda er         1400 Reves, PE         1400 Reves           (if applicable):         2404 NW 43rd s         1400 Reves         1400 Reves           sville         :         352-373-3541         1400 Reves         1400 Reves           'ED ACTION:         'ED ACTION:         ''''''''''''''''''''''''''''''''''''	T AUTHORIZATION mission Doctor and Auto Care, LLC h Terrace, Alachua, FL 32615 State: FAX: ngineers-surveyors-planners, inc.	Title: _President/Owner
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: 352-373-3541 ED ACTION:	FAX: <u>352-373-7249</u>	
ED ACTION:		e-mail: <u>sreyes@edafl.com</u>
	l associated infrastructure	
If for purposes of the $-\alpha$	ent permit related to the proper s application.	
		Signature of Co-applicant
	applicant	Typed or printed name of co-applicant
DRIDA	County of	LACHUA
pplication is acknow	vledged before me this $\underline{32}$	day of MARCH , 20/7 by BRODESALD
		has/have produced M624441642970
, who israte t		N
		Malina Watsu
into locaro p		Signature of Notary Public, State of FLORIDA
	ation for a developm If for purposes of thi <u>President</u> d name and title of a <u>DRIDA</u> pplication is acknow	President d name and title of applicant <u>DRIDA</u> County of <u>A</u> pplication is acknowledged before me this <u>22</u>



**City of Alachua** 

TRACI L. GRESHAM CITY MANAGER

June 7, 2017

PLANNING & COMMUNITY DEVELOPMENT DIRECTOR KATHY WINBURN, AICP

Also sent by electronic mail to <a href="mailto:sreyes@edafl.com">sreyes@edafl.com</a>

Sergio Reyes, P.E. eda, inc. 2404 NW 43<sup>rd</sup> St Gainesville, FL 32606

### RE: Approval of Alternative Landscape Plan for "Transmission Doctor and Auto Care"

Dear Mr Reyes:

The City has reviewed your request for the approval of an alternative landscape plan for the above referenced project pursuant to provisions established in Section 6.2.2(D)(9) of the LDRs. Section 6.2.2(D)(9) states, "[a]n alternative landscape plan may be used where unreasonable or impractical situations would result from the application of this section... Alternative plans may be justified from natural conditions, such as streams, natural rock formations, topography, and physical conditions related to the site. Also, the lot configuration and utility easements may justify an alternative landscape plan."

Due to the lot configuration and utility easements related to the site, your request for the approval of an alternative landscape plan is found to be consistent with the provisions of Section 6.2.2(D)(9) and is hereby approved.

If you have any questions regarding this approval, please contact the Planning & Community Development Department at 386-418-6121.

Sincerely,

acit Suns

Traci L. Gresham LDR Administrator/City Manager

c: Kathy Winburn, AICP, Planning & Community Development Director Justin Tabor, AICP, Principal Planner Adam Hall, AICP, Planner File May 18, 2017

Traci L. Gresham City Manager City of Alachua 15100 NW 142<sup>nd</sup> Terrace Alachua, FL 32616



#### **RE:** Transmission Doctor and Auto Care – Alternative Landscape Request

Dear Ms. Gresham:

We are submitting a request for the acceptance of an alternative landscape plan for the Transmission Doctor and Auto Care site plan.

This alternative landscape plan request is for the reduction in the total number of required trees and the alteration of the spacing requirements allowed under the City of Alachua Land Development Regulations, section 6.2.2(D)(9)(b)(i) within the buffers along the western, southern, and northern boundaries of the parcel due to overhead electric lines and underground utilities along these boundaries.

Due to the overhead electric running along the entire southern and western boundaries plus a portion of the northern boundary of the parcel, understory trees in place of canopy trees are being planted in accordance with Section 6.2.1(D)(4)(f). In locations where permitted, canopy trees are being placed within 7.5' of the parcel boundary to maintain a 10-foot separation between the tree and the overhead electric. We request an alternate landscape design acceptance for the following:

- Southern buffer requirement: Canopy trees be allowed to be placed approximately 2.5' outside of the 5-foot required buffer for Type B separation for fulfilling the buffer requirement.
- Western buffer requirement: Understory trees be allowed in placed of canopy trees that are required for any of the options allowed and allowance for the four canopy trees located along the fence on the vacant adjacent property to count towards the tree buffer requirements.
- Northern buffer requirement: Allowance for the four canopy trees (oaks), wood fence, and shrubs located within the buffer of the adjacent property (Sunstate Credit Union) to count towards the buffer requirement for the northern buffer as allowed per Section 6.2.2(D)(3)(d)(ii).

Given the nature of the parcel and business located on the property, a reduction in the number of trees required for interior landscaping for parking lot landscaping is requested. The parking for the parcel is unique and does not follow traditional parking. Much of the pavement area is for turning capacity for larger trucks and vehicles to safely turn around for the drop off vehicles. Placement of islands within the interior would limit the turning radius allowed within the pavement area. Larger canopy trees have been located along the parking area buffer to provide greater canopy over the pavement.

If the reductions are allowed, there are still 38 trees and 82 shrubs being proposed for this 1.04-acre parcel, which will provide significant canopy coverage within twenty years. Thank you for your consideration in this request for the reduction in the total number and spacing of trees required.

Sincerely

Larry Teague, ASLA







### **DRAINAGE DESIGN NOTES**

Prepared for: Transmission Doctor

I hereby certify that the design of the stormwater management system for the project known as Transmission Doctor meets all of the requirements and has been designed substantially in accordance with the Suwannee River Water Management District Design Criteria.





## DRAINAGE DESIGN NOTES for TRANSMISSION DOCTOR



Engineer of Record: Carol Chadwick, PE Cert. No. 82560

**Project Designer:** Carol Chadwick, PE

Submitted to: City of Alachua, FL

Submitted: March 30, 2017

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	Drainage Design Notes: Tranmission Doctor
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E. Maintenance	
F. Drainage Design	
Attachment A	Pre and Post-Development Drainage Map Aerial Photograph
Attachment B	Report of Geotechnical Consulting Services for Transmission Doctor by Universal Engineering Services (UES Report No. 1420020) dated January 16, 2017
Attachment C	Soils Map
Attachment D	FEMA map
Attachment E	Time of Concentration Calculations
Attachment F	Pre and Post-Development Conditions PONDS Model
Attachment G	WQTV Recovery Analysis PONDS Model
Attachment H	7 Day One Half Volume Recovery 30 Day Full Volume Recovery PONDS Model

### DRAINAGE DESIGN NOTES

PROJECT NAME:	Transmission Doctor
	City of Alachua, FL

#### PROJECT LOCATION:

County:	Alachua		
Sections:	14	Township:8 S_Range:	18 E
General Lo	ocation:	14616 NW 128th Terrace	
		Alachua, FL	

#### **GENERAL PROJECT INFORMATION**

Transmission Doctor is an existing commercial auto repair business. The site currently contains a shop, two pole barns and other outbuildings. The remainder of the site is pavement and sparce grass. A new 40'x80' building will be constructed. The parking area driveway will be paved. A retention basin will be constructed as part of this project. See Pre- & Post-Development plans and Aerial Photograph in Attachment A.

#### A. DRAINAGE AND DESIGN CRITERIA

1. <u>Suwannee River Water</u> <u>Management District (SRWMD)</u>

Meet requirements of 40C-42.

2. City of Alachua Land Development Code

City of Alachua Land Development Code (Article 6.9.3)

#### B. SITE SOILS INFORMATION

A subsurface investigation was completed by Universal Engineering Sciences: "Report of Geotechnical Consulting Services for Transmission Doctor" (UES Report No. 1420020) dated January 16, 2017. A copy of the report is provided in Attachment B.

#### C. EXISTING SITE CONDITIONS

The existing site contains primarily Type A some existing impervious area. The slope of the existing topography is approximately 1.8% to 2.0%. The site currently accepts very little runoff from the properties to the north and west. See Attachment C for the Web Soil Survey obtained from the NRCS.

#### D. DRAINAGE DESCRIPTION

#### 1. Pre-development Conditions

In pre-development conditions, runoff from the site trends in a southeast direction accross the site to the intersection of NW 146th Place and NW 128th Terrace. The water will stage in the road side ditches until it overtops the road intersection.

#### 2. Post-development Conditions

- a) The proposed project consists of one drainage area, which is conveyed to a proposed dry detention basin that will discharge to the southeast as in predevelopment conditions.
- b) The discharge rate and volume is controlled to be below pre-development conditions by a wier. Six inches of freeboard is provided for the dry detention system.

#### E. MAINTENANCE

See the Stormwater Pollutin Prevention Plan on sheet C0.30 of the construction plans for maintenance specifications.

#### F. DRAINAGE DESIGN

#### 1. DRAINAGE AREA

#### **Pre-Development Drainage**

Drainage Area One	Area (sf)	Area (Acres)	Curve CN
Existing Impervious	7,323	0.17	98
Open on-site	28,358	0.65	39
Basin	0	0.00	100
TOTALS	35,681	0.82	51

#### Post-Development Drainage

Drainage Area One Runoff to Southeast	Area (sf)	Area (Acres)	Curve CN	Runoff Coefficient
Existing Impervious	15,209	0.35	98	0.95
Open on-site	15,251	0.35	39	0.20
Basins	5,221	0.12	100	0.20
TOTALS	35,681	0.82	73	0.52

#### 2. BASIN STORAGE DATA

Stage	Area	Area	Volume	Vol. V1
(MSL)	(SF)	(ac)	(CF)	(AC-FT)
83.0	1957	0.04	0	0
83.5	2582	0.06	1,135	0.026
84.5	3851	0.09	4,351	0.100
85.5	5221	0.12	8,887	0.204
86.0	5544	0.13	11,579	0.266

#### 3. WATER QUALITY TREATMENT VOLUME

The basin will provide water quality treatment volume per SRWMD and City of Alachua critera. SRWMD Requirement: The minimum stormwater treatment volume shall be the runoff from the first 2.0 inches of rainfall from the design storm

City of Alachua Requirement: The first one-half inch of stormwater runoff shall be treated in an off line retention system or according to other best management practices as described in the SRWMD's Surface Water Management Permitting Manual, as amended

	SRWMD Volume (cf)	CoA Volume (cf)	Treatment Required (cf)	Treat. Vol Provided
Proposed Basin	3,090	1,487	3,090	3,470
4. BASIN GEOMETRY & DETAILS				
Basin Perimeter	368.0			
Equivalent basin width (ft)	41.6			
Equivalent basin length (ft)	142.4			
Average unsaturated area (sf)	1,957			

#### 5. SUBSURFACE INVESTIGATION INFORMATION

Based on the subsurface investigation completed by Universal Engineering Sciences: "Report of Geotechnical Consulting Services for Transmission Doctor" (UES Report No. 1420020) dated January 16, 2017, the recommendations of the soil characteristics are summarized below:

Report No. 1420020, Table 3			
Soil Boring	B-1/B-2		
Average Depth to Confining Layer, feet	3.5		
Estimated Unsaturated Vertical Infiltration Rate, feet per day	15		
Average Saturated Vertical Infiltration Rate, feet per day	23		
Estimated Horizontal Hydraulic Conductivity, feet per day	30		
Estimated Fillable Porosity, percentage	25		
Estimated Depth of Seasonal High Water Table, feet	3		

Table 3 of the original report was revised with over excavation recommendations. Over excavation of the clayey sandy soils encountered near the proposed bottom of the pond (elevation 83) will help to facilitate stormwater exfiltration and prevent non-uniform dry retention. If the basin is undercut a minimum of 5 feet and backfilled with sandy soils having a permeability rate of 20 feet per day, the soils parameters listed below may be considered. The report is included as Attachement B.

#### Report No. 1420020, Revised Table 3

Soil Boring	B-1/B-2	
Average Elevation of Confining Layer, feet	78.0	
Estimated Unsaturated Vertical Infiltration Rate, feet per day	15	
Estimated Horizontal Hydraulic Conductivity, feet per day	16	
Estimated Fillable Porosity, percentage	30	
Estimated Elevation of Seasonal High Water Table, feet	79	

#### 6. STAGE-STORAGE DISCHARGE INFORMATION

The dry detention system will have the following structures for discharge and recovery. The WQTV recovery time was analyzed using PONDS 3.2 Slug Load. In order to analyze the recovery for half of the inflow volume in seven days and the full inflow volume in thirty days, PONDS 3.2 detailed results can be seen in Attachment G and a summary below:

Discharge Structure		
Rectangul	ar Weir:	
	Elevation	85.40
7.7.7.7	Length (in.)	12.00
Rectangul	ar Weir:	7.610
	Elevation	84.20
	Length (in.)	0.72
Recovery		
WQTV Tre	atm. Vol (cf)	3,090
Time (hrs)		2.40
C. C		

#### One Half of Volume within Seven Days Recovery

Storm Event	1/2 Stage (elevation)	Recovery (days)	Back to Back Required
100yr-1hr	83.10	0.02	no
100yr-2hr	83.44	0.03	no
100yr-4hr	84.04	0.09	no
100yr-8hr	84.08	0.15	no
100yr-24hr	84.14	0.52	no
100yr-72hr	84.62	2.39	no
100yr-168hr	84.79	6.39	no
100yr-240hr	84.93	7.48	yes

The back to back storms analysis does not overtop the proposed stormwater basin.

#### **Total Volume within Thirty Days Recovery**

Storm Event	Recovery Stage (elevation)	Recovery (days)	Back to Back Required
100yr-1hr	83.00	0.00	no
100yr-2hr	83.00	1.12	no
100yr-4hr	83.00	1.20	no
100yr-8hr	83.00	2.37	no
100yr-24hr	83.00	4.04	no
100yr-3d	83.00	7.04	no
100yr-7d	83.00	12.04	no
100yr-10d	83.00	15.03	no

#### 7. STORM ROUTING RESULTS

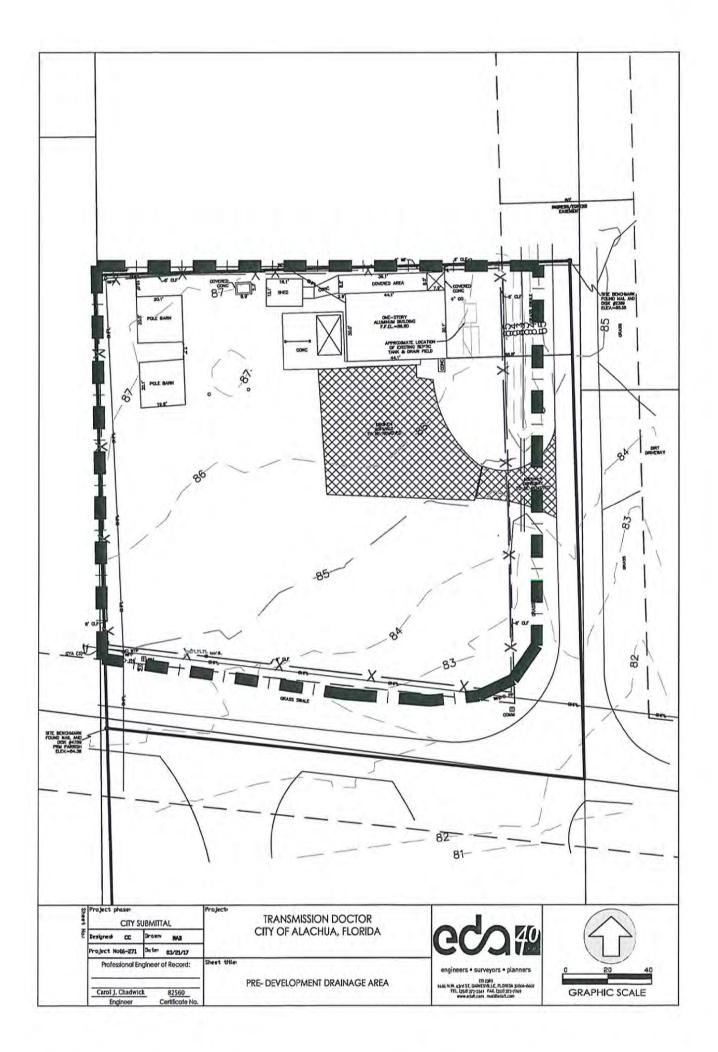
The computer program PONDS 3.2 was used to route the design storms through the proposed detention systems, as well as the pre-development conditions. The 100-yr critical storms were analyzed as required by SRWMD. The input data and storm routing results can be seen in the Attachment F.

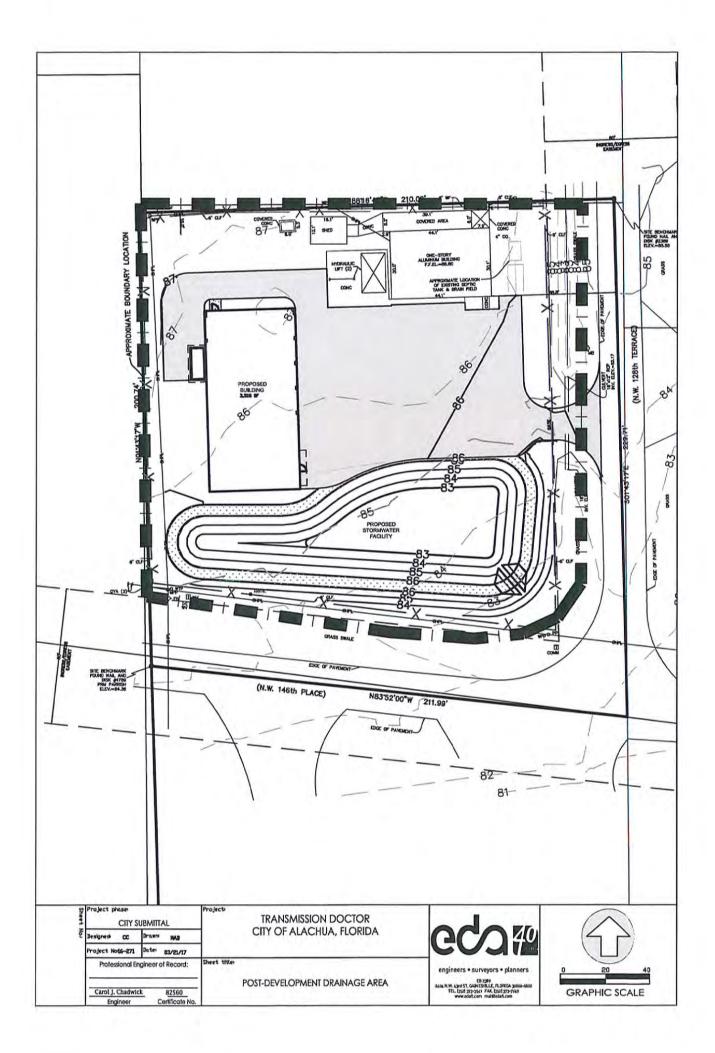
Storm	Storm Basin		ates
Event	Stage	Pre (cfs)	Post (cfs)
100 yr-1 hr	84.45	0.75	0.02
100 yr-2 hr	84.67	0.67	0.06
100 yr-4 hr	85.02	0.86	0.14
100 yr-8 hr	85.04	1.02	0.14
100 yr-24 hr	85.07	0.42	0.15
100 yr-72 hr	85.33	0.42	0.23
100 yr-168 hr	85.42	0.32	0.27
100 yr-240 hr	85.50	0.41	0.38
WQTV	84.15	0.00	0.00

## Attachment A

Pre and Post-Development Drainage Map Aerial Photograph

.







Report of Geotechnical Consulting Services for Transmission Doctor by Universal Engineering Services (UES Report No. 1420020) dated January 16, 2017



### UNIVERSAL ENGINEERING SCIENCES

Consultants in: Geotechnical Engineering • Environmental Engineering Construction Materials Testing • Threshold Inspection • Private Provider Inspection

January 16, 2017

Jim Markle 14616 NW 128<sup>th</sup> Terrace Alachua, FL 32615

Attention: Mr. Jim Markle

LOCATIONS: Atlanta Daytona Beach Fort Myers Fort Pierce Gainesville Jacksonville Kissimmee Leesburg Miami Ocala Orlando (Headquarters) Palm Coast Panama City Pensacola Rockledge Sarasota Tampa West Palm Beach

Reference: Report of Geotechnical Consulting Services Transmission Doctor Auto Care Stormwater Management System Tax Parcel: 03212-010-000 Alachua, Alachua County, Florida UES Project No. 0230.1600128.0000

UES Report No. 1420020

Dear Mr. Markle:

Universal Engineering Sciences, Inc. (UES) has completed geotechnical engineering services for the proposed new stormwater management system at the subject project in Alachua, Alachua County, Florida, as authorized in Proposal 1402504, dated November 21, 2016. This report presents the results of our subsurface field exploration, laboratory soil testing programs, and recommendations for the proposed stormwater management facility.

#### Objectives

The objectives of our geotechnical consulting services on this portion of the project have been summarized as follows:

- Explore the subsurface conditions within the proposed stormwater management area,
- Perform a series of laboratory tests on selected subsurface soil specimens to assist with engineering soil classifications and to establish the relevant soil composition and permeability characteristics,
- Classify and stratify the various soil strata encountered in the soil test borings,
- Evaluate the groundwater level in the area of exploration and make appropriate recommendations,
- Recommend appropriate subsurface soil design parameter values for design of the on-site stormwater management system.

#### **Project Information**

The subject parcel is located at tax parcel: 03212-010-000 in Alachua, Alachua County, Florida. Current site development plans include construction of stormwater management facilities. The number and locations of the borings were selected by EDA engineers-surveyors-planners, Inc. (EDA).

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By contract, our exploration was confined to the zone of soil likely to be stressed by the proposed construction. Our work did not address the potential for surface expression of deep geological conditions, such as sinkholes. This evaluation requires a more extensive range of field services than performed in this study. We will be pleased to conduct an exploration to evaluate the probable effect of the regional geology upon the proposed construction, if you desire.

#### Site Conditions

UES personnel visited the project parcel during the performance of the field portion of this geotechnical study. Our on-site observations have been summarized as follows. At the time of our exploration, the project parcel was mostly developed and clear. The surface soils were observed to be sandy and dry. Surface organic soils, surface debris, were not observed on the project site.

#### Local Geology

The general geology of Alachua County is characterized by a surface veneer of Pleistocene and Pliocene sands and sandy clays overlying the Miocene age Hawthorn Group, a highly variable mixture of interbedded quartz sands, clays, carbonates, pebbles and grains occurring in thickness of up to 150 feet. Underlying the Hawthorn Group is the upper Eocene age Ocala Formation, occurring as a uniform limestone, which is approximately 200 feet thick and overlies the Eocene age Avon Park Formation, which can be up to 500 feet thick. Both the Ocala and Hawthorn Formations dip to the northeast by approximately one degree.

The general hydrogeology of Alachua County consists of three aquifer systems: a surficial aquifer, and intermediate aquifer, and the Floridan aquifer system. The surficial aquifer exists as an unconfined water table situated over the impermeable Hawthorn Group and is usually a subdued reflection of surface topography. The intermediate aquifer system includes all rocks that collectively retard the exchange of water between the overlying surficial aquifer system and the underlying Floridan aquifer system. Water in this system is contained under confined conditions. The Floridan aquifer system is a thick carbonate sequence that functions regionally as a water-yielding hydraulic unit. The direction of shallow groundwater flow is generally toward surface water bodies. The surface of the upper Floridan Aquifer in the general project site area is estimated in the elevation range of +40 to +50 feet NGVD.

#### **General Area Soils Information**

The USDA Soil Survey of Alachua County, Florida describes the near-surface soil profile in the general project area as Arredondo soils, 0-5% slopes. Arredondo soil, 0-5% slopes, is described as nearly level to gently sloping and well drained, with a water table at a depth of more than 72 inches. Permeability is rapid in the surface and subsurface layers, and moderately slow to moderate in the loamy subsoil. Relevant engineering index properties of Arredondo soils are summarized in Table 1.

Table	e 1 – Relevant Engine	ering Index Pr	operties of	Arredonde	o Soil (3B), 0-	5% Slopes
Depth, Inches	Texture	Classification	% Passing #200 Sieve	Plasticit y Index	Shrink-swell Potential	Permeability
0-49	Fine sand	SP-SM, SM	5-15	NP	Low	6.0-20 in/hr
49-54	Loamy sand, loamy fine sand, sandy loam	SM, SM-SC	13-25	NP-7	Low	2.0-6.0 in/hr
54-86	Sandy loam, fine sandy loam, sandy clay loam	SM-SC, SC	20-40	NP-20	Low	0.2-2.0 in/hr

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#### Subsurface Exploration

The field geotechnical testing activities were started and completed on January 6, 2017. Field tests for this portion of the geotechnical study included two (2) soil test borings to a depth of 15 feet performed at the locations shown on the attached Boring Location Plan. The actual test locations shown were approximate, and were staked in the field by UES personnel using existing landmarks and site features. The boreholes were backfilled to grade upon field work completion.

<u>Standard Penetration Test (SPT) Borings:</u> Penetration tests were performed in accordance with ASTM Procedure D-1586, *Penetration Test and Split-Barrel Sampling of Soils*. This test procedure generally involves driving a 1.4-inch I.D. split-tube sampler into the soil profile in six inch increments for a minimum distance of 18 inches using a 140-pound hammer free-falling 30 inches. The total number of blows required to drive the sampler the second and third 6-inch increments is designated as the N-value, and provides an indication of in-place soil strength, density and consistency.

The results of the classification and stratification have been shown on the attached Boring Logs. It should be noted that soil conditions might vary between soil test boring locations, and between the subsurface soil strata interfaces which have been shown on the Boring Logs. The soil test boring data reflect information from the specific test locations only. This report presents an evaluation of site conditions on the basis of traditional geotechnical procedures for site characterization. The recovered samples were not examined, either visually or analytically, for chemical composition or environmental hazards.

#### Subsurface Findings

The field exploration performed for this project disclosed subsurface conditions that are consistent with the local geology and general area soils information described above. The subsurface conditions found in the soil test borings have been summarized in the attached Boring Logs and described below.

The soil test borings generally encountered sand to sand with silt [SP/SP-SM] to depths of 3 to 4 feet, followed by clayey sand to sandy clay [SC/CH] to the maximum boring termination depth of 15 feet.

The groundwater table was not encountered in any of the soil borings at the time of exploration. Fluctuations of perched groundwater level conditions on this project parcel should be expected to occur seasonally as a result of irrigation, rainfall, surface runoff, and nearby construction activities.

#### Laboratory Soil Tests

The soil samples recovered from the field exploration program were placed in containers and returned to our soils laboratory, where the Geotechnical Engineer visually classified the samples. Laboratory soil tests are performed to aid in the classification of the soils, and to help in the evaluation of engineering characteristics of the soils. Representative soil samples were selected for percent fines determination, moisture content and permeability tests. The test results have been presented on the attached Boring Logs and summarized in Table 2.

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	Tabl	e 2 – Laboratory Soil	Test Results	
Soil Boring	Sample Depth	Type of Test	Results	Soil Description
		% Finer #200	10 %	1
B-1	1 foot	Moisture Content	7 %	Sand with silt
		Permeability	20 Feet/day	
		% Finer #200	7 %	
B-2	2.5 feet	Moisture Content	5 %	Sand with silt
		Permeability	25 Feet/day	

<u>Percent Passing No. 200 Sieve</u>: Certain recovered soil samples were selected to determine the percentage of fines. In these tests the soil samples were dried and washed over a No. 200 mesh sieve. The percent of soil by weight passing the sieve was the percentage of fines or portion of the sample in the silt and clay size range. This test was conducted in accordance with ASTM Procedure D-1140, Amount of Material in Soils Finer Than the #200 Sieve.

<u>Permeability</u>: Representative soil samples were selected to determine the permeability rate of the soil. Constant head permeability tests were performed on remolded representative samples of the near surface soils from the proposed stormwater management area. These tests were conducted following the concepts outlined in ASTM D-2434, *Standard Test Method for Permeability of Granular Soils (Constant Head and Falling Head)*.

<u>Moisture Content</u>: Certain recovered soil samples were selected to determine their moisture content. The moisture content is the ratio expressed as a percentage of the weight of water in a given mass of soil to the weight of the solid particles. This test was conducted in accordance with ASTM Procedure D-2216, *Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock.* 

#### Stormwater Management System

The laboratory test data indicates that the surficial sand soils within the proposed stormwater management area for this project generally have permeability rates of 20 to 25 feet per day at the boring locations. The clayey sands encountered directly underneath the surficial clean sands would behave as a confining layer in the stormwater management area. Based upon the above findings, we recommend that you consider the soil parameters presented in Table 3 for design of the stormwater management system on the subject project site. It should be noted that the above referenced values are measured values and do not incorporate factor of safety.

Table 3 – Stormwater Management System Soil Design Parameters			
Corresponding Soil Boring Test Locations	B-1/B-2		
Average Depth to Confining Layer, feet	3.5		
Estimated Unsaturated Vertical Infiltration rate, feet per day	15		
Average Saturated Vertical Infiltration Rate, feet per day	23		
Estimated Horizontal Hydraulic Conductivity, feet per day	30		
Estimated Fillable Porosity, percentage	25		
Estimated Depth of Seasonal High Water Table feet1	3		

<sup>1</sup> Normal seasonal high water table (SHWT) will be the result of perched conditions.

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Table 3 – Stormwater Management System Soil Design Parameters		
Corresponding Soil Boring Test Locations	B-1/B-2	
Average Elevation of Confining Layer, feet	78	
Estimated Unsaturated Vertical Infiltration rate, feet per day	15	
Estimated Horizontal Hydraulic Conductivity, feet per day	16	
Estimated Fillable Porosity, percentage	25	
Estimated Elevation of Seasonal High Water Table feet <sup>1</sup>	78.5	

<sup>1</sup>Normal seasonal high water table (SHWT) will be the result of perched conditions.

<sup>2</sup> It should be noted that clayey sandy soils were encountered near the proposed bottom of the pond. Over excavation of this material beneath the pond bottom (elevation 83) will help to facilitate stormwater exfiltration and to prevent a non-uniform dry retention area. If you should undercut that basin area a minimum of 5 feet and backfill it with sandy soils having permeability rate of 20 feet per day, these soil parameters may be considered.

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#### Stormwater Management System Fill Suitability

The recovered soil samples were classified using visual and textural means, and limited laboratory testing. We offer the following *preliminary guidelines* for the use of on-site soils, such as those excavated from the proposed shallow retention areas, as fill material for the project.

Soil materials excavated and classified as fine sands to sand with silts and sand with clay (SP, SP-SM, SP-SC), with typically 12% fines or less (silt/clay fraction), may be considered suitable for use as utility trench backfill, as well as building pad and pavement subgrade structural fill, provided said materials are properly dried, placed, and compacted.

Soil materials excavated and classified as silty fine sands [SM], with typically 12% to 25% fines, may also be considered suitable for use as utility trench backfill, as well as building pad and pavement subgrade structural fill, after significant drying and some mixing with the fine sand material described above. Proper placement, proof rolling and compaction must also be performed.

Soil materials excavated and classified as clayey sand, silt or clay (SC, ML, MH, CL, and CH) and any organic-laden soils (5% or greater organics by weight) should not be reused as fill beneath buildings or pavement sections. These materials could be used in green areas, if applicable and in non-structural applications where excessive ground subsidence will not create functional or aesthetic problems. It should be noted that silt and clay materials will retain water and if used may become saturated and soft for a significant period of time following a rain event.

Soil borings for a typical geotechnical report are widely spaced and generally not sufficient for reliably detecting the presence of isolated, anomalous surface or subsurface conditions, or reliably estimating unsuitable or suitable material quantities. Accordingly, UES does not recommend relying on our boring information to negate presence of anomalous materials or for estimation of material quantities unless our contracted services *specifically* include sufficient exploration for such purpose(s) and within the report we so state that the level of exploration provided should be sufficient to detect such anomalous conditions or estimate such quantities. Therefore, UES will not be responsible for any extrapolation or use of our data by others beyond the purpose(s) for which it is applicable or intended.

#### **Report Limitations**

This Report has been prepared for the exclusive use of Jim Markle, and members of the Design/Construction Team for the specific project discussed in this Report. This Report has been prepared in accordance with generally accepted local geotechnical engineering practices; no other warranty is expressed or implied. If any changes in the design or location of the project elements as outlined in this Report are planned, the conclusions and recommendations contained in this Report shall not be considered valid unless the changes are reviewed and the conclusions modified or approved, in writing, by UES.

UES performs hydraulic conductivity tests, including the two most common, i.e., DRI and remolded laboratory permeaility testing, using generally accepted practices of the local engineering community. These common tests are the quickest and most economical for stormwater management system design. However, the user of this information is cautioned that the potential variability of results and reproducibility associated with these types of tests can be significant. It is important to note that there are many factors influencing the permeability of a soil. These factors include, but are not limited to, soil grain size, soil particle arrangement and structure, dispersion of soil fines, density, and degree of saturation, soil heterogeneity, and soil anisotropy. Also, the permeability measured by such tests may not be representative of that of the total effective aquifer thickness.

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Factors of safety can compensate for part of the inherent test limitations but the Designer must exercise judgment regarding final selection and applicability of provided soil design input parameters. Should the modeling analysis indicate marginally acceptable compliance with Water Management District design criteria, it may be advisable to perform more extensive and representative in-situ permeability testing by collecting "undisturbed" horizontal and vertical soil samples and/or installing grouted piezometers or wells for slug testing. UES can perform these field tests if desired.

Additionally, the actual exfiltration rates from the pond may be influenced by pond geometry, natural soil variability, in-situ depositional characteristics and soil density, retention volume, and groundwater mounding effects. Also, it is important to note that the upper in-situ soil zone is usually altered during the excavation and grading operations by heavy, vibrating earthwork equipment. Due to these numerous factors cited above, published literature suggests that the permeability of a soil can only be estimated to within an order of magnitude. Therefore, appropriate factors of safety should be incorporated into the design process.

#### Closure

We have enjoyed being a part of the engineering team on this project, and appreciate the opportunity to have assisted you towards its successful completion. Please contact our office if you have any questions or need further assistance.

Respectfully submitted, UNIVERSAL ENGINEERING SCIENCES, INC.

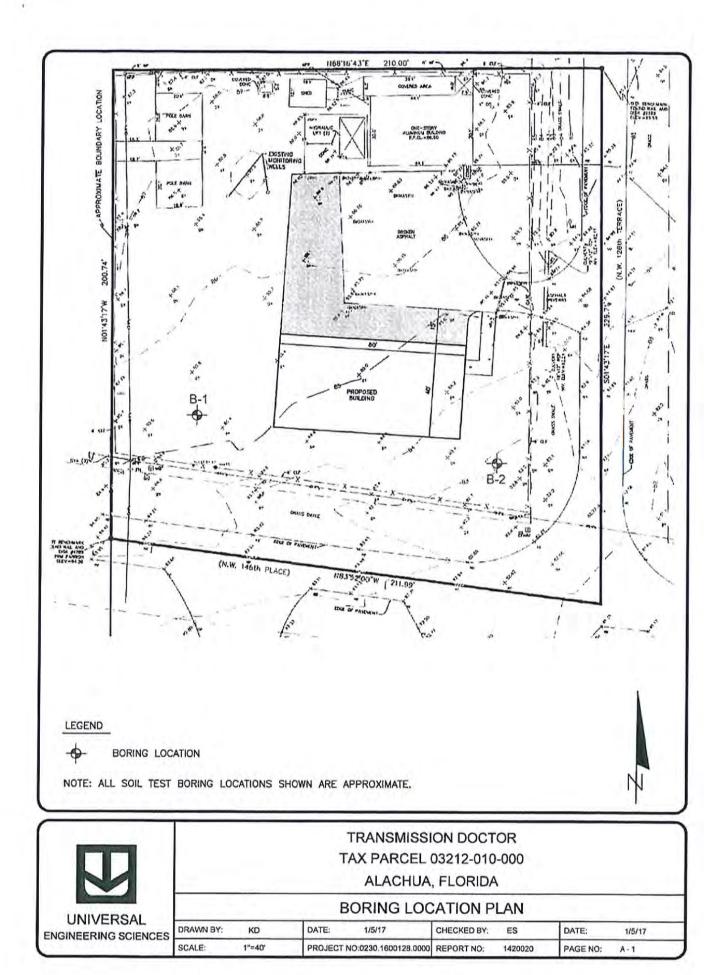
Certificate of Authorization Number 549

Timothy E. Kwiatkowski, E.I Staff Geotechnical Engineer

No. 60272 17 Eduardo Statez, P.E. Senior Geotochnical Florida P.E.N Date: 01.16

Attachments: Boring Location Plan, Boring Logs, Key to Boring Log

cc: EDA engineers-surveyors-planners, Inc. (2)





REMARKS:

# UNIVERSAL ENGINEERING SCIENCES **BORING LOG**

PROJECT NO .: 0230.1600128.0000 REPORT NO .: 1420020 PAGE: A-2

1 of 1

PROJECT: TRANSMISSION DOCTOR TAX PARCEL 03212-010-000 ALACHUA, FLORIDA CLIENT: TRANSMISSION DOCTOR AND AUTO CARE, LLC LOCATION: SEE BORING LOCATION PLAN

BORING NO: B-1

SHEET: TOWNSHIP:

RANGE:

GS ELEVATION(ft): WATER TABLE (ft): NE DATE OF READING: NA

SECTION:

EST. WSWT (ft):

DATE STARTED: 1/6/17 DATE FINISHED: 1/6/17

DRILLED BY: M. BOATRIGHT

TYPE OF SAMPLING: ASTM D-1586

DEPTH (FT.) E	BLOWS PER 6"	N VALUE	W.T.	S Y M B	DESCRIPTION	-200	МС	ATTER	RBERG	к	ORG
(FT.) PLE	INCREMENT	TALUL		B O L	BESONF HON	(%)	(%)	LL	PI	K (FT/ DAY)	CONT (%)
0				3 X C 3 X E	Dark brown SAND, with trace of silt [SP-SM]						
2-	2-1-1	2			Very loose to loose light brown and light orange SAND, with trace of silt [SP-SM]	10	7			20	
3-	WOH-1-3	4			Loose brown and orange clayey SAND [SC]	-					
5-	3-3-3	 6	) ) ((s = s))		Medium dense orange and light grav clavev	142-14			· · · · · · · ·		
6	4-6-6	12			Medium dense orange and light gray clayey SAND [SC]						
8-\ 9-\	4-4-6	10		111 111 111 111							
10	5-7-10	17			o Adampanya ang ang ang ang ang ang ang ang ang an				ldagd		antio
11					Medium dense light brown, gray and orange clayey SAND [SC], with rock						
15	4-5-5	. 10		599	clayey SAND [SČ], with rock				ététik i	********	(14) <b>-</b> - 14



REMARKS:

# UNIVERSAL ENGINEERING SCIENCES **BORING LOG**

PROJECT NO.: 0230.1600128.0000 REPORT NO .: 1420020

PAGE:

TOWNSHIP:

A-3 SHEET: 1 of 1

PROJECT: TRANSMISSION DOCTOR TAX PARCEL 03212-010-000 ALACHUA, FLORIDA CLIENT: TRANSMISSION DOCTOR AND AUTO CARE, LLC LOCATION: SEE BORING LOCATION PLAN

BORING NO: B-2

RANGE:

GS ELEVATION(ft): WATER TABLE (ft): NE DATE OF READING: NA

SECTION:

EST. WSWT (ft):

DATE STARTED: 1/6/17 DATE FINISHED: 1/6/17

DRILLED BY: M. BOATRIGHT

TYPE OF SAMPLING: ASTM D-1586

DEPTH M (FT.) L E	BLOWS PER 6" INCREMENT	N VALUE	W.T.	S Y M B	DESCRIPTION	-200 (%)	MC	ATTER	RBERG	K (FT/	ORG CONT
(FT.) P L E	INCREMENT			ÖL		(%)	(%)	LL	PI	DAY)	(%)
0					Dark brown SAND, with trace of silt [SP-SM]						
1 2	2-1-1	2		月二日 月一日 月二日	Very loose light brown and tan SAND, with trace of silt [SP-SM]	7	5			25	
з-Х										20	
4	1-1-1	2			Very loose brown and orange clayey SAND [SC]	-					
5-A	1/12"-2	2 2	a saapa				kiralı.	(1009100)	reizente		
7	4-5-9	14			Medium dense orange and light gray clayey SAND [SC]						
8-	5-6-7	13			Very stiff gray and orange sandy CLAY [CH]						
9-V	5-6-9	15			la a alar a la alar de activitar de activitar de la composición de la composición de la composición de la comp	a presente					
11-											
12-					Medium dense brown, gray and orange clayey SAND [SC]						
14	4-4-6	10	-ingin-						istini.		
					Boring Terminated at 15'						
								_			



# **KEY TO BORING LOGS**

		SYMBOLS
1	22	Number of Blows of a 140—lb Weight Falling 30 in. Required to Drive Standard Spoon One Foot
	WOR	Weight of Drill Rods
p	s	Thin-Wall Shelby Tube Undisturbed Sampler Used
	90% Rec.	Percent Core Recovery from Rock Core-Drilling Operations
	-	Sample Taken ot this Level
C	1	Sample Not Taken at this Level
-		Change in Soil Strata
70		Free Ground Water Level
-		Seasonal High Ground Water Level

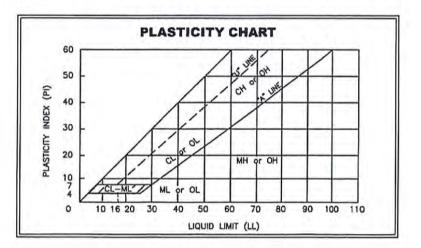
# RELATIVE DENSITY (sand-silt)

Very	loose	-	Less Than 4 Blows/Ft.
	Loose	-	4 to 10 Blows/Ft.
Medium	Dense	-	10 to 30 Blows/Ft.
	Dense	-	30 to 50 Blows/Ft.
Very	Dense	-	More Than 50 Blows/Ft.

# CONSISTANCY (clay)

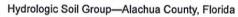
Very	Soft	-	Less	Tha	n 2	Blows/Ft.
	Soft	-	2 to	4 E	Blows	/Ft.
	Firm	-	4 to	8 E	Blows	Ft.
	Stiff	-	8 to	15	Blow	s/Ft.
Very	Stiff	-	15 to	30	Blo	ws/Ft.
	Hard	-	More	Tha	n 30	Blows/Ft.
Base	ed on	S	ofety I	lam	mer	N-Value:

M	AJOR DIVISI	ONS	GROUP SYMBOLS	TYPICAL NAMES
sieve*	, et	CLEAN GRAVELS	GW	Well—graded gravels and gravel—sand mixtures, little or no fines
8	GRAVELS 50% or more o coarse fraction retained on No. 200 sieve	CLEAN GRAVEL	GP	Poorly graded gravels and gravel—san mixtures, little or no fines
No.	GRAVELS 00% or more coarse fractic retained on No. 200 siev	ELS H ES	GM	Silty gravels, gravel—sand—silt mixture
ed on	50% Coo	GRAVELS WITH FINES	GC	Clayey gravels, gravel—sand—clay mixtures
COARSE-GRAINED SOILS 50% retained on No. 2	60% of iction 4 sieve	AN DS	SW	Well—graded sands and gravelly sands little or no fines
	SANDS More than 50% o coarse fraction passes No. 4 sie	CLEAN	SP	Poorly graded sands and gravelly sands, little or no fines
than	SAN More tha coarse passes h	ES HDS	SM	Silty sands, sand—silt mixtures
More	Mor	SANDS WITH FINES	SC	Clayey sands, sand—clay mixtures
sieve*	AYS	\$9	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands
0	siLTS AND CLAYS Liquid limit	0% or less	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays silty clays, lean clays
INED SO	SILT	ũ	OL	Organic silts and organic silty clays of low plasticity
FINE-GRAINED SOILS more passes No. 20	<b>CLAYS</b> nit	an 50%	мн	Inorganic silts, micaceous or diotomacaceous fine sands or silts, elastic silts
Ъ	or uid		СН	Inorganic clays or high plasticity, fat clays
50%	SILT	greo	ОН	Organic clays of medium to high plasticity
н	ighly organic	Soils	PT	Peat, muck and other highly organic soils



# Attachment C

Soils Map





National Cooperative Soil Survey

**Conservation Service** 

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Hydrologic Soil Group—Alachua County, Florida

terest (AOI) Area of Interest (AOI) A A/D B A/D B/D Not rated or not available ing Lines A A/D B/D B/D C/D D Not rated or not available ing Points A Not rated or not available B/D Not rated or not available B/D B/D Not rated or not available B/D Not rated or not available B/D Not rated or not available B/D B/D		The soil surveys that comprise your AOI were mapped at	CO	Marning: Soil Map may not be valid at this scale	5	Not rated or not available Enlargement of maps beyond the scale of mapping can cause	Water Features Water Features	Streams and Canals contrasting soils that could have been shown at a more detailed	Transportation Scale.	+++ Rails Diesse rely on the har scale on each man cheet for man	Interstate Highways	US Routes US Routes Source of Map: Natural Resources Conservation Service	Web Soil Survey URL: Major Roads Coordinate System: Web Mercator (FDSG:3857)	I neal Roads	maps from the vect contracted on the vect contracted are been assed on the vect mercator	Background Aerial Photography	Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.	This product is generated from the USDA-NRCS certified data as	of the version date(s) listed below.	Soil Survey Area: Alachua County, Florida	CURVES PARA PARA. VERSION 11, VERSION 21, VERSION	Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.	22, 2011	The orthophoto or other base map on which the soil lines were	compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor	shifting of map unit boundaries may be evident.		
	-111	Area of Interest (AOI)	Area of Interest (A(		Soil Rating Polygons	A	A/D	6	α	B/D	U	C/D	٥	Not rated or not av	Soil Rating Lines	×	12	8	B/D	U	C/D	۵	Soil Rating Points	A	A/D	В	B/D	

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USDA Natural Resources Conservation Service

Web Soil Survey National Cooperative Soil Survey

# Hydrologic Soil Group

Hy	drologic Soil Group— Su	mmary by Map Unit — A	lachua County, Florida (FL0	01)
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Arredondo fine sand, 0 to 5 percent slopes	A	1.0	94.0%
4	Arredondo-Urban land complex, 0 to 5 percent slopes	A	0.1	6.0%
Totals for Area of Inter	rest	-	1.1	100.0%

# Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

# **Rating Options**

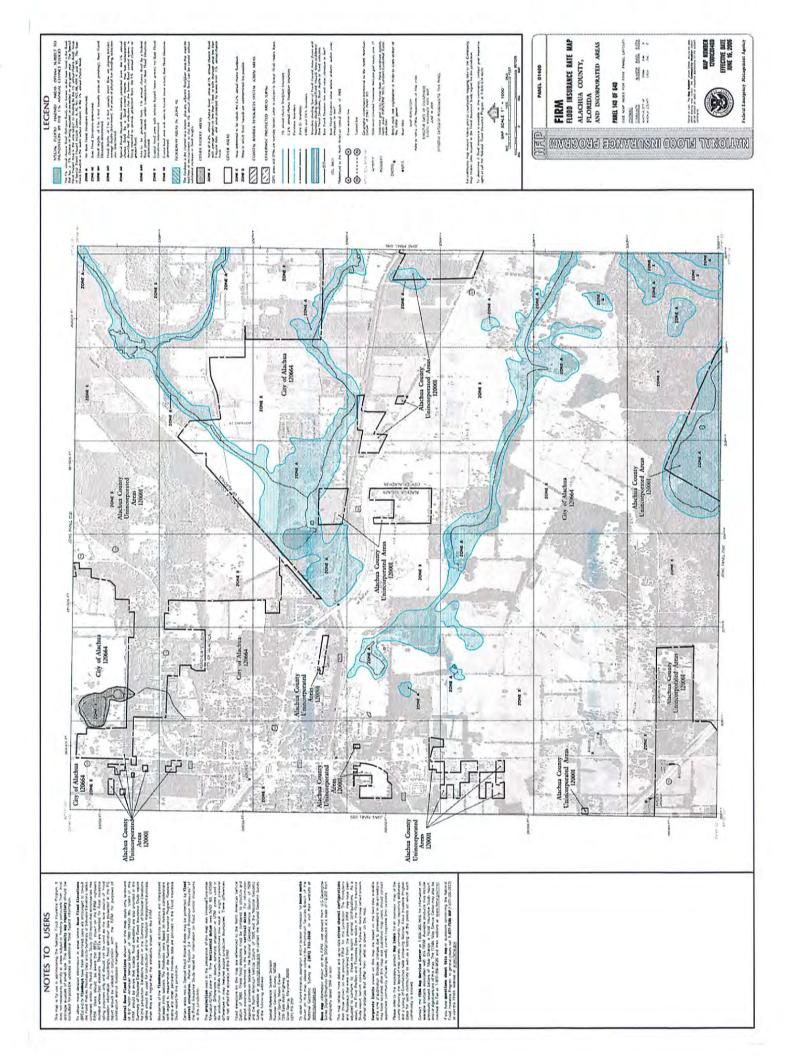
Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

# **Attachment D**

1

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FEMA map



# Attachment E

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**Time of Concentration Calculations** 

# **TR55 Tc Worksheet**

# SCS

Transmission Doctor Pre-Development

<b>Description</b>		Δ		B		<u>C</u>		<u>Totals</u>
Sheet Flow								
Manning's n-value	=	0.180		0.011		0.011		
Flow length (ft)	=	250.0		0.0		0.0		
Two-year 24-hr precip. ((in))	=	5.40		0.00		0.00		
Land slope (%)	=	1.80		0.00		0.00		
Travel Time (min)	=	18.95	+	0.00	+	0.00	Ę.	18.95
Shallow Concentrated Flow								
Flow length (ft)	=	0.00		0.00		0.00		
Watercourse slope (%)	=	0.00		0.00		0.00		
Surface description	=	Paved		Paved		Paved		
Average velocity (ft/s)	=	0.00		0.00		0.00		
Travel Time (min)								
	H	0.00	+	0.00	+	0.00	=	0.00
Channel Flow								
X sectional flow area ((sqft))	=	0.00		0.00		0.00		
Wetted perimeter ((ft))	=	0.00		0.00		0.00		
Channel slope (%)	=	0.00		0.00		0.00		
Manning's n-value	=	0.015		0.015		0.015		
Velocity (ft/s)	=	0.00		0.00		0.00		
Flow length (ft)	=	0.0		0.0		0.0		
Travel Time (min)	=	0	+	0	+	0	=	0.00
Total Travel Time, Tc								19.00 m

Hydraflow Express by Intelisolve

# **TR55 Tc Worksheet**

# SCS

Transmission Doctor Post-Development

<u>Description</u>		Δ		B		<u>C</u>		<b>Totals</b>
Sheet Flow								
Manning's n-value	=	0.011		0.011		0.011		
Flow length (ft)	=	250.0		0.0		0.0		
Two-year 24-hr precip. ((in))	=	5.40		0.00		0.00		
Land slope (%)	=	2.00		0.00		0.00		
Travel Time (min)	=	1.94	+	0.00	+	0.00	=	1.94
Shallow Concentrated Flow								
Flow length (ft)	=	0.00		0.00		0.00		
Watercourse slope (%)	=	0.00		0.00		0.00		
Surface description	=	Paved		Paved		Paved		
Average velocity (ft/s)	=	0.00		0.00		0.00		
Travel Time (min)								
	=	0.00	+	0.00	+	0.00	=	0.00
Channel Flow								
X sectional flow area ((sqft))	=	0.00		0.00		0.00		
Wetted perimeter ((ft))	=	0.00		0.00		0.00		
Channel slope (%)	=	0.00		0.00		0.00		
Manning's n-value	=	0.015		0.015		0.015		
Velocity (ft/s)	=	0.00		0.00		0.00		
Flow length (ft)	-	0.0		0.0		0.0		
Travel Time (min)	Ŧ	0	+	0	+	0	=	0.00
Total Travel Time, Tc								2.00 min

Hydraflow Express by Intelisolve

# Attachment F

Pre and Post-Development Conditions

**PONDS Model** 

# Project Data

Project Name:	Tranmission Doctor	
Simulation Description: Project Number:		
Engineer :	Carol Chadwick, P.E.	
Supervising Engineer:		
Date:	03-28-2017	

# Aquifer Data

Base Of Aquifer Elevation, [B] (ft datum):	78.00
Water Table Elevation, [WT] (ft datum):	78.50
Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day):	8.00
Fillable Porosity, [n] (%):	25.00
Unsaturated Vertical Infiltration Rate, [Iv] (ft/day):	10.0
Maximum Area For Unsaturated Infiltration, [Av] (ft <sup>2</sup> ):	5221.0

# **Geometry Data**

Equivalent Pond Length, [L] (ft):	142.0	
Equivalent Pond Width, [W] (ft):	42.0	

Ground water mound is expected to intersect the pond bottom

# Stage vs Area Data

Stage (ft datum)	Area (ft²)
83.00	1957.0
83.50	2582.0
84.50	3851.0
85.50	5221.0
86.00	5544.0

#### **Discharge Structures**

Discharge Structure #1 is active as weir

Structure Parameters

Description: weir

Weir elevation, (ft datum):	85.40
Weir coefficient:	3.13
Weir length, (ft):	1.00
Weir exponent:	1.5

Tailwater - disabled, free discharge

# Discharge Structure #2 is active as weir

Structure Parameters

Description:

Weir elevation, (ft datum):	84.2
Weir coefficient:	3.13
Weir length, (ft):	0.060
Weir exponent:	1.5

Tailwater - disabled, free discharge

Discharge Structure #3 is inactive

## Scenario Input Data

Scenario 1 :: 100yr-1hr pre

Hydrograph Type:	Inline SCS		(*)	
Modflow Routing:	Not routed			
Repetitions:	1			
Basin Area (acres)	c	0.819		
Time Of Concentratio	on (minutes) 1	16.0		
DCIA (%)	C	0.0		
Curve Number	5	50.4		
Design Rainfall Dept	h (inches) 4	1.4		
Design Rainfall Dura	tion (hours) 1	1.0		
		All a state		
Shape Factor	ι	JHG 323		
		JHG 323 FDOT 1 Hour		
Shape Factor	F			
Shape Factor Rainfall Distribution	F	DOT 1 Hour	Time After	Time After
Shape Factor Rainfall Distribution Initial ground water le Time After Storm Event	F evel (ft datum) Time After Storm Event	DOT 1 Hour 78.50 (default) Time After Storm Event	Storm Event	Storm Event
Shape Factor Rainfall Distribution Initial ground water le Time After Storm Event (days)	F evel (ft datum) Time After Storm Event (days)	DOT 1 Hour 78.50 (default) Time After Storm Event (days)	Storm Event (days)	Storm Event (days)
Shape Factor Rainfall Distribution Initial ground water le Time After Storm Event (days) 1.000	F evel (ft datum) Time After Storm Event (days) 7.000	DOT 1 Hour 78.50 (default) Time After Storm Event (days) 13.000	Storm Event (days) 19.000	Storm Event (days) 25.000
Shape Factor Rainfall Distribution Initial ground water le Time After Storm Event (days)	F evel (ft datum) Time After Storm Event (days)	DOT 1 Hour 78.50 (default) Time After Storm Event (days)	Storm Event (days)	Storm Event (days) 25.000 26.000
Shape Factor Rainfall Distribution Initial ground water le Time After Storm Event (days) 1.000 2.000	F evel (ft datum) Time After Storm Event (days) 7.000 8.000	DOT 1 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000	Storm Event (days) 19.000 20.000	Storm Event (days) 25.000 26.000 27.000
Shape Factor Rainfall Distribution Initial ground water le Time After Storm Event (days) 1.000 2.000 3.000	F evel (ft datum) Time After Storm Event (days) 7.000 8.000 9.000	DOT 1 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event

## Scenario 2 :: 100yr-2hr pre

Hydrograph Type: • Modflow Routing:	Inline SCS Not routed	
Repetitions:	1	
Basin Area (acres)		0.819
Time Of Concentratio	on (minutes)	16.0
DCIA (%)	1.4.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	0.0
Curve Number		50.4
Design Rainfall Dept	h (inches)	5.4
Design Rainfall Durat		2.0
Shape Factor		UHG 323
Rainfall Distribution		FDOT 2 Hour

Initial ground water level (ft datum) 78.50 (default)

No times after storm specified.

# Scenario Input Data (cont'd.)

Scenario 3 :: 100yr-4hr pre

•	Hydrograph Type: Modflow Routing: Repetitions:	Inline SCS Not routed 1	
	Basin Area (acres) Time Of Concentration DCIA (%) Curve Number Design Rainfall Depth Design Rainfall Duration Shape Factor Rainfall Distribution	(inches)	0.819 16.0 0.0 50.4 6.7 4.0 UHG 323 FDOT 4 Hour
	Initial ground water lev	el (ft datum)	78.50 (default)
	No times after storm s	pecified.	

Scenario 4 :: 100yr-8hr pre

Hydrograph Type: • Modflow Routing: Repetitions:	Inline SCS Not routed 1	
Basin Area (acres)		0.819
Time Of Concentration	(minutes)	16.0
DCIA (%)		0.0
Curve Number		50.4
Design Rainfall Depth	(inches)	8.0
Design Rainfall Duratio		8.0
Shape Factor		UHG 323
Rainfall Distribution		FDOT 8 Hour
Initial ground water leve	el (ft datum)	78.50 (default)

No times after storm specified.

Scenario 5 :: 100yr-24hr pre

Hydrograph Type: • Modflow Routing: Repetitions:	Inline SCS Not routed 1	
Basin Area (acres)		0.819
Time Of Concentration	n (minutes)	16.0
DCIA (%)		0.0
Curve Number		50.4
Design Rainfall Depth	(inches)	11.0
Design Rainfall Durati	on (hours)	24.0
Shape Factor		UHG 323
Rainfall Distribution		FDOT 24 Hour
Initial ground water lev	vel (ft datum)	78.50 (default)

No times after storm specified.

# Scenario Input Data (cont'd.)

Scenario 6 :: 100yr-72hr pre

Hydrograph Type: Modflow Routing: Repetitions:	Inline SCS Not routed 1	
Basin Area (acres) Time Of Concentration DCIA (%) Curve Number Design Rainfall Depth Design Rainfall Duratio Shape Factor Rainfall Distribution	(inches)	0.819 16.0 0.0 50.4 13.8 72.0 UHG 323 FDOT 72 Hour
Initial ground water lev	el (ft datum)	78.50 (default)
No times after storm s	pecified.	

## Scenario 7 :: 100yr-168hr pre

Hydrograph Type: • Modflow Routing: Repetitions:	Inline SCS Not routed 1	
Basin Area (acres)		0.819
Time Of Concentration	(minutes)	16.0
DCIA (%)	and the second second	0.0
Curve Number		50.4
Design Rainfall Depth (	inches)	16.0
Design Rainfall Duration		168.0
Shape Factor		UHG 323
Rainfall Distribution		FDOT 168 Hour
Initial ground water leve	el (ft datum)	78.50 (default)

No times after storm specified.

Scenario 8 :: 100yr-240hr pre

	Hydrograph Type:	Inline SCS	
•	Modflow Routing:	Not routed	
	Repetitions:	1	
	Basin Area (acres)		0.819
	Time Of Concentration	n (minutes)	16.0
	DCIA (%)		0.0
	Curve Number		50.4
	<b>Design Rainfall Depth</b>	(inches)	18.0
	Design Rainfall Duration		240.0
	Shape Factor	1.6.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.	UHG 323
	Rainfall Distribution		FDOT 240 Hour
	Initial ground water lev	el (ft datum)	78.50 (default)

No times after storm specified.

## Scenario Input Data (cont'd.)

Scenario 9 :: 100yr-1hr post

Hydrograph Type: Modflow Routing:	Inline SCS Routed with	infiltration		
Repetitions:	1			
Basin Area (acres)		0.819		
Time Of Concentrati	ion (minutes)	10.0		
DCIA (%)		0.0		
Curve Number		73		
Design Rainfall Dep		4.4		
Design Rainfall Dura		1.0		
Shape Factor		UHG 323		
Rainfall Distribution		FDOT 1 Hour		
		FDOT 1 Hour 78.50 (default)		
Rainfall Distribution Initial ground water I Time After			Time After	Time After
Initial ground water I	level (ft datum)	78.50 (default)	Time After Storm Event	Time After Storm Event
Initial ground water I Time After	level (ft datum) Time After	78.50 (default) Time After		
Initial ground water I Time After Storm Event	level (ft datum) Time After Storm Event	78.50 (default) Time After Storm Event	Storm Event	Storm Event (days)
Initial ground water I Time After Storm Event (days) 1.000 2.000	level (ft datum) Time After Storm Event (days)	78.50 (default) Time After Storm Event (days)	Storm Event (days)	Storm Event
Initial ground water I Time After Storm Event (days) 1.000 2.000 3.000	level (ft datum) Time After Storm Event (days) 7.000 8.000 9.000	78.50 (default) Time After Storm Event (days) 13.000	Storm Event (days) 19.000	Storm Event (days) 25.000 26.000
Initial ground water I Time After Storm Event (days) 1.000 2.000 3.000 4.000	level (ft datum) Time After Storm Event (days) 7.000 8.000 9.000 10.000	78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000 16.000	Storm Event (days) 19.000 20.000	Storm Event (days) 25.000 26.000 27.000
Initial ground water I Time After Storm Event (days) 1.000 2.000 3.000	level (ft datum) Time After Storm Event (days) 7.000 8.000 9.000	78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event (days) 25.000

# Scenario 10 :: 100yr-2hr post

Hydrograph Type: Modflow Routing:	Inline SCS Routed wit	h infiltration
Repetitions:	1	an initiation
Basin Area (acres)		0.819
Time Of Concentration	on (minutes)	10.0
DCIA (%)	27. 1 M 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.0
Curve Number		73
Design Rainfall Dept	h (inches)	5.4
Design Rainfall Dura		2.0
Shape Factor		UHG 323
<b>Rainfall Distribution</b>		FDOT 2 Hour

Initial ground water level (ft datum) 78.50 (default)

| Time After<br>Storm Event<br>(days) |
|-------------------------------------|-------------------------------------|-------------------------------------|-------------------------------------|-------------------------------------|
| 1.000                               | 7.000                               | 13.000                              | 19.000                              | 25.000                              |
| 2.000                               | 8.000                               | 14.000                              | 20.000                              | 26.000                              |
| 3.000                               | 9.000                               | 15.000                              | 21.000                              | 27.000                              |
| 4.000                               | 10.000                              | 16.000                              | 22.000                              | 28.000                              |
| 5.000                               | 11.000                              | 17.000                              | 23.000                              | 29.000                              |
| 6.000                               | 12.000                              | 18.000                              | 24.000                              | 30.000                              |

### Scenario Input Data (cont'd.)

Scenario 11 :: 100yr-4hr post

Hydrograph Type:	Inline SCS			
Modflow Routing:	Routed with	infiltration		
Repetitions:	1			
Basin Area (acres)		0.819		
Time Of Concentrati	ion (minutes)	10.0		
DCIA (%)	and the second second	0.0		
Curve Number		73		
Design Rainfall Dep	th (inches)	6.7		
Design Rainfall Dura	ation (hours)	4.0		
Ohone Fester		UHG 323		
Shape Factor		010 323		
		FDOT 4 Hour		
Shape Factor Rainfall Distribution		FDOT 4 Hour		
Rainfall Distribution		21.1.27 (T-057)		
		FDOT 4 Hour	Time After	Time After
Rainfall Distribution	level (ft datum)	FDOT 4 Hour 78.50 (default)	Time After Storm Event	
Rainfall Distribution Initial ground water I Time After	level (ft datum) Time After	FDOT 4 Hour 78.50 (default) Time After		Time After Storm Event (days)
Rainfall Distribution Initial ground water I Time After Storm Event	level (ft datum) Time After Storm Event	FDOT 4 Hour 78.50 (default) Time After Storm Event (days)	Storm Event	Storm Event (days)
Rainfall Distribution Initial ground water I Time After Storm Event (days)	level (ft datum) Time After Storm Event (days)	FDOT 4 Hour 78.50 (default) Time After Storm Event (days) 13.000	Storm Event (days)	Storm Event (days) 25.000
Rainfall Distribution Initial ground water I Time After Storm Event (days) 1.000	level (ft datum) Time After Storm Event (days) 7.000	FDOT 4 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000	Storm Event (days) 19.000	Storm Event (days) 25.000 26.000
Rainfall Distribution Initial ground water I Time After Storm Event (days) 1.000 2.000	level (ft datum) Time After Storm Event (days) 7.000 8.000	FDOT 4 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000	Storm Event (days) 25.000 26.000 27.000
Rainfall Distribution Initial ground water I Time After Storm Event (days) 1.000 2.000 3.000	level (ft datum) Time After Storm Event (days) 7.000 8.000 9.000	FDOT 4 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000 16.000	Storm Event (days) 19.000 20.000 21.000	Storm Event

#### Scenario 12 :: 100yr-8hr post

Hydrograph Type: Modflow Routing: Repetitions:	Inline SCS Routed with 1	infiltration
Basin Area (acres)		0.819
Time Of Concentratio	on (minutes)	10.0
DCIA (%)	Contraction of the second	0.0
Curve Number		73
Design Rainfall Dept	h (inches)	8.0
Design Rainfall Dura	tion (hours)	8.0
Shape Factor	and the second	UHG 323
Rainfall Distribution		FDOT 8 Hour
Initial ground water le	evel (ft datum)	78.50 (default)

Time After Time After Time After Time After Time After Storm Event Storm Event Storm Event Storm Event Storm Event (days) (days) (days) (days) (days) 1.000 7.000 13.000 19.000 25.000 2.000 3.000 8.000 14.000 20.000 26.000 27.000 9.000 15.000 21.000 4.000 10.000 16.000 22.000 28.000 5.000 11.000 17.000 23.000 29.000 6.000 12.000 18.000 24.000 30.000

### Scenario Input Data (cont'd.)

Scenario 13 :: 100yr-24hr post

Hydrograph Type: Modflow Routing:	Inline SCS Routed with	infiltration		
Repetitions:	1			
Basin Area (acres)		0.819		
Time Of Concentrat	ion (minutes)	10.0		
DCIA (%)		0.0		
Curve Number		73		
Design Rainfall Dep	oth (inches)	11.0		
Design Rainfall Dura	ation (hours)	24.0		
		11110 000		
Shape Factor		UHG 323		
Shape Factor Rainfall Distribution		FDOT 24 Hour		
Rainfall Distribution Initial ground water Time After Storm Event (days)	level (ft datum) Time After Storm Event (days)	FDOT 24 Hour 78.50 (default) Time After Storm Event (days)	Time After Storm Event (days)	(days)
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000	level (ft datum) Time After Storm Event (days) 7.000	FDOT 24 Hour 78.50 (default) Time After Storm Event (days) 13.000	Storm Event (days) 19.000	Storm Event (days) 25.000
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000	level (ft datum) Time After Storm Event (days) 7.000 8.000	FDOT 24 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000	Storm Event (days) 19.000 20.000	Storm Event (days) 25.000 26.000
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000 3.000	level (ft datum) Time After Storm Event (days) 7.000 8.000 9.000	FDOT 24 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event (days) 25.000 26.000 27.000
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000	level (ft datum) Time After Storm Event (days) 7.000 8.000	FDOT 24 Hour 78.50 (default) Time After Storm Event (days) 13.000 14.000 15.000 16.000	Storm Event (days) 19.000 20.000	Storm Event

Scenario 14 :: 100yr-72hr post

Hydrograph Type:	Inline SCS	
Modflow Routing:	Routed wit	h infiltration
Repetitions:	1	
Basin Area (acres)		0.819
Time Of Concentratio	on (minutes)	10.0
DCIA (%)		0.0
Curve Number		73
Design Rainfall Dept	h (inches)	13.8
Design Rainfall Dura		72.0
Shape Factor	and the second	UHG 323
Rainfall Distribution		FDOT 72 Hour

Initial ground water level (ft datum) 78.50 (default)

| Time After<br>Storm Event<br>(days) |
|-------------------------------------|-------------------------------------|-------------------------------------|-------------------------------------|-------------------------------------|
| 1.000                               | 7.000                               | 13.000                              | 19.000                              | 25.000                              |
| 2.000                               | 8.000                               | 14.000                              | 20.000                              | 26.000                              |
| 3.000                               | 9.000                               | 15.000                              | 21.000                              | 27.000                              |
| 4.000                               | 10.000                              | 16.000                              | 22.000                              | 28.000                              |
| 5.000                               | 11.000                              | 17.000                              | 23.000                              | 29.000                              |
| 6.000                               | 12.000                              | 18.000                              | 24.000                              | 30.000                              |
|                                     |                                     |                                     |                                     |                                     |

# Scenario Input Data (cont'd.)

Scenario 15 :: 100yr-168hr post

	Inline SCS			
Modflow Routing:	Routed with	infiltration		
Repetitions:	1			
Basin Area (acres)		0.819		
Time Of Concentrat	ion (minutes)	10.0		
DCIA (%)		0.0		
Curve Number		73		
Design Rainfall Dep	th (inches)	16.0		
Design Rainfall Dura		168.0		
Shape Factor		UHG 323		
Rainfall Distribution		FDOT 168 Hour		
Initial ground water	level (it datum)	78.50 (default)		
Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
Storm Event (days) 1.000	Storm Event (days) 7.000	Storm Event (days) 13.000	Storm Event (days) 19.000	Storm Event (days) 25.000
Storm Event (days) 1.000 2.000	Storm Event (days) 7.000 8.000	Storm Event (days) 13.000 14.000	Storm Event (days) 19.000 20.000	Storm Event (days) 25.000 26.000
Storm Event (days) 1.000 2.000 3.000	Storm Event (days) 7.000 8.000 9.000	Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event (days) 25.000 26.000 27.000
Storm Event (days) 1.000 2.000 3.000 4.000	Storm Event (days) 7.000 8.000 9.000 10.000	Storm Event (days) 13.000 14.000 15.000 16.000	Storm Event (days) 19.000 20.000 21.000 22.000	Storm Event (days) 25.000 26.000 27.000 28.000
Storm Event (days) 1.000 2.000 3.000	Storm Event (days) 7.000 8.000 9.000	Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event

#### Scenario 16 :: 100yr-240hr post

Hydrograph Type: Modflow Routing:	Inline SCS Routed with inf	iltration		
Repetitions:	1			
Basin Area (acres)	0.8	19		
Time Of Concentrat	tion (minutes) 10.	0		
DCIA (%)	0.0			
Curve Number	73			
Design Rainfall Dep		.0		
Design Rainfall Dur		0.0		
		IG 323		
Shape Factor	UH	0 323		
Shape Factor Rainfall Distribution	FD	OT 240 Hour		
Rainfall Distribution Initial ground water Time After Storm Event (days)	FD level (ft datum) 7 Time After Storm Event (days)	OT 240 Hour 8.50 (default) Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000	FD level (ft datum) 7 Time After Storm Event (days) 7.000	OT 240 Hour 8.50 (default) Time After Storm Event (days) 13.000	Storm Event	Storm Event (days)
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000	FD level (ft datum) 7 Time After Storm Event (days) 7.000 8.000	OT 240 Hour 8.50 (default) Time After Storm Event (days) 13.000 14.000	Storm Event (days) 19.000 20.000	Storm Event (days) 25.000
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000 3.000	FD level (ft datum) 7 Time After Storm Event (days) 7.000 8.000 9.000	OT 240 Hour 8.50 (default) Time After Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event (days) 25.000 26.000 27.000
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000 3.000 4.000	FD level (ft datum) 7 Time After Storm Event (days) 7.000 8.000 9.000 10.000	OT 240 Hour 8.50 (default) Time After Storm Event (days) 13.000 14.000 15.000 16.000	Storm Event (days) 19.000 20.000 21.000 22.000	Storm Event (days) 25.000 26.000 27.000 28.000
Rainfall Distribution Initial ground water Time After Storm Event (days) 1.000 2.000 3.000	FD level (ft datum) 7 Time After Storm Event (days) 7.000 8.000 9.000	OT 240 Hour 8.50 (default) Time After Storm Event (days) 13.000 14.000 15.000	Storm Event (days) 19.000 20.000 21.000	Storm Event (days) 25.000 26.000 27.000

# Scenario Input Data (cont'd.)

Scenario 17 :: 3090 ft3 slug load

Hydrograph Type:	Slug Load
Modflow Routing:	Routed with infiltration

Treatment Volume (ft<sup>3</sup>) 3090

Initial ground water level (ft datum) 78.50 (default)

Time After Storm Event (days)	Time After Storm Event (days)
0.100	2.000
0.250	2.500
0.500	3.000
1.000	3.500
1.500	4.000

## Scenario 20 :: 100yr-240hr post back to back

Hydrograph Type: Modflow Routing:	Inline SCS Routed wit	h infiltration
Repetitions:	2	
Basin Area (acres)		0.819
Time Of Concentration (minutes)		10.0
DCIA (%)		0.0
Curve Number		73
Design Rainfall Depth	n (inches)	18.0
<b>Design Rainfall Durat</b>	tion (hours)	240.0
Shape Factor		UHG 323
Rainfall Distribution		FDOT 240 Hour

#### Initial ground water level (ft datum) 78.50 (default)

| Time After<br>Storm Event<br>(days) |
|-------------------------------------|-------------------------------------|-------------------------------------|-------------------------------------|-------------------------------------|
| 1.000                               | 7.000                               | 13.000                              | 19.000                              | 25.000                              |
| 2.000                               | 8.000                               | 14.000                              | 20.000                              | 26.000                              |
| 3.000                               | 9.000                               | 15.000                              | 21.000                              | 27.000                              |
| 4.000                               | 10.000                              | 16.000                              | 22.000                              | 28.000                              |
| 5.000                               | 11.000                              | 17.000                              | 23.000                              | 29.000                              |
| 6.000                               | 12.000                              | 18.000                              | 24.000                              | 30.000                              |
|                                     |                                     |                                     |                                     |                                     |

# Attachment G

WQTV Recovery Analysis

PONDS Model

## Sort-By-Category Report

Scenarios Considered: 1 to 17, 20

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#### Stage - Maximum

Rank	Scenario Number	Maximum Stage (ft datum)	Time (hours)	Description	_
1	20	85.530	1144.933	100yr-240hr post back to back	
2	16	85.501	184.067	100yr-240hr post	
23	15	85.423	160.089	100yr-168hr post	
4	14	85.331	64.044	100yr-72hr post	
5	13	85.069	21.067	100yr-24hr post	
6	12	85.039	5.133	100yr-8hr post	
7	11	85.018	3.289	100yr-4hr post	
4 5 6 7 8 9	10	84.665	1.911	100yr-2hr post	
9	9	84.451	1.156	100yr-1hr post	
10	17	84.152	0.002	3090 ft <sup>3</sup> slug load	
11	1	Not Available	Not Available	100yr-1hr pre	
12	2 3	Not Available	Not Available	100yr-2hr pre	
13	3	Not Available	Not Available	100yr-4hr pre	
14	4	Not Available	Not Available	100yr-8hr pre	
15	5	Not Available	Not Available	100yr-24hr pre	
16	6	Not Available	Not Available	100yr-72hr pre	
17	7	Not Available	Not Available	100yr-168hr pre	
18	8	Not Available	Not Available	100yr-240hr pre	

#### Discharge - Rate - Maximum Positive

Rank	Scenario Number	Maximum Positive Discharge Rate (ft <sup>3</sup> /s)	Time (hours)	Description
1	4	1.0211	4.053	100yr-8hr pre
2	3	0.8557	2.596	100yr-4hr pre
3	1	0.7504	0.818	100yr-1hr pre
4	2	0.6709	0.996	100yr-2hr pre
5	20	0.4343	1144.933	100yr-240hr post back to back
2 3 4 5 6 7		0.4247	12.053	100yr-24hr pre
7	5 6	0.4156	59.982	100yr-72hr pre
8 9	8	0.4125	184.000	100yr-240hr pre
9	16	0.3784	184.067	100yr-240hr post
10	7	0.3182	160.000	100yr-168hr pre
11	15	0.2653	160.089	100yr-168hr post
12	14	0.2259	64.044	100yr-72hr post
13	13	0.1522	21.067	100yr-24hr post
14	12	0.1444	5.133	100yr-8hr post
15	11	0.1388	3.289	100yr-4hr post
16	10	0.0596	1.911	100yr-2hr post
17	9	0.0236	1.156	100yr-1hr post
18	17	None	. N.A.	3090 ft <sup>3</sup> slug load

# Sort-By-Category Report (cont'd.)

## Discharge - Cumulative Volume - Maximum Positive

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft <sup>3</sup> )	Time (hours)	Description
1	20	33823.3	1225.822	100yr-240hr post back to back
23	8	29622.5	241.316	100yr-240hr pre
3	7	24593.6	169.387	100yr-168hr pre
4	6	19261.3	73.351	100yr-72hr pre
5	16	14325.5	264.911	100yr-240hr post
6	15	13029.6	216.911	100yr-168hr post
7	5	12975.6	25.351	100yr-24hr pre
8	14	11215.9	96.911	100yr-72hr post
9	13	8074.5	48.911	100yr-24hr post
10	4	6834.9	9.387	100yr-8hr pre
11	3	4613.9	5.404	100yr-4hr pre
12	2	2645.9	3.413	100yr-2hr pre
13	12	2377.2	32.911	100yr-8hr post
14	1	1436.8	2.418	100yr-1hr pre
15	11	836.6	4.911	100yr-4hr post
16	10	258.3	2.911	100yr-2hr post
17	9	47.0	1.889	100yr-1hr post
18	17	None	N.A.	3090 ft <sup>3</sup> slug load

# Attachment H

7 Day One Half Volume Recovery 30 Day Full Volume Recovery

**PONDS Model** 

Detailed Results :: Scenario 9 :: 100yr-1hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>a</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flov Type
0.000	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	N.A
0.022	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.067	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.089	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.111	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.133	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	UU
0.156 0.178	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0 0.0	Ŭ
0.200	0.0000	0.0000	78.500 78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.222	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.244	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.267	0.0000	0.0000	78.500	0.00020	0.00000	0.0	0.0	0.0	ŭ
0.289	0.0008	0.0000	78.500	0.00239	0.00000	0.0	0.0	0.0	ŭ
0.311	0.0080	0.0000	78.500	0.01288	0.00000	0.4	0.4	0.0	Ŭ
0.333	0.0347	0.0000	78.502	0.04405	0.00000	2.1	2.1	0.0	Ū
0.356	0.0987	0.0000	78.506	0.11258	0.00000	7.4	7.4	0.0	Ŭ
0.378	0.2181	0.0000	78.515	0.19337	0.00000	20.1	20.1	0.0	Ũ
0.400	0.3981	0.0000	83.003	0.22915	0.00000	44.8	38.4	0.0	U/F
0.422	0.6296	0.0000	83.015	0.23154	0.00000	85.9	56.8	0.0	U/F
0.444	0.8993	0.0000	83.036	0.23540	0.00000	147.0	75.4	0.0	U/F
0.467	1.1937	0.0000	83.068	0.24080	0.00000	230.7	94.4	0.0	U/F
0.489	1.5019	0.0000	83.111	0.24770	0.00000	338.6	113.9	0.0	U/F
0.511	1.8094	0.0000	83.164	0.25598	0.00000	471.0	134.1	0.0	U/I
0.533	2.1013	0.0000	83.225	0.26542	0.00000	627.4	154.9	0.0	U/F
0.556	2.3653	0.0000	83.294	0.27576	0.00000	806.1	176.5	0.0	U/I
0.578	2.5923	0.0000	83.368	0.28675	0.00000	1004.4	199.0	0.0	U/I
0.600	2.7888	0.0000	83.446	0.29820	0.00000	1219.7	222.4	0.0	U/I
0.622	2.9472	0.0000	83.526	0.30995	0.00000	1449.1	246.7	0.0	U/I
0.644	3.0601	0.0000	83.607	0.32173	0.00000	1689.4	272.0	0.0	U/F
0.667	3.1122	0.0000	83,686	0.33325	0.00000	1936.3	298.2	0.0	U/I
0.689	3.0973	0.0000	83.764	0.34432	0.00000	2184.7	325.3	0.0	U/I
0.711	3.0562	0.0000	83.837	0.35486	0.00000	2430.8	353.3	0.0	U/F
0.733	2.9948	0.0000	83.907	0.36482	0.00000	2672.8	382.1	0.0	U/F
0.756	2.9180	0.0000	83.973	0.37420	0.00000	2909.3	411.7	0.0	U/I
0.778	2.8198	0.0000	84.035	0.38298	0.00000	3138.9	442.0	0.0	U/F
0.800	2.7104	0.0000	84.092	0.39115	0.00000	3360.1	473.0	0.0	U/F
0.822	2.5873	0.0000	84.146	0.39870	0.00000	3572.0	504.6	0.0	U/F
0.844	2.4445	0.0000	84.195	0.40559	0.00000	3773.3	536.7	0.0	U/I
0.867	2.2686	0.0000	84.240	0.41176	0.00149	3961.8	569.5	0.1	U/F
0.889	2.0518	0.0000	84.279	0.41714	0.00418	4134.6	602.6	0.3	U/F
0.911	1.8367	0.0000	84.313	0.42174	0.00713	4290.1	636.2	0.7	U/F
0.933	1.6313	0.0000	84.342	0.42562	0.01001	4428.9	670.1	1.4	U/F
0.956	1.4517	0.0000	84.366	0.42887	0.01267	4552.2	704.3	2.3	U/I
0.978	1.2932	0.0000	84.386	0.43158	0.01504	4662.0	738.7	3.4	U/F
1.000	1.1533	0.0000	84.402	0.43381	0.01711	4759.8	773.3	4.7	U/F
1.022	1.0291	0.0000	84.416	0.43563	0.01887	4847.1	808.1	6.2	U/F
1.044	0.9175	0.0000	84.427	0.43708	0.02034	4925.0	843.0	7.7	U/F
1.067	0.8156	0.0000	84.436	0.43820	0.02151	4994.3	878.1	9.4	U/I
1.089	0.7199	0.0000	84.442	0.43902	0.02241	5055.7	913.2	11.2	U/I
1.111	0.6329	0.0000	84.447	0.43955	0.02304	5109.8	948.3	13.0	U/I
1.133	0.5515	0.0000	84.450	0.43984	0.02342	5157.2	983.5	14.8	U/F
1.156	0.4813	0.0000	84.451	0.43990	0.02358	5198.5	1018.7	16.7	U/I
1.178	0.4190	0.0000	84.450	0.43977	0.02354	5234.5	1053.9	18.6	U/I
1.200	0.3631	0.0000	84.449	0.43946	0.02333	5265.8	1089.0	20.5	U/I
1.222	0.3132	0.0000	84.446	0.43900	0.02296	5292.9	1124.2	22.3	U/I
1.244	0.2688	0.0000	84.443	0.43841 0.43769	0.02246	5316.2	1159.3	24.1	U/I
1.267			84.438		0.02184	5336.1	1194.3	25.9	U/I
1.289	0.1951	0.0000	84.433	0.43687 0.43596	0.02112 0.02033	5353.1 5367.5	1229.3 1264.2	27.6 29.3	U/I U/I
1.311	0.1643		84.427						
1.333	0.1371	0.0000	84.421	0.43497	0.01946	5379.5	1299.1 1222 P	30.9	U/I
1.356 1.378	0.1138 0.0940	0.0000	84.414	0.43390 0.43278	0.01854	5389.6 5397.9	1333.8 1368.5	32.4 33.9	U/I
1.378	0.0940		84.406				1368.5	35.2	U/I
1.400		0.0000	84.398	0.43161	0.01659	5404.7			
1.422	0.0619	0.0000	84.390	0.43039	0.01558	5410.2	1437.6	36.5	U/I
1.444	0.0494	0.0000	84.382	0.42914	0.01456	5414.7	1471.9	37.7	U/I
1.467	0.0391	0.0000	84.373	0.42785	0.01354	5418.2	1506.2	38.8	U/I
1.489	0.0306	0.0000	84.364	0.42655	0.01252	5421.0	1540.4	39.9	U/I
1.511	0.0234	0.0000	84.355	0.42522	0.01151	5423.2	1574.5	40.8	U/I
1.533	0.0176	0.0000	84.346	0.42388	0.01052	5424.8	1608.4	41.7	U/F
1.556	0.0130	0.0000	84.337	0.42253	0.00955	5426.0	1642.3	42.5	U/F
1.578	0.0094 0.0068	0.0000	84.328	0.42116 0.41980	0.00860	5426.9	1676.0 1709.7	43.3 43.9	U/F
	0.0000	0.0000	84.319	0.41980	0.00768	5427.6	1709.7	43.8	0/1

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (fl <sup>a</sup> )	Flow Type
1.644	0.0032	0.0000	84.300	0.41705	0.00595	5428.3	1776.6	45.0	U/P
1.667	0.0022	0.0000	84.291	0.41568	0.00513	5428.6	1809.9	45.4	U/P
1.689	0.0016	0.0000	84.281	0.41430	0.00436	5428.7	1843.1	45.8	U/P
1.711	0.0011	0.0000	84.272	0.41293	0.00363	5428.8	1876.2	46.1	U/P
1.733	0.0008	0.0000	84.263	0.41156	0.00295	5428.9	1909.2	46.4	U/P
1.756	0.0005	0.0000	84.253	0.41019	0.00232	5428.9	1942.1	46.6	U/P
1.778	0.0003	0.0000	84.244	0.40882	0.00174	5429.0	1974.8	46.8	U/P
1.800	0.0002	0.0000	84.235	0.40745	0.00122	5429.0	2007.5	46.9	U/P
1.822	0.0001	0.0000	84.226	0.40609	0.00077	5429.0	2007.5		U/P
1.844	0.0000	0.0000	84.216	0.40472	0.00039	5429.0	2072.4	47.0	U/P
1.867	0.0000	0.0000	84.207	0.40336				47.0	
1.889	0.0000	0.0000	84.198		0.00011	5429.0	2104.8	47.0	U/P
1.911	0.0000	0.0000		0.40199	0.00000	5429.0	2137.0	47.0	U/P
25.911	0.0000		84.188	0.40094	0.00000	5429.0	2169.1	47.0	U/P
49.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
49.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
73.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
97.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
121.911	0.0000	0.0000			- iiii	5429.0	5382.0	47.0	dry
145.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
169.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
193.911	0.0000	0.0000			(and and	5429.0	5382.0	47.0	dry
217.911	0.0000	0.0000		and the second		5429.0	5382.0	47.0	dry
241.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
265.911	0.0000	0.0000		_		5429.0	5382.0	47.0	dry
289.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
313.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
337.911	0.0000	0.0000		· · · · ·		5429.0	5382.0	47.0	dry
361.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
385.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
409.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
433.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
457.911	0.0000	0.0000			1000	5429.0	5382.0	47.0	dry
481.911	0.0000	0.0000		****	C and a	5429.0	5382.0	47.0	dry
505.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
529.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
553.911	0.0000	0.0000			_	5429.0	5382.0	47.0	dry
577.911	0.0000	0.0000	5.5			5429.0	5382.0	47.0	dry
601,911	0.0000	0.0000		-		5429.0	5382.0	47.0	dry
625.911	0.0000	0.0000		- <u>a</u>		5429.0	5382.0	47.0	
649.911	0.0000	0.0000							dry
673.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
697.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
721.911	0.0000	0.0000				5429.0	5382.0	47.0	dry
/21.911	0.0000	0.0000		(manual)		5429.0	5382.0	47.0	dry

:: Scenario 10 :: 100yr-2hr post

**Detailed Results** 

Elapsed Inflow Outside Stage Infiltration Overflow Cumulative Cumulative Cumulative Discharge Time Rate Recharge Elevation Rate Inflow Infiltration Discharge Flow (hours) (ft?/s) (ft/day) (ft datum) (ft3/s) (ft<sup>3</sup>/s) Volume (ft<sup>a</sup>) Volume (ft<sup>3</sup>) Volume (fl3) Type 0.0000 0.000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 N.A. 0.0000 0.00000 0.00000 0.022 78,500 0.0 0.0 0.0 U 0.044 0.0000 78,500 0.0 0.0 0.0 UU 0.067 0.0000 0.0000 0.00000 0.00000 78.500 0.0 0.0 0.0 0.0000 0.00000 ũ 0.089 0.0000 78.500 0.00000 0.0 0.0 0.0 0.0000 0.0000 78.500 0.00000 0.00000 0.0 ũ 0.111 0.0 0.0 0.133 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 Ũ 0.156 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 ũ 0.178 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 U 0.200 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 υ 0.00000 0.0000 78.500 0.00000 0.0 0.0 0.0 00000 0.0000 0.244 78.500 0.00015 0.0 0.0 0.0 0.267 0.0006 0.0000 78.500 0.00128 0.00000 0.0 0.0 0.0 0.289 0.0039 0.0000 78.500 0.00549 0.00000 0.2 0.2 0.0 0.311 0.0135 0.0000 78.501 0.01614 0.00000 0.9 0.9 0.0 0.333 0.0336 0.0000 78.502 0.03706 0.00000 0.0 ũ 2.8 2.8 0.356 0.0675 0.0000 78.505 0.07076 0.00000 6.8 6.8 0.0 U 0.378 0.1145 0.0000 78.511 0.11731 0.00000 14.1 14.1 0.0 U 0.1728 0.2436 0.400 0.0000 0.00000 78.520 0.17593 25.6 25.6 0.0 U 0.21820 78.532 42.3 42.3 0.0 U 0.00000 0.444 0.3281 0.0000 83,002 0.22863 65.1 60.5 U/P 0.0 0.467 0.4289 0.0000 0.00000 83.008 0.22982 U/P 95.4 78.8 0.0 0.5473 0.0000 0.00000 0.0 0.489 83.019 0.23168 134.5 97.3 U/P 0.511 0.6748 0.0000 83.034 0.00000 115.9 0.0 0.23423 183.3 U/P 0.533 0.8072 0.0000 83.054 0.23748 0.00000 0.0 U/P 242.6 134.8 0.556 0.9401 0.0000 83.079 0.24141 0.00000 312.5 153.9 0.0 U/P 1.0717 0.578 0.0000 83.108 0.24597 0.00000 393.0 173.4 0.0 U/P 0.0000 0.25112 0.00000 0.600 1.2011 83.142 483.9 193.3 0.0 U/P 0.0000 0.622 1.3347 0.0 U/P 83.180 585.3 213.6 0.644 1.4778 0.0000 83.221 0.00000 0.26315 697.8 U/P 234.4 1.6371 0.0000 83.267 0.00000 0.667 0.27008 822.4 255.7 0.0 U/P 0.689 1.8156 0.0000 0.00000 0.0 83.317 0.27764 960.5 277.6 U/P 0.711 1.9973 0.0000 83.371 0.28581 0.00000 300.1 0.0 U/P 1113.1 0.733 2.1767 2.3469 0.0000 83.430 0.29452 0.00000 1280.0 323.3 0.0 U/P 0.756 0.0000 83.492 0.30377 0.00000 1461.0 347.2 0.0 U/P 2.5075 0.31347 0.00000 0.0000 83.557 1655.1 371.9 0.0 U/P 0.800 2.6589 0.0000 1861.8 83.624 397.4 0.0 U/P 0.822 2.7729 0.0000 83.693 0.33353 0.00000 U/P 2079.1 423.7 0.0 2.8308 0.0000 0.34348 0.0 0.844 83.761 0.00000 2303.2 450.7 U/P 0.867 2.8084 0.0000 83.828 0.35302 0.00000 0.0 U/P 2528.8 478.6 0.889 2.6976 0.0000 83.891 0.36193 0.00000 2749.0 507.2 0.0 U/P 0.911 2.5632 0.0000 83.949 0.37011 0.00000 2959.5 536.5 0.0 U/P 0.933 2.4292 0.0000 84.003 0.37762 0.00000 3159.2 566.4 0.0 U/P 2.3143 2.2213 0.0000 84.052 0.38455 0.00000 3348.9 596.9 0.0 U/P 0.978 0.0000 84.097 0.39101 0.00000 3530.3 U/P 628.0 0.0 1.000 2.1464 0.0000 0.39708 0.00000 3705.0 0.0 U/P 84.140 659.5 1.022 2.0792 0.0000 84.180 0.40280 0.00000 3874.1 691.5 U/P 0.0 1.044 2.0101 0.0000 84.217 0.40818 0.00043 4037.6 723.9 U/P 1.067 1.9289 0.0000 84.253 0.41320 4195.2 756.8 U/P 0.00228 1.089 1.8316 0.0000 84.286 0.41784 0.00471 4345.6 0.4 U/P 790.1 1.111 1.7361 0.0000 84.316 0.42209 0.00741 4488.3 823.7 0.9 U/P 1.133 1.6426 0.0000 84.344 0.42600 0.01021 4623.5 857.6 1.6 U/P 0.0000 1.5641 1.156 84.369 0.42959 0.01304 4751.7 891.8 2.5 U/P 1.4964 0.01585 1.178 84.392 0.43292 4874.1 3.7 U/P 926.3 1.200 1.4380 0.0000 0.43602 4991.5 5.1 6.7 84.414 0.01861 961.1 U/P 1.3856 0.0000 84.434 0.43892 0.02133 1.222 5104.5 996.1 U/P 1.3363 0.0000 1.244 84.454 0.44163 0.02397 5213.3 1031.3 U/P 8.5 1.267 1.2870 0.0000 84.471 0.44415 0.02653 5318.3 1066.8 10.5 U/P 1.289 1.2365 0.0000 84.488 0.44657 0.02899 5419.2 1102.4 U/P 12.7 1.311 1.1891 0.0000 84.503 0.44889 0.03133 5516.2 1138.2 15.1 U/P 1.1446 1.333 0.0000 84.517 0.45107 0.03355 5609.6 1174.2 17.7 U/P 1.1064 5699.6 1.356 0.0000 0.45309 0.03566 1210.4 1246.7 20.5 23.4 U/P U/P 84.530 1.378 1.0735 0.0000 84.543 5786.8 0.0000 26.5 1.400 1.0445 84.554 0.45676 0.03958 5871.5 1283.2 U/P 1.0171 0.0000 1.422 84.565 0.45843 0.04141 5954.0 1319.8 29.7 U/P 1.444 0.9895 0.0000 84.575 0.46000 0.04316 6034.3 1356.5 33.1 U/P 1.467 0.9597 0.0000 84.585 0.46145 0.04480 1393.4 36.6 U/P 6112.2 1.489 0.9264 0.0000 84.593 0.46279 0.04634 6187.7 1430.4 40.3 U/P 1.511 0.8945 0.0000 84.601 0.46401 0.04777 6260.5 1467.4 44.1 U/P 1.533 0.8651 0.0000 84.609 0.46513 0.04909 6330.9 1504.6 47.9 U/P 1.556 0.8403 0.46616 1541.8 1579.2 0.0000 84.616 0.05031 6399.1 51.9 11/P 0.0000 0.8196 1.578 84.622 0.05144 6465.5 U/P 56.0 1.600 0.8023 0.0000 84.628 0.46801 0.05251 6530.4 1616.6 60.1 U/P 1.622 0.7858 0.0000 84.633 0.46884 0.05351 6593.9 1654.1 U/P 64.4

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flow Type
1.644	0.7682	0.0000	84.638	0.46961	0.05444	6656.1	1691.6	68.7	U/P
1.667	0.7474	0.0000	84.643	0.47030	0.05530	6716.7	1729.2	73.1	U/P
1.689	0.7224	0.0000	84.647	0.47092	0.05608	6775.5	1766.9	77.5	U/P
1.711	0.6977	0.0000	84.650	0.47145	0.05676	6832.3	1804.5	82.1	U/P
1.733	0.6745	0.0000	84.653	0.47191	0.05734	6887.2	1842.3	86.6	U/P
1.756	0.6547	0.0000	84.656	0.47230	0.05785	6940.4	1880.1	91.2	U/P
1.778	0.6380	0.0000	84.658	0.47264	0.05828	6992.1	1917.9	95.9	U/P
1.800 1.822	0.6239	0.0000	84.660 84.662	0.47293 0.47317	0.05866 0.05898	7042.5 7091.9	1955.7 1993.5	100.5 105.3	U/P U/P
1.844	0.5942	0.0000	84.663	0.47336	0.05924	7140.1	2031.4	110.0	U/P
1.867	0.5745	0.0000	84.664	0.47348	0.05943	7186.8	2069.3	114.7	U/P
1.889	0.5502	0.0000	84.665	0.47354	0.05954	7231.8	2107.1	119.5	U/P
1.911	0.5259	0.0000	84.665	0.47352	0.05956	7274.8	2145.0	124.3	U/P
1.933	0.5029	0.0000	84.665	0.47343	0.05949	7316.0	2182.9	129.0	U/P
1.956	0.4831	0.0000	84.664	0.47327	0.05934	7355.4	2220.8	133.8	U/P
1.978	0.4664	0.0000	84.663	0.47307	0.05912	7393.4	2258.6	138.5	U/P
2.000	0.4521	0.0000	84.661	0.47282	0.05885	7430.1	2296.5	143.2	U/P
2.022	0.4356	0.0000	84.660	0.47252	0.05852	7465.7	2334.3	147.9	U/P
2.044 2.067	0.4137 0.3824	0.0000	84.658 84.655	0.47215 0.47168	0.05812 0.05763	7499.6	2372.1	152.6 157.2	U/P U/P
2.087	0.3404	0.0000	84.652	0.47108	0.05700	7531.5 7560.4	2409.8 2447.5	161.8	U/P
2.111	0.2975	0.0000	84.648	0.47037	0.05623	7585.9	2485.2	166.3	U/P
2.133	0.2568	0.0000	84.643	0.46953	0.05531	7608.1	2522.8	170.8	U/P
2.156	0.2217	0.0000	84.637	0.46859	0.05425	7627.2	2560.3	175.2	U/P
2.178	0.1919	0.0000	84.631	0.46755	0.05309	7643.7	2597.8	179.5	U/P
2.200	0.1666	0.0000	84.624	0.46644	0.05185	7658.1	2635.1	183.7	U/P
2.222	0.1451	0.0000	84.617	0.46527	0.05053	7670.6	2672.4	187.8	U/P
2.244	0.1267	0.0000	84.609	0.46404	0.04917	7681.4	2709.6	191.7	U/P
2.267	0.1108	0.0000	84.601	0.46277	0.04775	7690.9	2746.6	195.6	U/P
2.289	0.0968	0.0000	84.593	0.46146	0.04631	7699.2	2783.6	199.4	U/P
2.311 2.333	0.0845 0.0731	0.0000	84.585 84.576	0.46011 0.45873	0.04484 0.04335	7706.5 7712.8	2820.5 2857.2	203.0 206.6	U/P U/P
2.356	0.0635	0.0000	84.568	0.45732	0.04184	7718.3	2893.9	210.0	U/P
2.378	0.0551	0.0000	84.559	0.45590	0.04033	7723.0	2930.4	213.3	U/P
2.400	0.0475	0.0000	84.550	0.45445	0.03882	7727.1	2966.8	216.4	U/P
2.422	0.0409	0.0000	84,540	0.45299	0.03730	7730.6	3003.1	219.5	U/P
2.444	0.0350	0.0000	84.531	0.45151	0.03579	7733.7	3039.3	222.4	U/P
2.467	0.0298	0.0000	84.522	0.45002	0.03429	7736.3	3075.4	225.2	U/P
2.489	0.0252	0.0000	84.512	0.44852	0.03280	7738.5	3111.3	227.9	U/P
2.511	0.0212	0.0000	84.503	0.44701	0.03132	7740.3	3147.1	230.4	U/P
2.533	0.0176	0.0000	84.493	0.44553	0.02986	7741.9	3182.8	232.9	U/P
2.556 2.578	0.0146 0.0120	0.0000 0.0000	84.484 84.474	0.44410	0.02841 0.02699	7743.2 7744.2	3218.4 3253.9	235.2 237.4	U/P U/P
2.600	0.0097	0.0000	84.465	0.44269 0.44128	0.02558	7745.1	3289.2	239.5	U/P
2.622	0.0078	0.0000	84.455	0.43986	0.02420	7745.8	3324.5	241.5	U/P
2.644	0.0061	0.0000	84.446	0.43844	0.02285	7746.3	3359.6	243.4	U/P
2.667	0.0047	0.0000	84.436	0.43702	0.02152	7746.8	3394.6	245.2	U/P
2.689	0.0036	0.0000	84.426	0.43561	0.02021	7747.1	3429.5	246.9	U/P
2.711	0.0026	0.0000	84.417	0.43419	0.01894	7747.4	3464.3	248.4	U/P
2.733	0.0018	0.0000	84.407	0.43277	0.01769	7747.5	3499.0	249.9	U/P
2.756	0.0012	0.0000	84.397	0.43136	0.01647	7747.7	3533.6	251.3	U/P
2.778	0.0008	0.0000	84.388	0.42995	0.01529	7747.7	3568.0	252.5	U/P
2.800	0.0004	0.0000	84.378	0.42854	0.01413	7747.8	3602.4	253.7	U/P
2.822	0.0002	0.0000	84.369	0.42713	0.01301	7747.8	3636.6	254.8	U/P
2.844 2.867	0.0001	0.0000	84.359 84.350	0.42573	0.01192 0.01087	7747.8	3670.7 3704.7	255.8 256.7	U/P U/P
2.889	0.0000	0.0000	84.340	0.42293	0.00985	7747.8	3738.6	257.5	U/P
2.911	0.0000	0.0000	84.331	0.42188	0.00887	7747.8	3772.4	258.3	U/P
26.911	0.0000	0.0000	81.456	0.02151	0.00000	7747.8	7489.6	258.3	U/S
50.911	0.0000	0.0000	80.950	0.00000	0.00000	7747.8	7489.6	258.3	S
74.911	0.0000	0.0000	80.667	0.00000	0.00000	7747.8	7489.6	258.3	S
98.911	0.0000	0.0000	80.476	0.00000	0.00000	7747.8	7489.6	258.3	S
122.911	0.0000	0.0000	80.332	0.00000	0.00000	7747.8	7489.6	258.3	555
146.911	0.0000	0.0000	80.219	0.00000	0.00000	7747.8	7489.6	258.3	S
170.911	0.0000	0.0000	80.126	0.00000	0.00000	7747.8	7489.6	258.3	SS
194.911	0.0000	0.0000	80.048	0.00000	0.00000	7747.8	7489.6	258.3	S
218.911	0.0000	0.0000	79.981	0.00000	0.00000	7747.8	7489.6	258.3	S
242.911 266.911	0.0000	0.0000	79.922 79.871	0.00000	0.00000	7747.8 7747.8	7489.6 7489.6	258.3 258.3	S
290.911	0.0000	0.0000	79.824	0.00000	0.00000	7747.8	7489.6	258.3	S
314.911	0.0000	0.0000	79.782	0.00000	0.00000	7747.8	7489.6	258.3	S
338.911	0.0000	0.0000	79.744	0.00000	0.00000	7747.8	7489.6	258.3	s
362.911	0.0000	0.0000	79.709	0.00000	0.00000	7747.8	7489.6	258.3	S
386.911	0.0000	0.0000	79.677	0.00000	0.00000	7747.8	7489.6	258.3	S

## Detailed Results (cont,d.) :: Scenario 10 :: 100yr-2hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft <sup>3</sup> /s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flow Type
410.911	0.0000	0.0000	79.648	0.00000	0.00000	7747.8	7489.6	258.3	S
434.911	0.0000	0.0000	79.620	0.00000	0.00000	7747.8	7489.6	258.3	S
458.911	0.0000	0.0000	79.595	0.00000	0.00000	7747.8	7489.6	258.3	S
482.911	0.0000	0.0000	79.571	0.00000	0.00000	7747.8	7489.6	258.3	
506.911	0.0000	0.0000	79.548	0.00000	0.00000	7747.8	7489.6	258.3	SS
530.911	0.0000	0.0000	79.527	0.00000	0.00000	7747.8	7489.6	258.3	S
554.911	0.0000	0.0000	79.507	0.00000	0.00000	7747.8	7489.6	258.3	S
578.911	0.0000	0.0000	79.488	0.00000	0.00000	7747.8	7489.6	258.3	S
602.911	0.0000	0.0000	79.470	0.00000	0.00000	7747.8	7489.6	258.3	S
626.911	0.0000	0.0000	79.454	0.00000	0.00000	7747.8	7489.6	258.3	S
650.911	0.0000	0.0000	79.437	0.00000	0.00000	7747.8	7489.6	258.3	S
674.911	0.0000	0.0000	79.422	0.00000	0.00000	7747.B	7489.6	258.3	S
698.911	0.0000	0.0000	79.407	0.00000	0.00000	7747.8	7489.6	258.3	S
722.911	0.0000	0.0000	79.393		++	7747.8	7489.6	258.3	N.A.

Detailed Results :: Scenario 11 :: 100yr-4hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (fl/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>2</sup> )	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flow Type
0.000	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.022	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.067	0.0000 0.0000	0.0000	78.500 78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.005	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.133	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.156	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.178	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.200	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.222 0.244	0.0000	0.0000 0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.244	0.0000	0.0000	78.500 78.500	0.00000	0.00000	0.0 0.0	0.0	0.0	UU
0.289	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.311	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ũ
0.333	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.356	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.378	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.400	0.0000	0.0000	78.500 78.500	0.00000	0.00000	0.0	0.0	0.0	UU
0.444	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.467	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.489	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.511	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.533	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.556 0.578	0.0000	0.0000	78.500 78.500	0.00000	0.00000	0.0	0.0	0.0	UU
0.600	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
0.622	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.644	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.667	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	υ
0.689	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.711	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.733	0.0000	0.0000	78.500 78.500	0.00000	0.00000	0.0 0.0	0.0	0.0	U
0.778	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.800	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
0.822	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	U
0.844	0.0000	0.0000	78.500	0.00001	0.00000	0.0	0.0	0.0	U
0.867	0.0001 0.0004	0.0000	78.500 78.500	0.00013 0.00059	0.00000	0.0	0.0	0.0	U
0.911	0.0015	0.0000	78.500	0.00176	0.00000	0.0	0.1	0.0	ŭ
0.933	0.0037	0.0000	78.500	0.00413	0.00000	0.3	0.3	0.0	ŭ
0.956	0.0076	0.0000	78,501	0.00803	0.00000	0.8	0.8	0.0	U
0.978	0.0132	0.0000	78.501	0.01354	0.00000	1.6	1.6	0.0	U
1.000	0.0202	0.0000	78.502	0.02085	0.00000	2.9	2.9	0.0	U
1.022	0.0298	0.0000	78.504 78.506	0.03068 0.04415	0.00000	4.9 7.8	4.9 7.8	0.0	UU
1.067	0.0610	0.0000	78.509	0.06251	0.00000	12.0	12.0	0.0	ŭ
1.089	0.0852	0.0000	78.514	0.08615	0.00000	17.8	17.8	0.0	ŭ
1.111	0.1133	0.0000	78.520	0.11400	0.00000	25.8	25.8	0.0	U
1.133	0.1443	0.0000	78.528	0.14469	0.00000	36.1	36.1	0.0	U
1.156	0.1769	0.0000	78.537	0.17714	0.00000	48.9	48.9	0.0	U
1.178 1.200	0.2105 0.2447	0.0000	78.549 78.563	0.21065 0.22783	0.00000	64.4 82.6	64.4 82.6	0.0	Ŭ
1.222	0.2793	0.0000	83.001	0.22825	0.00000	103.6	100.9	0.0	U/P
1.244	0.3139	0.0000	83.004	0.22875	0.00000	127.3	119.1	0.0	U/P
1.267	0.3486	0.0000	83.008	0.22945	0.00000	153.8	137.5	0.0	U/P
1.289	0.3832	0.0000	83.014	0.23035	0.00000	183.1	155.9	0.0	U/P
1.311	0.4175	0.0000	83.021	0.23143	0.00000	215.1	174.3	0.0	U/P
1.333 1.356	0.4516 0.4852	0.0000	83.029 83.038	0.23270 0.23415	0.00000	249.9 287.4	192.9 211.6	0.0	U/P U/P
1.356	0.5184	0.0000	83.049	0.23415	0.00000	327.5	230.4	0.0	U/P
1.400	0.5510	0.0000	83.061	0.23755	0.00000	370.3	249.3	0.0	U/P
1.422	0.5831	0.0000	83.074	0.23949	0.00000	415.6	268.4	0.0	U/P
1.444	0.6145	0.0000	83.087	0.24158	0.00000	463.5	287.6	0.0	U/P
1.467	0.6454	0.0000	83.102	0.24381	0.00000	513.9	307.0	0.0	U/P
1.489 1.511	0.6755	0.0000	83.118 83.135	0.24617	0.00000	566.8	326.6	0.0	U/P
1.511	0.7076	0.0000	83.135 83.153	0.24868 0.25136	0.00000	622.1 680.2	346.4 366.4	0.0	U/P U/P
1.556	0.7950	0.0000	83.172	0.25425	0.00000	741.9	386.6	0.0	U/P
1.578	0.8581	0.0000	83.193	0.25743	0.00000	808.0	407.1	0.0	U/P
1.600	0.9304	0.0000	83.216	0.26091	0.00000	879.5	427.8	0.0	U/P
1.622	1.0043	0.0000	83.241	0.26469	0.00000	956.9	448.8	0.0	U/P

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Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft²/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flo Typ
1.644	1.0756	0.0000	83.268	0.26876	0.00000	1040.1	470.2	0.0	U/
1.667	1.1421	0.0000	83.297	0.27307	0.00000	1128.8	491.8	0.0	U/
1.689	1.2041	0.0000	83.328	0.27759	0.00000	1222.7	513.8	0.0	U/
1.711	1.2618	0.0000	83.360	0.28230	0.00000	1321.3	536.2	0.0	U/
1.733	1.3158	0.0000	83.393	0.28716	0.00000	1424.4	559.0	0.0	U/
1.756	1.3668	0.0000	83.427	0.29215	0.00000	1531.7	582.2	0.0	U/
1.778	1.4151	0.0000	83.462	0.29725	0.00000	1643.0	605.8	0.0	U/
1.800		0.0000	83.498						
	1.4611			0.30250	0.00000	1758.1	629.7	0.0	U/
1.822	1.5052	0.0000	83.534	0.30786	0.00000	1876.7	654.2	0.0	U/
1.844	1.5475	0.0000	83.570	0.31328	0.00000	1998.8	679.0	0.0	U/
1.867	1.5876	0.0000	83.608	0.31876	0.00000	2124.2	704.3	0.0	U/
1.889	1.6260	0.0000	83.645	0.32429	0.00000	2252.8	730.0	0.0	U/
1.911	1.6627	0.0000	83.683	0.32985	0.00000	2384.3	756.2	0.0	U/
1.933	1.6980	0.0000	83.721	0.33545	0.00000	2518.7	782.8	0.0	U/
1.956	1.7318	0.0000	83.759	0.34106	0.00000	2655.9	809.8	0.0	U/
1.978	1.7642	0.0000	83.797	0.34669	0.00000	2795.8	837.4	0.0	U/
2.000	1.7954	0.0000	83.836	0.35232	0.00000	2938.2	865.3	0.0	U/
2.022	1.8204	0.0000	83.874	0.35793	0.00000	3082.8	893.7	0.0	U/
2.044	1.8363	0.0000	83.912	0.36349	0.00000	3229.1	922.6	0.0	U/
2.067	1.8388	0.0000	83.950	0.36896	0.00000	3376.1	951.9	0.0	U/
2.089	1.8267	0.0000	83.987	0.37429	0.00000	3522.7	981.6	0.0	U/
2.111	1.8111	0.0000	84.022	0.37947	0.00000	3668.2	1011.8	0.0	U/
2.133	1.7956	0.0000	84.057	0.38452	0.00000	3812.5	1042.3	0.0	U/
2.156	1.7836	0.0000	84.091	0.38944	0.00000	3955.6	1073.3	0.0	U/
2.178	1.7755	0.0000	84.124	0.39425	0.00000	4098.0	1104.6	0.0	U/
2.200									
2.200	1.7706	0.0000	84.157	0.39898	0.00000	4239.8	1136.4	0.0	0/
2.222	1.7683	0.0000	84.188	0.40362	0.00000	4381.4	1168.5	0.0	U/
2.244	1.7679	0.0000	84.220	0.40819	0.00052	4522.8	1201.0	0.0	U/
2.267	1.7688	0.0000	84.251	0.41270	0.00214	4664.3	1233.8	0.1	U/
2.289	1.7707	0.0000	84.281	0.41714	0.00433	4805.9	1267.0	0.4	U/
2.311	1.7734	0.0000	84.311	0.42152	0.00695	4947.6	1300.5	0.8	U/
2.333	1.7760	0.0000	84.341	0.42585	0.00991	5089.6	1334.4	1.5	U/
2.356	1.7798	0.0000	84.370	0.43012	0.01314	5231.8	1368.7	2.4	U/
2.378	1.7839	0.0000	84.399	0.43434	0.01663	5374.4	1403.2	3.6	U/
2.400	1.7884	0.0000	84.427	0.43851	0.02033	5517.3	1438.2	5.1	U/
2.422	1.7931	0.0000	84.455	0.44263	0.02422	5660.5	1473.4	6.9	U/
2.444	1.7982	0.0000	84.483	0.44682	0.02828	5804.2	1509.0	9.0	U/
2.467	1.8034	0.0000	84.511	0.45112	0.03250	5948.3	1544.9	11.4	U/
2.489	1.8089	0.0000	84.538	0.45541	0.03685	6092.8	1581.2	14.2	U/
2.511	1.8108	0.0000	84.564	0.45963	0.04133	6237.5	1617.8	17.3	U/
2.533	1.8028	0.0000	84.591	0.46376	0.04588	6382.1	1654.7	20.8	U/
2.556	1.7793	0.0000	84.616	0.46775	0.05046	6525.4	1692.0	24.7	U/
2.578	1.7361	0.0000	84.641	0.47155	0.05499	6666.0	1729.5	28.9	- Ŭ/
2.600	1.6808	0.0000	84.664	0.47513	0.05938	6802.7	1767.4	33.5	U/
2.622	1.6250	0.0000	84.686	0.47849	0.06362	6934.9	1805.6	38.4	Ŭ/
2.644	1.5743	0.0000	84.706	0.48165	0.06768	7062.9	1844.0	43.6	Ŭ/
									U/
2.667	1.5314	0.0000	84.726	0.48464	0.07158	7187.1	1882.6	49.2	
2.689	1.4957	0.0000	84.744	0.48749	0.07535	7308.2	1921.5	55.1	U/
2.711	1.4661	0.0000	84.761	0.49020	0.07900	7426.7	1960.6	61.2	U
2.733	1.4416	0.0000	84.778	0.49281	0.08255	7543.0	1999.9	67.7	U
2.756	1.4211	0.0000	84.794	0.49533	0.08602	7657.5	2039.5	74.5	U/
2.778	1.4036	0.0000	84.810	0.49776	0.08942	7770.5	2079.2	81.5	U/
2.800	1.3887	0.0000	84.825	0.50012	0.09275	7882.2	2119.1	88.8	U/
2.822	1.3754	0.0000	84.839	0.50241	0.09602	7992.7	2159.2	96.3	U/
2.844	1.3638	0.0000	84.854	0.50465	0.09924	8102.3	2199.5	104.1	U/
2.867	1.3543	0.0000	84.867	0.50683	0.10241	8211.0	2240.0	112.2	U/
2.889	1.3461	0.0000	84.881	0.50896	0.10554	8319.0	2280.6	120.5	U/
2.911	1.3391	0.0000	84.894	0.51104	0.10864	8426.4	2321.4	129.1	U/
2.933	1.3330	0.0000	84.907	0.51309	0.11170	8533.3	2362.4	137.9	Ŭ/
2.956	1.3278	0.0000	84.920	0.51510	0.11473	8639.8	2403.5	146.9	Ŭ/
2.978	1.3235	0.0000	84.932		0.11773	8745.8	2444.8	156.2	Ŭ/
				0.51707					
3.000	1.3198	0.0000	84.945	0.51901	0.12071	8851.5	2486.2	165.8	U/
3.022	1.3067	0.0000	84.957	0.52088	0.12364	8956.6	2527.8	175.5	U/
3.044	1.2775	0.0000	84.968	0.52264	0.12647	9060.0	2569.6	185.6	U/
3.067	1.2236	0.0000	84.979	0.52424	0.12911	9160.0	2611.4	195.8	U/
3.089	1.1424	0.0000	84.988	0.52560	0.13144	9254.7	2653.4	206.2	U/
3.111	1.0572	0.0000	84.996	0.52672	0.13340	9342.6	2695.5	216.8	U/
3.133	0.9759	0.0000	85.002	0.52761	0.13498	9424.0	2737.7	227.5	U/
3.156	0.9055	0.0000	85.007	0.52829	0.13621	9499.2	2780.0	238.4	U/
3.178	0.8463	0.0000	85.011	0.52880	0.13715	9569.3	2822.2	249.3	Ŭ/
3.200	0.7963	0.0000	85.014	0.52917	0.13784	9635.0	2864.6	260.3	Ŭ/
3.222	0.7544	0.0000	85.016	0.52942	0.13832	9697.0	2906.9	271.4	Ŭ/
		0.0000	85.017	0.52956	0.13863	9756.0	2949.3	282.4	U/
3.244	0.7189								

## Detailed Results (cont,d.) :: Scenario 11 :: 100yr-4hr post

Elapsed Time (hours)	Inflow Rate (ft <sup>3</sup> /s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flow Type
28.911	0.0000	0.0000	82.145	0.02463	0.00000	11018.5	10181.9	836.6	U/S
52.911	0.0000	0.0000	81.491	0.00000	0.00000	11018.5	10181.9	836.6	S
76.911	0.0000	0.0000	81.131	0.00000	0.00000	11018.5	10181.9	836.6	S
100.911	0.0000	0.0000	80.890	0.00000	0.00000	11018.5	10181.9	836.6	\$\$\$\$\$
124.911	0.0000	0.0000	80.711	0.00000	0.00000	11018.5	10181.9	836.6	S
148.911	0.0000	0.0000	80.571	0.00000	0.00000	11018.5	10181.9	836.6	S
172.911	0.0000	0.0000	80.456	0.00000	0.00000	11018.5	10181.9	836.6	S
196.911	0.0000	0.0000	80.360	0.00000	0.00000	11018.5	10181.9	836.6	S
220.911	0.0000	0.0000	80.278	0.00000	0.00000	11018.5	10181.9	836.6	S
244.911	0.0000	0.0000	80.206	0.00000	0.00000	11018.5	10181.9	836.6	S
268.911	0.0000	0.0000	80.142	0.00000	0.00000	11018.5	10181.9	836.6	S
292.911	0.0000	0.0000	80.086	0.00000	0.00000	11018.5	10181.9	836.6	S
316.911	0.0000	0.0000	80.035	0.00000	0.00000	11018.5	10181.9	836.6	S
340.911	0.0000	0.0000	79.989	0.00000	0.00000	11018.5	10181.9	836.6	S
364.911	0.0000	0.0000	79.947	0.00000	0.00000	11018.5	10181.9	836.6	S
388.911	0.0000	0.0000	79.908	0.00000	0.00000	11018.5	10181.9	836.6	S
412.911	0.0000	0.0000	79.872	0.00000	0.00000	11018.5	10181.9	836.6	S
436.911	0.0000	0.0000	79.839	0.00000	0.00000	11018.5	10181.9	836.6	S
460.911	0.0000	0.0000	79.808	0.00000	0.00000	11018.5	10181.9	836.6	S
484.911	0.0000	0.0000	79.779	0.00000	0.00000	11018.5	10181.9	836.6	S
508.911	0.0000	0.0000	79.752	0.00000	0.00000	11018.5	10181.9	836.6	S
532.911	0.0000	0.0000	79.727	0.00000	0.00000	11018.5	10181.9	836.6	S
556.911	0.0000	0.0000	79.703	0.00000	0.00000	11018.5	10181.9	836.6	S
580.911	0.0000	0.0000	79.680	0.00000	0.00000	11018.5	10181.9	836.6	S
604.911	0.0000	0.0000	79.659	0.00000	0.00000	11018.5	10181.9	836.6	S
628.911	0.0000	0.0000	79.638	0.00000	0.00000	11018.5	10181.9	836.6	S
652.911	0.0000	0.0000	79.619	0.00000	0.00000	11018.5	10181.9	836.6	~~~~~
676.911	0.0000	0.0000	79.600	0.00000	0.00000	11018.5	10181.9	836.6	S
700.911	0.0000	0.0000	79,583	0.00000	0.00000	11018.5	10181.9	836.6	S
724.911	0.0000	0.0000	79.566		100 <u>00</u> 10	11018.5	10181.9	836.6	N.A.

Detailed Results :: Scenario 12 :: 100yr-8hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flov Typ
0.000	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	N.A
0.022	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ű
0.044	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.044	0.0000	0.0000	78.500	0.00000	0.00000			0.0	ŭ
0.089	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0		ŭ
0.111	0.0000	0.0000	78.500			0.0	0.0	0.0	Ŭ
				0.00000	0.00000	0.0	0.0	0.0	0
0.133	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.156	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.178	0.0000	0.0000	78.500	0.00000	0.00000	0,0	0.0	0.0	U
0.200	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.222	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.244	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	U
0.267	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.289	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.311	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.333	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.356	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.378	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	U
0.400	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.422	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ű
0.444	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.467	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.489	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.511	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.533	0.0000	0.0000	78.500						ŭ
				0.00000	0.00000	0.0	0.0	0.0	
0.556	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.578	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.600	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.622	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.644	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.667	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ų
0.689	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.711	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.733	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.756	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.778	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.800	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.822	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
0.844	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ũ
0.867	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.889	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.911	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.933	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.956	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
	0.0000								ŭ
0.978		0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	
1.000	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.022	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.044	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.067	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.089	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	υ
1.111	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	U
1.133	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.156	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.178	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.200	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.222	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
1.244	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ū
1.267	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.289	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.311	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
1 222	0.0000								
1.333		0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.356	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.378	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.400	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.422	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.444	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.467	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.489	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ũ
1.511	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
1.533	0.0000	0.0000	78.500	0.00000	0.00000	- 0.0	0.0	0.0	ŭ
1.556	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.578	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0		ŭ
1.600	0.0000	0.0000	78.500	0.00000	0.00000			0.0 0.0	Ŭ
	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	

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Detailed Results (cont.d.) :: Scenario 12 :: 100yr-8hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft <sup>9</sup> /s)	Overflow Discharge (ft%s)	Cumulative Inflow Volume (ft <sup>a</sup> )	Cumulative Infiltration Volume (ft <sup>2</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	FI Ty
3.289	1.3378	0.0000	83.394	0.28737	0.00000	1863.1	994.9	0.0	U
3.311	1.3876	0.0000	83.429	0.29245	0.00000	1972.1	1018.1	0.0	Ŭ
3.333	1.4362	0.0000	83.464	0.29765	0.00000	2085.0	1041.7	0.0	Ŭ
3.356	1.4808	0.0000	83.500	0.30297	0.00000	2201.7	1065.7	0.0	Ŭ
3.378	1.5231	0.0000	83.537	0.30840	0.00000	2321.9	1090.2	0.0	Ŭ
3.400	1.5632	0.0000	83.574	0.31388	0.00000	2445.3	1115.0	0.0	Ŭ
3.422	1.6014	0.0000	83.612	0.31941	0.00000	2571.9	1140.4	0.0	Ŭ
3.444	1.6377	0.0000	83.650	0.32497	0.00000	2701.5	1166.2	0.0	ŭ
						2833.9			ŭ
3.467	1.6723	0.0000	83.688	0.33056	0.00000	2833.9	1192.4	0.0	ŭ
3.489	1.7052	0.0000	83.726	0.33617	0.00000	2969.0	1219.0	0.0	
3.511	1.7366	0.0000	83.764	0.34178	0.00000	3106.6	1246.2	0.0	L
3.533	1.7665	0.0000	83.802	0.34740	0.00000	3246.8	1273.7	0.0	L
3.556	1.7949	0.0000	83.840	0.35302	0.00000	3389.2	1301.7	0.0	L
3.578	1.8219	0.0000	83.879	0.35864	0.00000	3533.9	1330.2	0.0	L
3.600	1.8477	0.0000	83.917	0.36424	0.00000	3680.7	1359.1	0.0	Ļ
3.622	1.8723	0.0000	83.955	0.36982	0.00000	3829.5	1388.5	0.0	U
3.644	1.8958	0.0000	83.993	0.37539	0.00000	3980.2	1418.3	0.0	L
3.667	1.9181	0.0000	84.031	0.38094	0.00000	4132.7	1448.6	0.0	L
3.689	1.9395	0.0000	84.069	0.38646	0.00000	4287.0	1479.3	0.0	L
3.711	1.9599	0.0000	84.106	0.39196	0.00000	4443.0	1510.4	0.0	L
3.733	1.9794	0.0000	84.143	0.39743	0.00000	4600.6	1542.0	0.0	U
3.756	1.9980	0.0000	84.181	0.40287	0.00000	4759.7	1574.0	0.0	L
3.778	2.0157	0.0000	84.217	0.40828	0.00043	4920.2	1606.4	0.0	L
3.800	2.0325	0.0000	84.254	0.41365	0.00236	5082.2	1639.3	0.1	ũ
3.822	2.0487	0.0000	84.291	0.41898	0.00511	5245.4	1672.6	0.4	ũ
3.844	2.0641	0.0000	84.327	0.42427	0.00846	5409.9	1706.3	1.0	ũ
3.867	2.0790	0.0000	84.362	0.42951	0.01229	5575.6	1740.5	1.8	ŭ
3.889	2.0932	0.0000	84.398	0.43471	0.01653	5742.5	1775.1	3.0	ĩ
									ŭ
3.911	2.1070	0.0000	84.433	0.43986	0.02112	5910.5	1810.0	4.5	
3.933	2.1203	0.0000	84.468	0.44503	0.02602	6079.6	1845.4	6.3	L
3.956	2.1333	0.0000	84.502	0.45035	0.03121	6249.8	1881.2	8.6	L
3.978	2.1459	0.0000	84.536	0.45574	0.03663	6420.9	1917.5	11.3	L
4.000	2.1581	0.0000	84.570	0.46106	0.04228	6593.1	1954.2	14.5	L
4.022	2.1532	0.0000	84.603	0.46627	0.04809	6765.6	1991.3	18.1	L
4.044	2.1198	0.0000	84.635	0.47126	0.05397	6936.5	2028.8	22.2	L
4.067	2.0437	0.0000	84.666	0.47593	0.05975	7103.0	2066.7	26.8	L
4.089	1.9207	0.0000	84.694	0.48018	0.06524	7261.6	2104.9	31.8	L
4.111	1.7896	0.0000	84.719	0.48398	0.07031	7410.0	2143.5	37.2	ι
4.133	1.6636	0.0000	84.742	0.48734	0.07491	7548.1	2182.4	43.0	L
4.156	1.5547	0.0000	84.762	0.49034	0.07907	7676.9	2221.5	49.1	ι
4.178	1.4632	0.0000	84.780	0.49302	0.08285	7797.6	2260.8	55.6	L
4.200	1.3863	0.0000	84.795	0.49544	0.08629	7911.6	2300.4	62.4	L
4.222	1.3220	0.0000	84.810	0.49763	0.08945	8019.9	2340.1	69.4	L
4.244	1.2674	0.0000	84.823	0.49964	0.09236	8123.5	2380.0	76.7	ũ
4.267	1.2203	0.0000	84.835	0.50149	0.09506	8223.0	2420.0	84.2	i
4.289	1.1791	0.0000	84.846	0.50320	0.09757	8319.0	2460.2	91.9	ũ
	1.1433	0.0000	84.857	0.50478	0.09991	8411.9	2500.5	99.8	ŭ
4.311				0.50625				107.9	ũ
4.333	1.1092	0.0000	84.866		0.10211	8502.0	2541.0 2581.5		ŭ
4.356	1.0810	0.0000	84.875	0.50762	0.10416	8589.6		116.1	
4.378	1.0559	0.0000	84.883	0.50890	0.10609	8675.0	2622.2	124.5	L
4.400	1.0335	0.0000	84.891	0.51011	0.10792	8758.6	2663.0	133.1	L.
4.422	1.0137	0.0000	84.899	0.51125	0.10965	8840.5	2703.8	141.8	L
4.444	0.9960	0.0000	84.906	0.51233	0.11129	8920.9	2744.8	150.6	L
4.467	0.9803	0.0000	84.912	0.51335	0.11285	8999.9	2785.8	159.6	L
4.489	0.9665	0.0000	84.918	0.51433	0.11434	9077.8	2826.9	168.7	L
4.511	0.9542	0.0000	84.924	0.51526	0.11577	9154.6	2868.1	177.9	L
4.533	0.9435	0.0000	84.930	0.51615	0.11714	9230.6	2909.3	187.2	U
4.556	0.9343	0.0000	84.936	0.51700	0.11846	9305.7	2950.7	196.6	L
4.578	0.9265	0.0000	84.941	0.51783	0.11974	9380.1	2992.1	206.2	L
4.600	0.9196	0.0000	84.946	0.51863	0.12098	9453.9	3033.5	215.8	L
4.622	0.9137	0.0000	84.951	0.51940	0.12219	9527.3	3075.0	225.5	ũ
4.644	0.9088	0.0000	84.956	0.52016	0.12336	9600.2	3116.6	235.3	ŭ
4.667	0.9049	0.0000	84.960	0.52090	0.12451	9672.7	3158.3	245.2	ŭ
		0.0000	84.965		0.12564	9745.0	3200.0	255.3	ŭ
4.689	0.9016			0.52162					
4.711	0.8989	0.0000	84.969	0.52233	0.12675	9817.0	3241.7	265.3	5
4.733	0.8969	0.0000	84.974	0.52303	0.12784	9888.8	3283.5	275.5	L
4.756	0.8957	0.0000	84.978	0.52371	0.12892	9960.5	3325.4	285.8	L
4.778	0.8949	0.0000	84.982	0.52439	0.12998	10032.2	3367.3	296.2	L
4.800	0.8947	0.0000	84.987	0.52506	0.13104	10103.7	3409.3	306.6	U
4.822	0.8948	0.0000	84,991	0.52573	0.13210	10175.3	3451.3	317.1	U
4.844	0.8952	0.0000	84.995	0.52639	0.13314	10246.9	3493.4	327.7	Ū
4.867	0.8960	0.0000	84.999	0.52705	0.13419	10318.6	3535.6	338.4	Ŭ
4.889	0.8970	0.0000	85.003	0.52771	0.13523	10390.3	3577.8	349.2	ũ
	0.8979	0.0000	85.007	0.52836	0.13627	10462.1	3620.0	360.1	ŭ

:: Scenario 12 :: 100yr-8hr post

Detailed Results (cont,d.)

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Fic Ty
8.222	0.0605	0.0000	84.552	0.18887	0.03925	14309.6	8373.9	1381.7	5
8.244	0.0531	0.0000	84.549	0.18730	0.03867	14314.2	8388.9	1384.B	5
8.267	0.0467	0.0000	84.545	0.18574	0.03807	14318.2	8403.9	1387.9	9
8.289	0.0410	0.0000	84.541	0.18418	0.03747	14321.7	8418.7	1390.9	
8.311	0.0361	0.0000	84.538	0.18263	0.03686	14324.8	8433.3	1393.9	5
8.333	0.0314	0.0000	84.534	0.18109	0.03624	14327.5	8447.9	1396,8	5
8.356	0.0275	0.0000	84.530	0.17955	0.03562	14329.8	8462.3	1399.7	5
8.378	0.0240	0.0000	84.526	0.17802	0.03499	14331.9	8476.6	1402.5	5
8.400	0.0209	0.0000	84.522	0.17650	0.03437	14333.7	8490.8	1405.3	-
8.422	0.0182	0.0000	84.518	0.17498	0.03374	14335.2	8504.9	1408.0	5
8.444	0.0157	0.0000	84.514	0.17347	0.03312	14336.6	8518.8	1410.7	5
8.467	0.0135	0.0000	84.511	0.17197	0.03249	14337.8	8532.6	1413.3	5
8.489	0.0115	0.0000	84.507	0.17048	0.03188	14338.8	8546.3	1415.9	\$
8.511	0.0098	0.0000	84.503	0.16899	0.03126	14339.6	8559.9	1418.4	5
8.533	0.0082	0.0000	84.499	0.16753	0.03065	14340.3	8573.3	1420.9	5
8.556	0.0069	0.0000	84.495	0.16607	0.03005	14340,9	8586.7	1423.3	5
8.578	0.0057	0.0000	84.491	0.16463	0.02945	14341.4	8599.9	1425.7	5
8.600	0.0047	0.0000	84.487	0.16320	0.02885	14341.9	8613.0	1428.0	5
8.622	0.0038	0.0000	84.483	0.16180	0.02827	14342.2	8626.0	1430.3	5
8.644	0.0030	0.0000	84.479	0.16041	0.02769	14342.5	8638.9	1432.5	5
8.667	0.0023	0.0000	84.475	0.15904	0.02711	14342.7	8651.7	1434.7	5
8.689	0.0018	0.0000	84.471	0.15769	0.02655	14342.8	8664.4	1436.9	5
8.711	0.0013	0.0000	84.468	0.15636	0.02599	14343.0	8676.9	1439.0	5
8.733	0.0009	0.0000	84.464	0.15505	0.02544	14343.0	8689.4	1441.0	5
8.756	0.0006	0.0000	84.460	0.15376	0.02490	14343.1	8701.7	1443.0	5
8.778	0.0004	0.0000	84.456	0.15250	0.02436	14343.1	8714.0	1445.0	5
8.800	0.0002	0.0000	84.453	0.15126	0.02384	14343.2	8726.1	1446.9	5
8.822	0.0001	0.0000	84.449	0.15005	0.02332	14343.2	8738.2	1448.8	5
8.844	0.0000	0.0000	84.445	0.14886	0.02281	14343.2	8750.1	1450.7	5
8.867	0.0000	0.0000	84.442	0.14770	0.02231	14343.2	8762.0	1452.5	5
8.889	0.0000	0.0000	84.438	0.14656	0.02181	14343.2	8773.8	1454.2	5
8.911	0.0000	0.0000	84.435	0.14588	0.02132	14343.2	8785.5	1456.0	5
32.911	0.0000	0.0000	83.277	0.01841	0.00000	14343.2	11376.9	2377.2	5
56.911	0.0000	0.0000	82.475	0.00341	0.00000	14343.2	11966.0	2377.2	5
80.911	0.0000	0.0000	81.975	0.00000	0.00000	14343.2	11966.0	2377.2	
104.911	0.0000	0.0000	81.642	0.00000	0.00000	14343.2	11966.0	2377.2	5
128.911	0.0000	0.0000	81.398	0.00000	0.00000	14343.2	11966.0	2377.2	5
152.911	0.0000	0.0000	81.207	0.00000	0.00000	14343.2	11966.0	2377.2	5
176.911	0.0000	0.0000	81.052	0.00000	0.00000	14343.2	11966.0	2377.2	5
200.911	0.0000	0.0000	80.923	0.00000	0.00000	14343.2	11966.0	2377.2	S
224.911	0.0000	0.0000	80.813	0.00000	0.00000	14343.2	11966.0	2377.2	
248.911	0.0000	0.0000	80.717	0.00000	0.00000	14343.2	11966.0	2377.2	S
272.911	0.0000	0.0000	80.633	0.00000	0.00000	14343.2	11966.0	2377.2	S
296.911	0.0000	0.0000	80.558	0.00000	0.00000	14343.2	11966.0	2377.2	S
320.911	0.0000	0.0000	80.490	0.00000	0.00000	14343.2	11966.0	2377.2	S
344.911	0.0000	0.0000	80,429	0.00000	0.00000	14343.2	11966.0	2377.2	00 00 00
368.911	0.0000	0.0000	80.373	0.00000	0.00000	14343.2	11966.0	2377.2	S
392.911	0.0000	0.0000	80.322	0.00000	0.00000	14343.2	11966.0	2377.2	S
416.911	0.0000	0.0000	80.275	0.00000	0.00000	14343.2	11966.0	2377.2	S
440.911	0.0000	0.0000	80.232	0.00000	0.00000	14343.2	11966.0	2377.2	S
464.911	0.0000	0.0000	80.191	0.00000	0.00000	14343.2	11966.0	2377.2	00 00
488.911	0.0000	0.0000	80.154	0.00000	0.00000	14343.2	11966.0	2377.2	S
512.911	0.0000	0.0000	80.118	0.00000	0.00000	14343.2	11966.0	2377.2	S
536.911	0.0000	0.0000	80.085	0.00000	0.00000	14343.2	11966.0	2377.2	S
560.911	0.0000	0.0000	80.054	0.00000	0.00000	14343.2	11966.0	2377.2	S
584.911	0.0000	0.0000	80.024	0.00000	0.00000	14343.2	11966.0	2377.2	S
608.911	0.0000	0.0000	79.996	0.00000	0.00000	14343.2	11966.0	2377.2	S
632.911	0.0000	0.0000	79.970	0.00000	0.00000	14343.2	11966.0	2377.2	S
656.911	0.0000	0.0000	79.945	0.00000	0.00000	14343.2	11966.0	2377.2	S
680.911	0.0000	0.0000	79.921	0.00000	0.00000	14343.2	11966.0	2377.2	S
704.911	0.0000	0.0000	79.898	0.00000	0.00000	14343.2	11966.0	2377.2	S
728.911	0.0000	0.0000	79.876	- 11 C	101.0040	14343.2	11966.0	2377.2	N./

:: Scenario 13 :: 100yr-24hr post

Elapsed Outside Stage Elevation Cumulative Inflow Infiltration Overflow Cumulative Cumulative Rate Recharge Discharge Time Infiltration Flow Rate Inflow Discharge (ft/day) Volume (ft3) (hours) (ft<sup>3</sup>/s) (ft datum) (ft<sup>3</sup>/s) (ft3/s) Volume (ft3) Volume (ft3) Тура 0.000 N.A. U 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.022 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.044 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 U 0.067 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.089 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.00000 78.500 78.500 0.00000 0.111 0.0 0.0 0.0 0.0000 0.133 0.00000 0.0 0.0 0.0 0.156 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 78.500 0.00000 0.0 0.178 0.00000 0.0 0.0 0.200 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.222 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.244 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.267 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.289 0.00000 0.00000 78.500 0.0 0.0 0.0 0.311 78,500 0.0 0.0 0.0 0.0000 0.0000 78.500 0.00000 0.333 0.00000 0.0 0.0 0.0 0.356 0.0000 0.00000 0.00000 0.0 0.0000 78,500 0.378 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.400 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.422 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.444 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.467 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.489 0.0000 0.0000 0.00000 0.00000 78,500 0.0 0.0 0.0 0.0000 78.500 0.0 0.0 0.0 0.0 0.511 0.0 0.0000 0.533 0.0000 78,500 0.00000 0.00000 0.00000 0.556 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.578 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.600 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.622 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.644 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.667 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 0.0000 78,500 0.689 0.00000 0.0 0.0 0.0 0.0000 0.00000 0.711 0.0000 78.500 0.00000 0.0 0.0 0.733 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 0.00000 0.756 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.778 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.800 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.822 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.00000 0.844 0.0000 0.0000 78,500 0.00000 0.0 0.0 0.0 0.0000 0.867 0.0 0.0 0.0 0.0 78.500 0.0 0.0 0.889 0.0000 0.0000 78,500 0.00000 0.00000 0.911 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.933 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 ũ 0.0 0.956 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.978 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.000 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 1.022 78.500 0.00000 0.00000 0.0 0.0 0.0 1.044 0.00000 0.00000 0.0000 0.0000 78,500 0.0 0.0 0.0 0.0000 1.067 0.0000 78,500 0.0 1.089 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0000 0.0000 0.00000 0.0 0.0 1.111 78.500 0.00000 0.0 0.0000 0.0000 78.500 0.00000 0.0 1.133 0.00000 0.0 0.0 1.156 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.178 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.200 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.00000 0.0000 1.222 78,500 0.00000 0.0 0.0 0.0 1.244 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.00000 1.267 0.0000 78.500 0.0000 0.0 0.0 1.289 0.0000 78.500 0.00000 0.00000 0.0 1.311 0.0000 0.0000 0.00000 0.0 0.0 78.500 0.00000 0.0 1.333 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.356 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.378 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.00000 1.400 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 0.0000 1,422 0.0000 78.500 0.0 0.0 0.0 0.0000 0.00000 1.444 0.00000 0.0 0.0 0.0 1.467 0.0000 0.0 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0000 0.0000 1.489 78.500 0.00000 0.00000 0.0 0.0 0.0 1.511 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.533 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.556 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.578 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 υ 1.600 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 U U 1.622 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0

Tranmission Doctor

**Detailed Results** 

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Detailed Results (cont,d.) :: Scenario 13 :: 100yr-24hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flo
11.511	0.7178	0.0000	83.822	0.34822	0.00000	7258.9	5227.1	0.0	U/
11.533	0.7201	0.0000	83.832	0.34967	0.00000	7316.4	5255.0	0.0	U/
11.556	0.7223	0.0000	83.842	0.35111	0.00000	7374.1	5283.0	0.0	U/
11.578	0.7243	0.0000	83.852	0.35256	0.00000	7432.0	5311.1	0.0	U/
11.600	0.7262	0.0000	83.861	0.35400	0.00000	7490.0	5339.4	0.0	U/
11.622	0.7280	0.0000	83.871	0.35543	0.00000	7548.2	5367.8	0.0	U/
11.644	0.7297	0.0000	83.881	0.35686	0.00000	7606.5	5396.3	0.0	U/
11.667	0.7313	0.0000	83.891	0.35828	0.00000	7664.9	5424.9	0.0	U/
11.689	0.7328	0.0000	83.900	0.35970	0.00000	7723.5	5453.6	0.0	UI
11.711 11.733	0.7342 0.7355	0.0000	83.910	0.36111	0.00000	7782.1	5482.4	0.0	U/I
11.756	0.7368	0.0000	83.919	0.36252 0.36392	0.00000	7840.9	5511.4	0.0	U
11.778	0.7380	0.0000	83.929 83.939		0.00000	7899.8	5540.4	0.0	U/I
11.800	0.7391	0.0000	83.948	0.36532 0.36671	0.00000	7958.8 8017.9	5569.6 5598.9	0.0	U/I
11.822	0.7402	0.0000	83.957	0.36809	0.00000	8077.1	5628.3	0.0	U/I
11.844	0.7413	0.0000	83.967	0.36947	0.00000	8136.3	5657.8	0.0	U/I
11.867	0.7423	0.0000	83.976	0.37084	0.00000	8195,7	5687.4	0.0	U/I
11.889	0.7433	0.0000	83.986	0.37220	0.00000	8255.1	5717.1	0.0	U/I
11.911	0.7443	0.0000	83.995	0.37356	0.00000	8314.6	5746.9	0.0	U/F
11.933	0.7452	0.0000	84.004	0.37491	0.00000	8374.2	5776.9	0.0	U/F
11.956	0.7462	0.0000	84.013	0.37625	0.00000	8433.8	5806.9	0.0	U/F
11.978	0.7471	0.0000	84.022	0.37759	0.00000	8493.6	5837.1	0.0	U/F
12.000	0.7481	0.0000	84.031	0.37892	0.00000	8553.4	5867.3	0.0	U/F
12.022	0.7463	0.0000	84.040	0.38023	0.00000	8613.2	5897.7	0.0	U/I
12.044	0.7399	0.0000	84.049	0.38151	0.00000	8672.6	5928.2	0.0	U/I
12.067	0.7268	0.0000	84.058	0.38273	0.00000	8731.3	5958.8	0.0	U/I
12.089	0.7061	0.0000	84.066	0.38388	0.00000	8788.6	5989.4	0.0	U/F
12.111	0.6842	0.0000	84.073	0.38494	0.00000	8844.2	6020.2	0.0	U/F
12.133	0.6633	0.0000	84.080	0.38593	0.00000	8898.1	6051.0	0.0	U/F
12.156	0.6453	0.0000	84.087	0.38684	0.00000	8950.5	6081.9	0.0	U/F
12.178	0.6301	0.0000	84.093	0.38770	0.00000	9001.5	6112.9	0.0	U/F
12.200	0.6174	0.0000	84.099	0.38850	0.00000	9051.4	6144.0	0.0	U/F
12.222	0.6068	0.0000	84.104	0.38927	0.00000	9100.3	6175.1	0.0	U/F
12.244	0.5978	0.0000	84.109	0.39000	0.00000	9148.5	6206.2	0.0	U/F
12.267	0.5901	0.0000	84.114	0.39070	0.00000	9196.0	6237.5	0.0	U/F
12.289 12.311	0.5833 0.5775	0.0000	84.118	0.39137	0.00000	9243.0	6268.7	0.0	U/F
12.333	0.5720	0.0000 0.0000	84.123	0.39201	0.00000	9289.4	6300.1	0.0	U/F
12.355	0.5674	0.0000	84.127 84.131	0.39264 0.39324	0.00000	9335.4	6331.5	0.0	U/F
12.378	0.5634	0.0000	84.135	0.39383	0.00000	9381.0 9426.2	6362.9 6394.4	0.0	U/F
12.400	0.5599	0.0000	84.139	0.39440	0.00000	9426.2	6425.9	0.0	U/F
12.422	0.5567	0.0000	84.143	0.39496	0.00000	9515.8	6457.5	0.0	U/F
12.444	0.5540	0.0000	84.147	0.39551	0.00000	9560.2	6489.1	0.0	U/F
12.467	0.5516	0.0000	84.151	0.39605	0.00000	9604.4	6520.8	0.0	U/F
12.489	0.5495	0.0000	84.154	0.39658	0.00000	9648.5	6552.5	0.0	U/F
12.511	0.5476	0.0000	84.158	0.39709	0.00000	9692.4	6584.2	0.0	U/F
12.533	0.5460	0.0000	84.161	0.39760	0.00000	9736.1	6616.0	0.0	U/F
12.556	0.5447	0.0000	84.165	0.39811	0.00000	9779.7	6647.8	0.0	U/F
12.578	0.5436	0.0000	84.168	0.39860	0.00000	9823.3	6679.7	0.0	U/F
12.600	0.5427	0.0000	84.172	0.39909	0.00000	9866.7	6711.6	0.0	U/F
12.622	0.5419	0.0000	84.175	0.39958	0.00000	9910.1	6743.6	0.0	U/F
12.644	0.5413	0.0000	84.178	0.40006	0.00000	9953.4	6775.6	0.0	U/F
12.667	0.5408	0.0000	84.182	0.40054	0.00000	9996.7	6807.6	0.0	U/F
12.689	0.5405	0.0000	84.185	0.40102	0.00000	10040.0	6839.6	0.0	U/F
12.711	0.5403	0.0000	84.188	0.40149	0.00000	10083.2	6871.7	0.0	U/F
12.733	0.5401	0.0000	84.191	0.32250	0.00000	10126.4	6903.9	0.0	U/F
12.756	0.5401	0.0000	84.198	0.17853	0.00000	10169.6	6923.3	0.0	U/S
12.778	0.5402	0.0000	84.208	0.11839	0.00013	10212.8	6932.4	0.0	S
12.800	0.5404	0.0000	84.217	0.12784	0.00043	10256.1	6942.3	0.0	S
12.822	0.5406	0.0000	84.227	0.13773	0.00082	10299.3	6952.9	0.1	S
12.844	0.5408	0.0000	84.236	0.14806	0.00127	10342.6	6964.3	0.2	S
12.867	0.5412	0.0000	84.245	0.15881	0.00177	10385.8	6976.6	0.3	S
12.889	0.5415	0.0000	84.253	0.16993	0.00229	10429.1	6989.7	0.4	S
12.911	0.5419	0.0000	84.261	0.18139	0.00285	10472.5	7003.8	0.7	S
12.933	0.5422	0.0000	84.269	0.19312	0.00342	10515.8	7018.8	0.9	SS
12.956	0.5426	0.0000	84.277	0.20506	0.00400	10559.2	7034.7	1.2	S
12.978	0.5429	0.0000	84.284	0.21712	0.00458	10602.7	7051.6	1.5	S
13.000	0.5433	0.0000	84.291	0.22922	0.00517	10646.1	7069.4	1.9	S
13.022	0.5427	0.0000	84.298	0.24126	0.00575	10689.5	7088.2	2.4	S
13.044	0.5407	0.0000	84.304	0.25314	0.00633	10732.9	7108.0	2.9	S
13.067	0.5363	0.0000	84.310	0.26472	0.00688	10776.0	7128.7	3.4	S
13.089 13.111	0.5295	0.0000	84.316	0.27591 0.28657	0.00742	10818.6 10860.7	7150.4	4.0	S
	0.5221	0.0000	84.321		0.00793		7172.9	4.6	S

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>a</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flow Type
24.667	0.0011	0.0000	84.792	0.06098	0.08553	22599.8	12485.5	4580.4	S
24.689	0.0009	0.0000	84.789	0.06080	0.08494	22599.9	12490.4	4587.2	S
24.711	0.0006	0.0000	84,786	0.06061	0.08435	22600.0	12495.3	4594.0	S
24.733	0.0004	0.0000	84.784	0.06043	0.08376	22600.0	12500.1	4600.7	S
24.756	0.0003	0.0000	84.781	0.06026	0.08318	22600.1	12504.9	4607.4	S
24.778	0.0002	0.0000	84.778	0.06008	0.08260	22600.1	12509.8	4614.0	S
24.800	0.0001	0.0000	84.776	0.05991	0.08203	22600.1	12514.6	4620.6	S
24.822	0.0001	0.0000	84.773	0.05975	0.08145	22600.1	12519.3	4627.1	S
24.844	0.0000	0.0000	84.770	0.05958	0.08089	22600.1	12524.1	4633.6	S
24.867	0.0000	0.0000	84.768	0.05942	0.08032	22600.1	12528.9	4640.1	S
24.889	0.0000	0.0000	84.765	0.05927	0.07976	22600.1	12533.6	4646.5	S
24.911	0.0000	0.0000	84.762	0.05913	0.07920	22600.1	12538.4	4652.8	S
48.911	0.0000	0.0000	83.994	0.00636	0.00000	22600.1	11961.1	8074.5	S
72.911	0.0000	0.0000	83.402	0.01484	0.00000	22600.1	13637.0	8074.5	S
96.911	0.0000	0.0000	82.903	0.00514	0.00000	22600.1	14525.6	8074.5	S
120.911	0.0000	0.0000	82.471	0.00000	0.00000	22600.1	14525.6	8074.5	S
144.911	0.0000	0.0000	82.156	0.00000	0.00000	22600.1	14525.6	8074.5	S
168.911	0.0000	0.0000	81.911	0.00000	0.00000	22600.1	14525.6	8074.5	S
192.911	0.0000	0.0000	81.713	0.00000	0.00000	22600.1	14525.6	8074.5	S
216.911	0.0000	0.0000	81.548	0.00000	0.00000	22600.1	14525.6	8074.5	S
240.911	0.0000	0.0000	81.407	0.00000	0.00000	22600.1	14525.6	8074.5	S
264.911	0.0000	0.0000	81.285	0.00000	0.00000	22600.1	14525.6	8074.5	S
288.911	0.0000	0.0000	81.178	0.00000	0.00000	22600.1	14525.6	8074.5	S
312.911	0.0000	0.0000	81.083	0.00000	0.00000	22600.1	14525.6	8074.5	S
336.911	0.0000	0.0000	80.997	0.00000	0.00000	22600.1	14525.6	8074.5	S
360.911	0.0000	0.0000	80.920	0.00000	0.00000	22600.1	14525.6	8074.5	S
384.911	0:0000	0.0000	80.850	0.00000	0.00000	22600.1	14525.6	8074.5	S
408.911	0.0000	0.0000	80.785	0.00000	0.00000	22600.1	14525.6	8074.5	S
432.911	0.0000	0.0000	80.726	0.00000	0.00000	22600.1	14525.6	8074.5	S
456.911	0.0000	0.0000	80.671	0.00000	0.00000	22600.1	14525.6	8074.5	S
480.911	0.0000	0.0000	80.620	0.00000	0.00000	22600.1	14525.6	8074.5	S
504.911	0.0000	0.0000	80.572	0.00000	0.00000	22600.1	14525.6	8074.5	S
528.911	0.0000	0.0000	80.528	0.00000	0.00000	22600.1	14525.6	8074.5	S
552.911	0.0000	0.0000	80.486	0.00000	0.00000	22600.1	14525.6	8074.5	S
576.911	0.0000	0.0000	80.446	0.00000	0.00000	22600.1	14525.6	8074.5	S
600.911	0.0000	0.0000	80.409	0.00000	0.00000	22600.1	14525.6	8074.5	S
624.911	0.0000	0.0000	80.374	0.00000	0.00000	22600.1	14525.6	8074.5	ຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑຑ
648.911	0.0000	0.0000	80.341	0.00000	0.00000	22600.1	14525.6	8074.5	S
672.911	0.0000	0.0000	80.309	0.00000	0.00000	22600.1	14525.6	8074.5	S
696.911	0.0000	0.0000	80.279	0.00000	0.00000	22600.1	14525.6	8074.5	S
720.911	0.0000	0.0000	80.250	0.00000	0.00000	22600.1	14525.6	8074.5	S
744.911	0.0000	0.0000	80.223			22600.1	14525.6	8074.5	N.A.

Detailed Results (cont,d.) :: Scenario 13 :: 100yr-24hr post

:: Scenario 14 :: 100yr-72hr post

Elapsed Inflow Outside Stage Infiltration Overflow Cumulative Cumulative Cumulative Discharge Time Rate Recharge Elevation Discharge Rate Inflow Infiltration Flow (hours) (ft3/s) (ft/day) (ft3/s) Volume (ft<sup>a</sup>) (ft datum) (ft3/s) Volume (ft<sup>a</sup>) Volume (ft<sup>a</sup>) Туре 0.000 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 N.A. 0.022 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 U 0.0000 0.0000 0.044 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 U 0.067 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 U 0.089 0.0000 0.0000 78.500 78.500 0.00000 U 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.111 0.00000 0.0 0.0 0.0 20000 0.133 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.156 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0000 0.0000 78.500 0.00000 0.00000 0.178 0.0 0.0 0.0 0.200 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 UU 0.222 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.0000 0.244 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.267 U 78.500 0.00000 0.00000 0.0 0.0 0.0 0.00000 0.00000 0.289 78.500 0.0 0.0 0.0 00000000 0.0000 0.0000 0.311 0.0 0.0 0.0 0.0000 0.333 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.356 0.0000 0.0000 0.00000 0.00000 0.0 0.0 78.500 0.378 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.400 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0,0 0.0 0.422 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.444 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0000000000 0.00000 0.00000 0.00000 0.0000 0.00000 78.500 0.0 0.0 0.0 0.489 0.0000 78.500 0.00000 0.0 0.0 0.0 0.511 0.0000 0.0000 0.00000 78.500 0.0 0.0 0.0 0.0000 0.0000 0.0 0.0 0.533 78.500 0.00000 0.00000 0.556 0.0000 0.0000 0.00000 0.00000 78.500 0.0 0.0 0.578 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.600 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.622 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.644 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.667 0.0000 0.0 0.0 0.0 0.0 78,500 0.00000 0.0 0.0 0.0000 0.00000 0.689 0.0000 78.500 0.0 0.0000 0.0000 0.00000 0.00000 0.711 78.500 0.0 0.0 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.733 0.0 0.756 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.778 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.0000 0.0000 0.800 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.822 0.0000 0.000.0 0.00000 0.00000 78,500 0.0 0.0 0.0 0.844 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 78.500 0.00000 0.867 0.0000 0.0 0.0 0.0 0.889 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.911 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.933 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.956 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 0.978 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 1.000 0.0000 0.0000 0.0000 0.00000 0.0000 78.500 0.00000 0.0 0.0 0.0 1.022 0.00000 78.500 0.0 0.0 0.0 0.00000 0.0000 1.044 78,500 0.00000 0.0 0.0 0.0 0.0000 0.0000 1.067 78,500 0.00000 0.0 0.0 0.0 0.0000 0.0000 78.500 0.00000 0.0 1.089 0.00000 0.0 0.0000 0.0000 0.00000 0.00000 0.0 1.111 78.500 0.0 0.0 1.133 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.156 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.178 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.00000 0.00000 0.00000 0.00000 0.0000 1.200 0.0000 78.500 0.00000 0.0 0.0 0.0 1.222 0.0000 78,500 0.00000 0.0 0,0 0.0 0.0000 0.0000 1.244 78 500 0 00000 0.0 0.0 0.0 0.0000 0.0000 1.267 78.500 0.00000 0.0 0.0000 78.500 0.00000 0.0 0.0 1.289 0.0000 0.00000 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.311 1.333 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.356 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.378 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.00000 0.00000 0.00000 0.0000 1.400 0.0000 78.500 0.00000 0.0 0.0 0.0 78.500 1.422 0.0 0.0 0.0 1.444 0.0000 0.0000 78.500 0.00000 D.0 0.0 0.0 0.0000 0.0000 0.00000 0.0 1.467 78.500 0.00000 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 1.489 0.00000 1.511 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.533 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.556 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.578 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.600 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0,0 1.622 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0

Tranmission Doctor

**Detailed Results** 

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Detailed Results (cont,d.) :: Scenario 14 :: 100yr-72hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flov Typ
55.911	0.1385	0.0000	84.180	0.05784	0.00000	16079.0	12896.4	0.0	S
55.933	0.1386	0.0000	84.182	0.05786	0.00000	16090.1	12901.0	0.0	S
55.956	0.1386	0.0000	84.183	0.05788	0.00000	16101.2	12905.6	0.0	S
55.978	0.1386	0.0000	84.185	0.05790	0.00000	16112.3	12910.3	0.0	S
56.000	0.1386	0.0000	84.187	0.05792	0.00000	16123.4	12914.9	0.0	S
56.022	0.1427	0.0000	84.189	0.05796	0.00000	16134.6	12919.5	0.0	ŝ
56.044	0.1540	0.0000	84.191	0.05805	0.00000	16146.5	12924.2	0.0	c
56.067	0.1762	0.0000	84.194	0.05825	0.00000	16159.7	12928.8	0.0	0
56.089	0.2105	0.0000	84.197	0.05856	0.00000	16175.2	12933.5	0.0	0
56.111	0.2474	0.0000	84.201	0.05902	0.00000	16193.5	12938.2	0.0	0
56.133									5
	0.2830	0.0000	84.205	0.05961	0.00007	16214.7	12942.9	0.0	5
56.156	0.3140	0.0000	84.211	0.06030	0.00021	16238.6	12947.7	0.0	S
56.178	0.3403	0.0000	84.217	0.06104	0.00042	16264.8	12952.6	0.0	S
56.200	0.3625	0.0000	84.224	0.06184	0.00068	16292.9	12957.5	0.1	S
56.222	0.3811	0.0000	84.231	0.06265	0.00101	16322.6	12962.5	0.2	S
56.244	0.3970	0.0000	84.238	0.06349	0.00139	16353.8	12967.5	0.2	S
56.267	0.4108	0.0000	84.246	0.06432	0.00184	16386.1	12972.6	0.4	S
56.289	0.4229	0.0000	84.254	0.06515	0.00233	16419.4	12977.8	0.5	S
56.311	0.4335	0.0000	84.262	0.06598	0.00288	16453.7	12983.1	0.8	S
56.333	0.4435	0.0000	84.270	0.06679	0.00348	16488.8	12988.4	1.0	S
56.356	0.4520	0.0000	84.279	0.06759	0.00413	16524.6	12993.8	1.3	G
56.378	0.4595	0.0000	84.287	0.06838	0.00413	16561.0	12999.2	1.3	<i>。。</i> 。。。。。。。。。。。。。。。。。。。。。。。。。。。。。。。。。。
56.400	0.4662	0.0000	84.296	0.06915					0
56.400	0.4662	0.0000	84.305	0.06989	0.00557	16598.1	13004.7	2.1	50
					0.00635	16635.6	13010.3	2.6	S
56.444	0.4776	0.0000	84.313	0.07062	0.00717	16673.6	13015.9	3.1	S
56.467	0.4825	0.0000	84.322	0.07133	0.00803	16712.0	13021.6	3.7	S
56.489	0.4868	0.0000	84.331	0.07201	0.00892	16750.8	13027.3	4.4	S
56.511	0.4907	0.0000	84.340	0.07268	0.00985	16789.9	13033.1	5.1	S
56.533	0.4941	0.0000	84.349	0.07332	0.01081	16829.3	13038.9	6.0	S
56.556	0.4971	0.0000	84.358	0.07395	0.01179	16868.9	13044.8	6.9	S
56.578	0.4997	0.0000	84.367	0.07455	0.01281	16908.8	13050.8	7.9	S
56.600	0.5020	0.0000	84.376	0.07513	0.01386	16948.9	13056.7	8.9	S
56.622	0.5041	0.0000	84.385	0.07569	0.01493	16989.1	13062.8	10.1	S
56.644	0.5059	0.0000	84.394	0.07624	0.01602	17029.5	13068.9	11.3	S
56.667	0.5074	0.0000	84.403	0.07677	0.01713	17070.0	13075.0	12.6	ē
56.689	0.5087	0.0000	84.412	0.07727	0.01827	17110.7	13081.1	14.1	0
56.711	0.5099							14.1	0
		0.0000	84.420	0.07776	0.01942	17151.4	13087.3	15.6	5
56.733	0.5108	0.0000	84.429	0.07823	0.02060	17192.2	13093.6	17.2	S
56.756	0.5116	0.0000	84.438	0.07869	0.02179	17233.1	13099.9	18.9	S
56.778	0.5122	0.0000	84.447	0.07913	0.02299	17274.1	13106.2	20.6	S
56.800	0.5127	0.0000	84.455	0.07955	0.02421	17315.1	13112.5	22.5	S
56.822	0.5131	0.0000	84.464	0.07996	0.02545	17356.1	13118.9	24.5	S
56.844	0.5134	0.0000	84.472	0.08035	0.02669	17397.2	13125.3	26.6	S
56.867	0.5136	0.0000	84.481	0.08073	0.02795	17438.3	13131.8	28.8	S
56.889	0.5137	0.0000	84.489	0.08109	0.02922	17479.4	13138.2	31.1	S
56.911	0.5139	0.0000	84.498	0.08145	0.03050	17520.5	13144.7	33.5	S
56.933	0.5140	0.0000	84.506	0.08179	0.03179	17561.6	13151.3	36.0	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
56.956	0.5141	0.0000	84.514	0.08211	0.03308	17602.7	13157.8	38.6	S
56.978	0.5143	0.0000	84.522	0.08243	0.03439	17643.8	13164.4	41.3	S
57.000	0.5144	0.0000	84.531	0.08274	0.03570	17685.0	13171.0	44.1	8
57.022	0.5145	0.0000	84.539	0.08304	0.03703	17726.1	13177.6	47.0	c
57.044	0.5146	0.0000	84.547	0.08332	0.03836	17767.3	13184.3	50.0	555
57.067	0.5148	0.0000	84.555	0.08360	0.03969	17808.5	13191.0	53.1	S
57.089	0.5148	0.0000	84.563		0.04103				
				0.08387		17849.7	13197.7	56.3	S
57.111	0.5150	0.0000	84.571	0.08414	0.04238	17890.9	13204.4	59.7	SS
57.133	0.5152	0.0000	84.579	0.08439	0.04373	17932.1	13211.1	63.1	S
57.156	0.5153	0.0000	84.586	0.08464	0.04509	17973.3	13217.9	66.7	S
57.178	0.5154	0.0000	84.594	0.08488	0.04645	18014.5	13224.7	70.3	S
57.200	0.5155	0.0000	84.602	0.08511	0.04782	18055.8	13231.5	74.1	S
57.222	0.5157	0.0000	84.609	0.08534	0.04919	18097.0	13238.3	78.0	S
57.244	0.5158	0.0000	84.617	0.08555	0.05057	18138.3	13245.1	82.0	S
57.267	0.5159	0.0000	84.625	0.08577	0.05195	18179.5	13252.0	86.1	S
57.289	0.5160	0.0000	84.632	0.08598	0.05333	18220.8	13258.8	90.3	S
57.311	0.5162	0.0000	84.640	0.08618	0.05472	18262.1	13265.7	94.6	S
57.333	0.5163	0.0000	84.647	0.08637	0.05610	18303.4	13272.6	99.0	S
57 356					0.05010				0
57.356	0.5164	0.0000	84.654	0.08657	0.05749	18344.7	13279.5	103.6	S
57.378	0.5165	0.0000	84.662	0.08675	0.05889	18386.0	13286.5	108.2	S
57.400	0.5166	0.0000	84.669	0.08693	0.06028	18427.4	13293.4	113.0	S
57.422	0.5168	0.0000	84.676	0.08711	0.06168	18468.7	13300.4	117.9	S
57.444	0.5169	0.0000	84.683	0.08728	0.06308	18510.0	13307.4	122.9	S
57.467	0.5170	0.0000	84.690	0.08745	0.06448	18551.4	13314.4	128.0	S
57.489	0.5171	0.0000	84.697	0.08761	0.06588	18592.8	13321.4	133.2	S
57.511	0.5172	0.0000	84.704	0.08777	0.06728	18634.1	13328.4	138.5	SS
	S. S. 1 1 &	0.0000	01.101	0.08792	0.06869	18675.5	13335.4	100.0	-

Detailed Results (cont,d.) :: Scenario 14 :: 100yr-72hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flow Type
72,356	0.0076	0.0000	84.696	0.03386	0.06568	30346.8	16579.2	8633.5	S
72.378	0.0067	0.0000	84.695	0.03375	0.06533	30347.3	16581.9	8638.7	S
72.400	0.0058	0.0000	84.693	0.03365	0.06497	30347.8	16584.6	8643.9	S
72.422	0.0050	0.0000	84.691	0.03355	0.06461	30348.3	16587.3	8649.1	S
72.444	0.0044	0.0000	84.689	0.03345	0.06426	30348.7	16590.0	8654.3	S
72.467	0.0037	0.0000	84.687	0.03335	0.06390	30349.0	16592.7	8659.4	S
72.489	0.0032	0.0000	84.686	0.03326	0.06354	30349.3	16595.3	8664.5	S
72.511	0.0027	0.0000	84.684	0.03317	0.06318	30349.5	16598.0	8669.6	S
72.533	0.0023	0.0000	84.682	0.03308	0.06283	30349.7	16600.6	8674.6	S
72.556	0.0019	0.0000	84.680	0.03299	0.06247	30349.9	16603.3	8679.6	S
72.578	0.0016	0.0000	84.678	0.03291	0.06211	30350.0	16605.9	8684.6	S
72.600	0.0013	0.0000	84.676	0.03283	0.06176	30350.1	16608.5	8689.5	S
72.622	0.0010	0.0000	84.675	0.03275	0.06140	30350.2	16611.2	8694.5	S
72.644	0.0008	0.0000	84.673	0.03267	0.06105	30350.3	16613.8	8699.4	S
72.667	0.0006	0.0000	84.671	0.03259	0.06070	30350.3	16616.4	8704.2	S
72.689	0.0005	0.0000	84.669	0.03252	0.06035	30350.4	16619.0	8709.1	S
72.711	0.0004	0.0000	84.667	0.03245	0.06000	30350.4	16621.6	8713.9	S
72.733	0.0003	0.0000	84.666	0.03238	0.05965	30350.5	16624.2	8718.7	S
72.756	0.0002	0.0000	84.664	0.03231	0.05931	30350.5	16626.8	8723.4	S
72.778	0.0001	0.0000	84.662	0.03224	0.05897	30350.5	16629.4	8728.2	S
72.800	0.0001	0.0000	84.660	0.03218	0.05862	30350.5	16631.9	8732.9	S
72.822	0.0000	0.0000	84.658	0.03211	0.05828	30350.5	16634.5	8737.6	S
72.844	0.0000	0.0000	84.657	0.03205	0.05795	30350.5	16637.1	8742.2	S
72.867	0.0000	0.0000	84.655	0.03199	0.05761	30350.5	16639.6	8746.8	ŝ
72.889	0.0000	0.0000	84.653	0.03193	0.05728	30350.5	16642.2	8751.4	S
72.911	0.0000	0.0000	84.651	0.03187	0.05694	30350.5	16644.7	8756.0	S
96.911	0.0000	0.0000	84.097	0.00584	0.00000	30350.5	16231.4	11215.9	S
120.911	0.0000	0.0000	83.630	0.01406	0.00000	30350.5	17653.6	11215.9	S
144.911	0.0000	0.0000	83.226	0.00857	0.00000	30350.5	18660.4	11215.9	S
168.911	0.0000	0.0000	82.851	0.00274	0.00000	30350.5	19134.6	11215.9	S
192.911	0.0000	0.0000	82.523	0.00000	0.00000	30350.5	19134.6	11215.9	S
216.911	0.0000	0.0000	82.266	0.00000	0.00000	30350.5	19134.6	11215.9	S
240.911	0.0000	0.0000	82.057	0.00000	0.00000	30350.5	19134.6	11215.9	S
264.911	0.0000	0.0000	81.881	0.00000	0.00000	30350.5	19134.6	11215.9	S
288.911	0.0000	0.0000	81.730	0.00000	0.00000	30350.5	19134.6	11215.9	S
312.911	0.0000	0.0000	81.598	0.00000	0.00000	30350.5	19134.6	11215.9	S
336.911	0.0000	0.0000	81.483	0.00000	0.00000	30350.5	19134.6	11215.9	S
360.911	0.0000	0.0000	81.379	0.00000	0.00000	30350.5	19134.6	11215.9	S
384.911	0.0000	0.0000	81.286	0.00000	0.00000	30350.5	19134.6	11215.9	S
408.911	0.0000	0.0000	81.202	0.00000	0.00000	30350.5	19134.6	11215.9	S
432.911	0.0000	0.0000	81.125	0.00000	0.00000	30350.5	19134.6	11215.9	S
456.911	0.0000	0.0000	81.054	0.00000	0.00000	30350.5	19134.6	11215.9	S
480.911	0.0000	0.0000	80.989	0.00000	0.00000	30350.5	19134.6	11215.9	S
504.911	0.0000	0.0000	80.929	0.00000	0.00000	30350.5	19134.6	11215.9	S
528.911	0.0000	0.0000	80.872	0.00000	0.00000	30350.5	19134.6	11215.9	S
552.911	0.0000	0.0000	80.820	0.00000	0.00000	30350.5	19134.6	11215.9	S
576.911	0.0000	0.0000	80.771	0.00000	0.00000	30350.5	19134.6	11215.9	S
600.911	0.0000	0.0000	80.724	0.00000	0.00000	30350.5	19134.6	11215.9	S
624.911	0.0000	0.0000	80.681	0.00000	0.00000	30350.5	19134.6	11215.9	S
648.911	0.0000	0.0000	80.640	0.00000	0.00000	30350.5	19134.6	11215.9	S
672.911	0.0000	0.0000	80.601	0.00000	0.00000	30350.5	19134.6	11215.9	S
696.911	0.0000	0.0000	80.564	0.00000	0.00000	30350.5	19134.6	11215.9	S
720.911	0.0000	0.0000	80.529	0.00000	0.00000	30350.5	19134.6	11215.9	S
744.911	0.0000	0.0000	80.496	0.00000	0.00000	30350.5	19134.6	11215.9	<i>。 。 。 。</i>
768.911	0.0000	0.0000	80.464	0.00000	0.00000	30350.5	19134.6	11215.9	s
792.911	0.0000	0.0000	80.433			30350.5	19134.6	11215.9	N.A.

Detailed Results :: Scenario 15 :: 100yr-168hr post

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft <sup>3</sup> /s)	Cumulative Inflow Volume (ft²)	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>3</sup> )	Flor
0.000	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	N.A
0.022	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ű
0.044	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.067	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.089	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.111	0.0000	0.0000	78.500	0.00000	0.00000		0.0		ŭ
0.133	0.0000	0.0000	78.500	0.00000	0.00000	0.0		0.0	Ŭ
0.156	0.0000	0.0000				0.0	0.0	0.0	
			78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.178	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.200	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.222	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.244	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.267	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.289	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.311	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.333	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	0
0.356	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.378	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.400	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.422	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	U
0.444	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ū
0.467	0.0000	0.0000	78,500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
0.489	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.511	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
0.533	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.556	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.578	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.600	0.0000	0.0000	78.500	0.00000					
					0.00000	0.0	0.0	0.0	U
0.622	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.644	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.667	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.689	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.711	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.733	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.756	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.778	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.800	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.822	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.844	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
0.867	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ũ
0.889	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.911	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ũ
0.933	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.956	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
0.978	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.000	0.0000	0.0000	78.500	0.00000	0.00000	0.0		0.0	ŭ
	0.0000						0.0		
1.022		0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.044	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.067	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.089	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.111	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.133	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.156	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.178	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.200	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
1.222	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ũ
1.244	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ŭ
1.267	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ũ
1.289	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.311	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.333	0.0000	0.0000	78.500	0.00000	0.00000		0.0	0.0	ŭ
1.356	0.0000	0.0000				0.0			
			78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.378	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.400	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.422	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.444	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.467	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	U
1.489	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	Ū
1.511	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.533	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.556	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.578	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
1.600	0.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	ŭ
		0.0000	10.000	0.00000	0.00000	0.0	0.0	0.0	

Elapse Time (hours		Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ftª)	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flow Type
152.	933	0.3653	0.0000	84.699	0.05471	0.06611	24923.3	19037.1	743.3	s
152.		0.3653	0.0000	84.703	0.05491	0.06705	24952.5	19041.5	748.6	0
152.		0.3653	0.0000	84.708	0.05510	0.06799	24981.8	19045.9	754.0	0
153.		0.3654	0.0000	84.713	0.05529	0.06893	25011.0	19050.3		0
153.		0.3654	0.0000	84.717	0.05548	0.06987	25040.2	19054.8	759.5	S
153.0		0.3654	0.0000	84.722	0.05565	0.07081	25069.4		765.1	0
153.0		0.3655	0.0000	84.726	0.05583	0.07174		19059.2	770.7	S
153.0		0.3655	0.0000	84.731	0.05599	0.07268	25098.7	19063.7	776.4	S
153.1		0.3655	0.0000	84.736	0.05616	0.07361	25127.9	19068.2	782.2	S
153.		0.3656	0.0000	84.740	0.05631	0.07454	25157.2 25186.4	19072.6	788.0	S
153.		0.3656	0.0000	84.745	0.05647	0.07547		19077.1	793.9	S
153.1		0.3656	0.0000	84.749	0.05662		25215.6	19081.6	799.9	S
153.2		0.3657	0.0000	84.753	0.05676	0.07639 0.07732	25244.9	19086.2	806.0	S
153.2		0.3657	0.0000	84.758	0.05690	0.07824	25274.1 25303.4	19090.7	812.2	S
153.2		0.3657	0.0000	84.762	0.05704	0.07917		19095.3	818.4	S
153.2		0.3658	0.0000	84.767	0.05717	0.08009	25332.7 25361.9	19099.8	824.7	S
153.2		0.3658	0.0000	84.771	0.05730	0.08100		19104.4	831.1	S
153.3		0.3658	0.0000	84.775	0.05743	0.08192	25391.2	19109.0	837.5	S
153.3	333	0.3659	0.0000	84.779	0.05755		25420.4	19113.5	844.0	S
153.3		0.3659	0.0000	84.784	0.05767	0.08284 0.08375	25449.7 25479.0	19118.1	850.6	S
153.3		0.3659	0.0000	84.788	0.05779	0.08466		19122.8	857.3	S
153.4		0.3660	0.0000	84.792	0.05791	0.08557	25508.3	19127.4	864.0	S
153.4		0.3660	0.0000	84.796	0.05802	0.08647	25537.5 25566.8	19132.0	870.8	S
153.4		0.3660	0.0000	84.800	0.05813	0.08738	25596.1	19136.6	877.7	5
153.4		0.3661	0.0000	84.805	0.05823	0.08828	25625.4	19141.3 19145.9	884.7	S
153.4		0.3661	0.0000	84.809	0.05834	0.08918	25654.7	19150.6	891.7	0
153.5		0.3661	0.0000	84.813	0.05844	0.09008	25683.9	19155.3	898.8	20
153.5		0.3661	0.0000	84.817	0.05854	0.09097	25713.2	19160.0	906.0 913.2	\$
153.5		0.3662	0.0000	84.821	0.05863	0.09187	25742.5	19164.6	920.5	0
153.5		0.3662	0.0000	84.825	0.05873	0.09276	25771.8	19169.3	927.9	6
153.6	500	0.3662	0.0000	84.829	0.05882	0.09365	25801.1	19174.0	935.3	e
153.6	22	0.3663	0.0000	84.833	0.05891	0.09454	25830.4	19178.7	942.9	c
153.6		0.3663	0.0000	84.837	0.05899	0.09542	25859.7	19183.5	950.5	0
153.6	67	0.3663	0.0000	84.841	0.05908	0.09630	25889.0	19188.2	958.1	S
153.6	89	0.3664	0.0000	84.845	0.05916	0.09718	25918.3	19192.9	965.9	9
153.7	'11	0.3664	0.0000	84.848	0.05924	0.09806	25947.7	19197.7	973.7	SSS
153.7		0.3664	0.0000	84.852	0.05932	0.09894	25977.0	19202.4	981.6	S
153.7		0.3665	0.0000	84.856	0.05940	0.09981	26006.3	19207.1	989.5	S
153.7		0.3665	0.0000	84.860	0.05947	0.10068	26035.6	19211.9	997.5	~~~~~
153.8		0.3665	0.0000	84.864	0.05955	0.10155	26064.9	19216.7	1005.6	S
153.8		0.3666	0.0000	84.868	0.05962	0.10242	26094.2	19221.4	1013.8	S
153.8		0.3666	0.0000	84.871	0.05969	0.10328	26123.6	19226.2	1022.0	S
153.8		0.3666	0.0000	84.875	0.05976	0.10414	26152.9	19231.0	1030.3	S
153.8		0.3667	0.0000	84.879	0.05982	0.10500	26182,2	19235.8	1038.7	S
153.9		0.3667	0.0000	84.882	0.05989	0.10586	26211.6	19240.5	1047.1	S
153.9		0.3667	0.0000	84.886	0.05995	0.10671	26240.9	19245.3	1055.6	S
153.9		0.3668	0.0000	84,890	0.06001	0.10756	26270.2	19250.1	1064.2	S
153.9		0.3668	0.0000	84.893	0.06007	0.10841	26299.6	19254.9	1072.8	S
154.0		0.3668	0.0000	84.897	0.06013	0.10926	26328.9	19259.8	1081.5	S
154.0		0.3668	0.0000	84.900	0.06019	0.11010	26358.3	19264.6	1090.3	S
154.0		0.3669	0.0000	84.904	0.06025	0.11094	26387.6	19269.4	1099.2	S
154.0		0.3669	0.0000	84.908	0.06030	0.11178	26417.0	19274.2	1108.1	S
154.0		0.3669	0.0000	84.911	0.06035	0.11262	26446.3	19279.0	1117.0	S
154.1		0.3670	0.0000	84.915	0.06041	0.11345	26475.7	19283.9	1126.1	S
154.1 154.1		0.3670	0.0000	84.918	0.06046	0.11428	26505.0	19288.7	1135.2	S
		0.3670	0.0000	84.922	0.06051	0.11511	26534.4	19293.5	1144.4	S
154.1		0.3671	0.0000	84.925	0.06055	0.11594	26563.8	19298.4	1153.6	S
154.2		0.3671	0.0000	84.928	0.06060	0.11676	26593.1	19303.2	1162.9	S
154.2		0.3671	0.0000	84.932	0.06065	0.11758	26622.5	19308.1	1172.3	S
154.24		0.3671	0.0000	84.935	0.06069	0.11840	26651.9	19312.9	1181.7	S
154.20		0.3672	0.0000	84.939	0.06073	0.11921	26681.2	19317.8	1191.2	S
		0.3672	0.0000	84.942	0.06078	0.12003	26710.6	19322.6	1200.8	S
154.3 154.3	22	0.3672	0.0000	84.945	0.06082	0.12084	26740.0	19327.5	1210.4	S
		0.3673	0.0000	84.949	0.06086	0.12165	26769.4	19332.4	1220.1	S
154.3			0.0000	84.952	0.06090	0.12245	26798.8	19337.2	1229.9	~~~~
154.37		0.3673	0.0000	84.955	0.06093	0.12325	26828.1	19342.1	1239.7	S
154.40		0.3674	0.0000	84.959	0.06097	0.12405	26857.5	19347.0	1249.6	S
154.44		0.3674	0.0000	84.962	0.06101	0.12485	26886.9	19351.9	1259.6	S
		0.3674	0.0000	84.965	0.06104	0.12565	26916.3	19356.8	1269.6	S
154.46		0.3674	0.0000	84.968	0.06108	0.12644	26945.7	19361.6	1279.7	S
154.48		0.3675	0.0000	84.971	0.06111	0.12723	26975.1	19366.5	1289.8	S
154.5		0.3675	0.0000	84.975	0.06114	0.12802	27004.5	19371.4	1300.0	S
154.55		0.3675	0.0000	84.978	0.06117	0.12880	27033.9	19376.3	1310.3	S
104.00		0.0070	0.0000	84.981	0.06120	0.12958	27063.3	19381.2	1320.6	S

Detailed Results (cont.d.) :: Scenario 15 :: 100yr-168hr post

:: Scenario 15 :: 100yr-168hr post

Detailed Results (cont,d.)

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )
167.733	0.0701	0.0000	84.800	0.02693	0.08737	36508.5	21486.8	9451.7
167.756	0.0701	0.0000	84.800	0.02692	0.08719	36514.2	21488.9	9458.7
167.778	0.0701	0.0000	84.799	0.02692	0.08701	36519.8	21491.1	9465.7
167.800	0.0701	0.0000	84.798	0.02691	0.08684	36525.4	21493.2	9472.6
167.822	0.0701	0.0000	84.797	0.02690	0.08666	36531.0	21495.4	9479.6
167.845	0.0701	0.0000	84.796	0.02690	0.08648	36536.6	21495.4	9486.5
167.867	0.0701	0.0000	84.796	0.02689	0.08630	36542.2		9493.4
167.889	0.0701						21499.7	
		0.0000	84.795	0.02688	0.08613	36547.8	21501.8	9500.3
167.911	0.0701	0.0000	84.794	0.02688	0.08595	36553.4	21504.0	9507.2
167.933	0.0701	0.0000	84.793	0.02687	0.08578	36559.0	21506.1	9514.1
167.956	0.0701	0.0000	84.792	0.02687	0.08561	36564.7	21508.3	9520.9
167.978	0.0701	0.0000	84.792	0.02686	0.08543	36570.3	21510.4	9527.8
168.000	0.0701	0.0000	84.791	0.02685	0.08526	36575.9	21512.6	9534.6
168.022	0.0693	0.0000	84.790	0.02684	0.08509	36581.5	21514.7	9541.4
168.044	0.0671	0.0000	84.789	0.02682	0.08491	36586.9	21516.9	9548.2
168.067	0.0627	0.0000	84.788	0.02677	0.08472	36592.1	21519.0	9555.0
168.089	0.0561	0.0000	84.787	0.02671	0.08451	36596.9	21521.1	9561.8
168.111	0.0491	0.0000	84.786					
				0.02662	0.08427	36601.1	21523.3	9568.5
168.133	0.0424	0.0000	84.785	0.02648	0.08401	36604.7	21525.4	9575.3
168.156	0.0366	0.0000	84.784	0.02636	0.08372	36607.9	21527.5	9582.0
168.178	0.0317	0.0000	84.782	0.02623	0.08341	36610.6	21529.6	9588.6
168.200	0.0276	0.0000	84.781	0.02609	0.08309	36613.0	21531.7	9595.3
168.222	0.0241	0.0000	84.779	0.02595	0.08275	36615.0	21533.8	9601.9
168.244	0.0212	0.0000	84.777	0.02581	0.08241	36616.9	21535.9	9608.5
168.267	0.0186	0.0000	84.776	0.02567	0.08205	36618.4	21537.9	9615.1
168.289	0.0163	0.0000	84.774	0.02553	0.08169	36619.8	21540.0	9621.7
168.311	0.0144	0.0000	84.772	0.02539	0.08132	36621.1	21542.0	9628.2
168.333	0.0125	0.0000	84.771	0.02526	0.08094	36622.1	21544.0	9634.7
168.356	0.0110	0.0000	84.769	0.02512	0.08056	36623.1	21546.1	9641.1
168.378	0.0096	0.0000	84.767	0.02500	0.08018	36623.9	21548.1	9647.6
	0.0083	0.0000						
168.400	0.0083	0.0000	84.765	0.02487	0.07979	36624.6	21550.1	9654.0
168.422	0.0072	0.0000	84.763	0.02475	0.07940	36625.3	21552.0	9660.3
168.444	0.0063	0.0000	84.761	0.02463	0.07901	36625.8	21554.0	9666.7
168.467	0.0054	0.0000	84.760	0.02451	0.07862	36626.3	21556.0	9673.0
168.489	0.0046	0.0000	84.758	0.02440	0.07823	36626.7	21557.9	9679.3
168.511	0.0039	0.0000	84.756	0.02429	0.07784	36627.0	21559.9	9685.5
168.533	0.0033	0.0000	84.754	0.02418	0.07744	36627.3	21561.8	9691.7
168.556	0.0027	0.0000	84.752	0.02407	0.07705	36627.5	21563.8	9697.9
168.578	0.0023	0.0000	84.750	0.02397	0.07666	36627.7	21565.7	9704.0
168.600	0.0019	0.0000	84.748	0.02388	0.07627	36627.9	21567.6	9710.2
168.622	0.0015	0.0000	84.747	0.02378	0.07588	36628.0	21569.5	9716.2
168.644	0.0012	0.0000	84.745	0.02369	0.07549	36628.1	21571.4	9722.3
168.667	0.0009	0.0000	84.743	0.02360	0.07510	36628.2	21573.3	9728.3
168.689	0.0007	0.0000	84.741	0.02352	0.07471	36628.3	21575.2	9734.3
168.711	0.0005	0.0000	84.739					
				0.02343	0.07433	36628.3	21577.0	9740.3
168.733	0.0004	0.0000	84.737	0.02335	0.07394	36628.4	21578.9	9746.2
168.756	0.0002	0.0000	84.735	0.02327	0.07356	36628.4	21580.8	9752.1
168.778	0.0002	0.0000	84.734	0.02320	0.07318	36628.4	21582.6	9758.0
168.800	0.0001	0.0000	84.732	0.02313	0.07280	36628.4	21584.5	9763.8
168.822	0.0000	0.0000	84.730	0.02306	0.07242	36628.4	21586.3	9769.6
168.845	0.0000	0.0000	84.728	0.02299	0.07205	36628.4	21588.2	9775.4
168.867	0.0000	0.0000	84.726	0.02292	0.07168	36628.4	21590.0	9781.2
168.889	0.0000	0.0000	84.724	0.02285	0.07130	36628.4	21591.9	9786.9
168.911	0.0000	0.0000	84.723	0.02279	0.07094	36628.4	21593.7	9792.6
192.911	0.0000	0.0000	84.248	-0.00094	0.00200	36628.4	20262.8	12943.3
216.911	0.0000	0.0000	83.867	0.01230	0.00000	36628.4	21430.5	13029.6
240.911	0.0000	0.0000	83.530	0.00980	0.00000	36628.4	22387.3	13029.6
264.911	0.0000	0.0000	83.226	D.00701				
	0.0000				0.00000	36628.4	23124.1	13029.6
288.911		0.0000	82.943	0.00275	0.00000	36628.4	23598.9	13029.6
312.911	0.0000	0.0000	82.670	0.00000	0.00000	36628.4	23598.9	13029.6
336.911	0.0000	0.0000	82.449	0.00000	0.00000	36628.4	23598.9	13029.6
360.911	0.0000	0.0000	82.263	0.00000	0.00000	36628.4	23598.9	13029.6
384.911	0.0000	0.0000	82.103	0.00000	0.00000	36628.4	23598.9	13029.6
408.911	0.0000	0.0000	81.963	0.00000	0.00000	36628.4	23598.9	13029.6
432.911	0.0000	0.0000	81.838	0.00000	0.00000	36628.4	23598.9	13029.6
456.911	0.0000	0.0000	81.727	0.00000	0.00000	36628.4	23598.9	13029.6
480.911	0.0000	0.0000	81.627	0.00000	0.00000	36628.4	23598.9	13029.6
504.911	0.0000	0.0000	81.535	0.00000	0.00000	36628.4	23598.9	13029.6
528.911	0.0000			0.00000				
		0.0000	81.452		0.00000	36628.4	23598.9	13029.6
552.911	0.0000	0.0000	81.375	0.00000	0.00000	36628.4	23598.9	13029.6
576.911	0.0000	0.0000	81.304	0.00000	0.00000	36628.4	23598.9	13029.6
600.911	0.0000	0.0000	81.238	0.00000	0.00000	36628.4	23598.9	13029.6
624.911	0.0000	0.0000	81.176	0.00000	0.00000	36628.4	23598.9	13029.6
648.911	0.0000	0.0000	81.118	0.00000	0.00000	36628.4	23598.9	13029.6

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ftª)	Flov Typ
672.911	0.0000	0.0000	81.064	0.00000	0.00000	36628.4	23598.9	13029.6	S
696.911	0.0000	0.0000	81.013	0.00000	0.00000	36628.4	23598.9	13029.6	s
720.911	0.0000	0.0000	80.965	0.00000	0.00000	36628.4	23598.9	13029.6	Š
744.911	0.0000	0.0000	80.920	0.00000	0.00000	36628.4	23598.9	13029.6	s
768.911	0.0000	0.0000	80.877	0.00000	0.00000	36628.4	23598.9	13029.6	S
792.911	0.0000	0.0000	80.836	0.00000	0.00000	36628.4	23598.9	13029.6	S
816.911	0.0000	0.0000	80.797	0.00000	0.00000	36628.4	23598.9	13029.6	ŝ
840.911	0.0000	0.0000	80,760	0.00000	0.00000	36628.4	23598.9	13029.6	5
864.911	0.0000	0.0000	80,725	0.00000	0.00000	36628.4	23598.9	13029.6	ŝ
888.911	0.0000	0.0000	80.691		0.00000	36628.4	23598.9	13029.6	N.

:: Scenario 16 :: 100yr-240hr post

Inflow Elapsed Outside Stage Infiltration Overflow Cumulative Cumulative Cumulative Recharge (ft/day) Elevation Time Rate Rate Discharge Inflow Infiltration Discharge Flow (hours) (ft3/s) (ft datum) (ft³/s) (ft<sup>2</sup>/s) Volume (ft<sup>3</sup>) Volume (fl<sup>3</sup>) Volume (ft3) Type 0.00000 0.000 0.0000 0.0000 78 500 0.00000 0.0 0.0 0.0 N.A. 0.022 0.0000 0.0000 78.500 0.00000 0,0 0.0 0.0 υ 0.044 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 υ 0.067 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 U 0.0 0.089 0.0000 0.0000 0.00000 0.00000 78.500 0.0 0.0 0.0 υ 0.0000 0.0000 0.0000 0.0000 0.111 0.0000 78.500 0.00000 0.00000 0.0 0.0 ũ 0.0 0.133 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 ũ 0.0000 0.156 78.500 0.00000 0.00000 0.0 0.0 0.0 Ũ 0.0000 0.178 78,500 0.00000 0.00000 0.0 0.0 0.0 U 0.0000 0.200 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 U 0.222 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 UUU 0.244 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.267 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 UUU 0.289 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.311 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.333 0.356 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 Ũ 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 U 0.0000 0.378 78.500 0.00000 0.00000 0.00000 0.00000 0.0 0.0 0.0 000 0.400 0.0000 0.0000 0.00000 0.0 0.0 0.0 0.422 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 0.444 0.0000 0.0000 0.00000 78.500 0.00000 0.0 0.0 0.0 0000 0.467 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.489 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.511 0.533 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 78.500 0.00000 0.00000 0.0 0.0 0.0 UUU 0.556 78.500 0.00000 0.00000 0.0 0.0 0.0 0.578 0.0000 0.0000 78,500 0.0 0.0 0.0 0.0000 0.00000 0.600 0.0000 78.500 0.00000 0.0 0.0 0.0 0.622 0.0000 0.0000 0.00000 0.0 78.500 0.00000 0.0 0.644 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.667 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.689 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.711 0.733 78.500 0.00000 0.00000 0.0 0.0 0.0 78.500 78.500 0.00000 0.00000 0.00000 0.00000 0.0 0.0 0.0 0.756 0.0000 0.0000 0.00000 0.0 0.0 0.0 0.778 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 0.800 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.822 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.844 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0000 0.0000 0.0000 0.867 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.889 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.911 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.933 0.00000 0.0 0.0 0.0 0.0000 0.00000 0.956 0.0000 78.500 0.0 0.0 0.0 0.978 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 1.000 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.022 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.044 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.067 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.089 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.0000 0.00000 1.111 78.500 0.00000 0.0 0.0 0.0 0.0000 1.133 78.500 0.00000 0.0 0.0 0.0 1.156 0.0000 0.0000 78.500 0.00000 0.0 0.0 0.0 1.178 0.0000 0.0000 78.500 0.00000 0.00000 0.0 1.200 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 1.222 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 1.244 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.267 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 0.0000 1.289 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.00000 1.311 78,500 0.00000 0.0 0.0 0.0 1.333 0.0000 78.500 0.0 0.0 0.0 0,0000 0.00000 1.356 0.0000 78.500 0.00000 0.0 0.0 1.378 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 1.400 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 1.422 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.444 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.467 0.0000 0.0000 78,500 0.00000 0.00000 0.0 0.0 0.0 0.0000 1.489 78.500 0.00000 0.00000 0.0 0.0 0.0 0.0000 1.511 78,500 0.00000 0.00000 0.0 0.0 0.0 0.0000 0.0000 78.500 0.00000 1.533 0.0 0.0 0.0 1.556 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.578 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.600 0.0000 0.0000 78.500 0.00000 0.00000 0.0 0.0 0.0 1.622 0.0000 0.0000 78.500 0.0 ũ 0.00000 0.00000 0.0 0.0

Tranmission Doctor

**Detailed Results** 

:: Scenario 16 :: 100yr-240hr post

Detailed Results (cont,d.)

lapsed Time hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>a</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )
179.244	0.4796	0.0000	84.863	0.08642	0.10140	23775.5	17598.4	337.3
179.267	0.4797	0.0000	84.868	0.08643	0.10263	23813.9	17605.3	345.5
179.289	0.4797	0.0000	84.874	0.08644	0.10386	23852.3	17612.2	353.7
179.311	0.4798	0.0000	84.879	0.08645	0.10509	23890.7	17619.2	362.1
179.333	0.4799	0.0000	84.884	0.08646	0.10631	23929.1	17626.1	370.5
179.356	0.4799	0.0000	84.890	0.08647	0.10753	23967.4	17633.0	379.1
179.378	0.4800	0.0000	84.895	0.08647	0.10875	24005.8	17639.9	387.7
179.400	0.4800	0.0000	84.900	0.08648	0.10997	24044.2	17646.8	396.5
179.422	0.4801	0.0000	84.905	0.08648	0.11118	24082.6	17653.7	405.3
179.444	0.4802	0.0000	84.910	0.08649	0.11239			
179.467	0.4802	0.0000	84.915			24121.1	17660.7	414.3
179.489	0.4802	0.0000		0.08649	0.11360	24159.5	17667.6	423.3
			84.920	0.08649	0.11481	24197.9	17674.5	432.4
179.511	0.4804	0.0000	84.925	0.08649	0.11601	24236.3	17681.4	441.7
179.533	0.4804	0.0000	84.930	0.08649	0.11721	24274.8	17688.3	451.0
179.556	0.4805	0.0000	84.935	0.08649	0.11841	24313.2	17695.3	460.4
179.578	0.4805	0.0000	84.940	0.08649	0.11961	24351.6	17702.2	470.0
179.600	0.4806	0.0000	84.945	0.08648	0.12080	24390.1	17709.1	479.6
179.622	0.4807	0.0000	84.950	0.08648	0.12199	24428.5	17716.0	489.3
179.644	0.4807	0.0000	84.955	0.08648	0.12318	24467.0	17722.9	499.1
179.667	0.4808	0.0000	84.960	0.08647	0.12437	24505.4	17729.9	509.0
179.689	0.4809	0.0000	84.965	0.08646	0.12555	24543.9	17736.8	519.0
179.711	0.4809	0.0000	84.969	0.08646	0.12673	24582.4	17743.7	529.1
179.733	0.4810	0.0000	84.974	0.08645	0.12790	24620.9	17750.6	539.3
179.756	0.4810	0.0000	84.979	0.08644	0.12908	24659.3	17757.5	549.5
179.778	0.4811	0.0000	84.984	0.08643	0.13025	24697.8	17764.4	559.9
179.800	0.4812	0.0000	84.988	0.08642	0.13141	24736.3	17771.3	570.4
179.822	0.4812	0.0000	84.993	0.08641	0.13258	24774.8	17778.3	580.9
179.845	0.4813	0.0000	84.997	0.08640	0.13374	24813.3	17785.2	591.6
179.867	0.4813	0.0000	85.002	0.08638				
179.889	0.4814	0.0000	85.002		0.13490	24851.8	17792.1	602.3
179.009				0.08637	0.13605	24890.3	17799.0	613.2
179.911	0.4815	0.0000	85.011	0.08636	0.13720	24928.8	17805.9	624.1
179.933	0.4815	0.0000	85.016	0.08634	0.13835	24967.4	17812.8	635.1
179.956	0.4816	0.0000	85.020	0.08633	0.13950	25005.9	17819.7	646.2
179.978	0.4816	0.0000	85.025	0.08631	0.14064	25044.4	17826.6	657.4
180.000	0.4817	0.0000	85.029	0.08629	0.14178	25082.9	17833.5	668.7
180.022	0.4817	0.0000	85,034	0.08628	0.14291	25121.5	17840.4	680.1
180.044	0.4818	0.0000	85.038	0.08626	0.14405	25160.0	17847.3	691.6
180.067	0.4819	0.0000	85.042	0.08624	0.14517	25198.6	17854.2	703.2
180.089	0.4819	0.0000	85.047	0.08622	0.14630	25237.1	17861.1	714.8
180.111	0.4820	0.0000	85.051	0.08620	0.14742	25275.7	17868.0	726.6
180.133	0.4820	0.0000	85.055	0.08618	0.14854	25314.2	17874.9	738.4
180.156	0.4821	0.0000	85.060	0.08616	0.14966	25352.8	17881.8	750.4
180.178	0.4822	0.0000	85.064	0.08614	0.15077	25391.4	17888.7	762.4
180.200	0.4822	0.0000	85.068	0.08611	0.15188	25429.9	17895.6	774.5
180.222	0.4823	0.0000	85.072	0.08609	0.15299	25468.5	17902.5	786.7
180.244	0.4823	0.0000	85.076	0.08607	0.15409			
180.267	0.4824	0.0000	85.081	0.08604	0.15409	25507.1	17909.4	799.0
						25545.7	17916.3	811.3
180.289	0.4824	0.0000	85.085	0.08602	0.15628	25584.3	17923.1	823.8
180.311	0.4825	0.0000	85.089	0.08599	0.15738	25622.9	17930.0	836.3
180.333	0.4825	0.0000	85.093	0.08597	0.15847	25661.5	17936.9	849.0
180.356	0.4826	0.0000	85.097	0.08594	0.15955	25700.1	17943.8	861.7
180.378	0.4827	0.0000	85.101	0.08592	0.16063	25738.7	17950.7	874.5
180.400	0.4827	0.0000	85.105	0.08589	0.16171	25777.3	17957.5	887.4
180.422	0.4828	0.0000	85.109	0.08586	0.16279	25815.9	17964.4	900.4
180.444	0.4828	0.0000	85.113	0.08583	0.16386	25854.6	17971.3	913.4
180.467	0.4829	0.0000	85.117	0.08581	0.16493	25893.2	17978.1	926.6
180.489	0.4829	0.0000	85.121	0.08578	0.16599	25931.8	17985.0	939.8
180.511	0.4830	0.0000	85.125	0.08575	0.16705	25970.5	17991.9	953.1
180.533	0.4830	0.0000	85.129	0.08572	0.16811	26009.1	17998.7	966.6
180.556	0.4831	0.0000	85.133	0.08569	0.16917	26047.7	18005.6	980.0
180.578	0.4832	0.0000	85.137	0.08566	0.17022	26086.4	18012.4	993.6
180.600	0.4832	0.0000		0.08563	0.17022			
			85.140			26125.0	18019.3	1007.3
180.622	0.4833	0.0000	85.144	0.08559	0.17231	26163.7	18026.1	1021.0
180.644	0.4833	0.0000	85.148	0.08556	0.17335	26202.4	18033.0	1034.8
180.667	0.4834	0.0000	85.152	0.08553	0.17439	26241.0	18039.8	1048.8
180.689	0.4834	0.0000	85.156	0.08550	0.17542	26279.7	18046.7	1062.7
180.711	0.4835	0.0000	85.159	0.08546	0.17645	26318.4	18053.5	1076.8
180.733	0.4835	0.0000	85,163	0.08543	0.17748	26357.1	18060.3	1091.0
180.756	0.4836	0.0000	85.167	0.08540	0.17850	26395.8	18067.2	1105.2
180.778	0.4836	0.0000	85.170	0.08536	0.17952	26434.4	18074.0	1119.5
180.800	0.4837	0.0000	85.174	0.08533	0.18054	26473.1	18080.8	1133.9
180.822	0.4838	0.0000	85.178	0.08529	0.18155	26511.8	18087.6	1148.4
180.845	0.4838	0.0000	85.181	0.08529	0.18155	26550.5	18094.5	1148.4
						20000.0		

:: Scenario 16 :: 100yr-240hr post

lapsed Time (hours)	Inflow Rate (ft <sup>3</sup> /s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>3</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flor Typ
240.089	0.0202	0.0000	84.291	0.02149	0.00511	42375.7	24650.8	14152.6	S
240.111	0.0177	0.0000	84.290	0.02146	0.00510	42377.2	24652.5	14153.0	S
240.133	0.0153	0.0000	84.290	0.02142	0.00508	42378.5	24654.2	14153.4	S
240.156	0.0132	0.0000	84.290	0.02138	0.00506	42379.7	24655.9	14153.8	0000000000000
240.178	0.0114	0.0000	84.290	0.02133	0.00503	42380.7	24657.6	14154.2	š
240.200	0.0099	0.0000	84.289	0.02127	0.00500	42381.5	24659.3	14154.6	S
240.222	0.0087	0.0000	84.289	0.02122	0.00497	42382.3	24661.0	14155.0	S
240.244	0.0076	0.0000	84.288	0.02117	0.00494	42382.9	24662.7	14155.4	S
240.267	0.0067	0.0000	84.288	0.02111	0.00490	42383.5	24664.4	14155.8	S
240.289	0.0059	0.0000	84.288	0.02106	0.00486	42384.0	24666.1	14156.2	S
240.311	0.0052	0.0000	84.287	0.02100	0.00483	42384.4	24667.8	14156.6	S
240.333	0.0045	0.0000	84.287	0.02095	0.00479	42384.8	24669.5	14157.0	S
240.356	0.0040	0.0000	84.286	0.02089	0.00475	42385,2	24671.1	14157.4	S
240.378	0.0035	0.0000	84.286	0.02084	0.00471	42385.5	24672.8	14157.8	S
240.400	0.0030	0.0000	84.285	0.02079	0.00467	42385.7	24674.5	14158.1	S
240.422	0.0026	0.0000	84.285	0.02074	0.00462	42385.9	24676.1	14158.5	S
240.444	0.0023	0.0000	84.284	0.02069	0.00458	42386.1	24677.8	14158.9	S
240.467	0.0019	0.0000	84.284	0.02065	0.00454	42386.3	24679.4	14159.2	S
240.489	0.0017	0.0000	84.283	0.02060	0.00450	42386.4	24681.1	14159.6	50000
240.511	0.0014	0.0000	84.283	0.02055	0.00445	42386.6	24682.7	14160.0	
240.533	0.0012	0.0000	84,282	0.02051	0.00441	42386.7	24684.4	14160.3	S
240.556	0.0010 0.0008	0.0000	84.282	0.02047	0.00437	42386.8	24686.0	14160.7	S
240.578 240.600	0.0008	0.0000	84.281	0.02043	0.00433	42386.8	24687.7	14161.0	S
240.600	0.0005	0.0000	84.280 84.280	0.02039	0.00428	42386.9	24689.3	14161.3	S
240.622	0.0004	0.0000	84.279	0.02035 0.02031	0.00424 0.00420	42386.9 42387.0	24690.9	14161.7	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
240.667	0.0003	0.0000	84.279	0.02037	0.00420	42387.0	24692.5 24694.2	14162.0 14162.4	5
240.689	0.0003	0.0000	84.278	0.02024	0.00413	42387.0	24695.8	14162.7	0
240.711	0.0002	0.0000	84.278	0.02020	0.00407	42387.0	24697.4	14163.0	00
240.733	0.0001	0.0000	84.277	0.02017	0.00403	42387.1	24699.0	14163.3	0
240.756	0.0001	0.0000	84.277	0.02013	0.00399	42387.1	24700.6	14163.7	s
240.778	0.0001	0.0000	84.276	0.02010	0.00394	42387.1	24702.2	14164.0	S
240.800	0.0000	0.0000	84.276	0.02007	0.00390	42387.1	24703.8	14164.3	s
240.822	0.0000	0.0000	84.275	0.02004	0.00386	42387.1	24705.4	14164.6	S
240.845	0.0000	0.0000	84.275	0.02001	0.00382	42387.1	24707.0	14164.9	S
240.867	0.0000	0.0000	84.274	0.01998	0.00378	42387.1	24708.6	14165.2	S
240.889	0.0000	0.0000	84.273	0.01995	0.00374	42387.1	24710.2	14165.5	S
240.911	0.0000	0.0000	84.273	0.01993	0.00370	42387.1	24711.8	14165.8	S
264.911	0.0000	0.0000	83.904	0.01157	0.00000	42387.1	25779.1	14325.5	s
288.911	0.0000	0.0000	83.582	0.00959	0.00000	42387.1	26711.5	14325.5	S
312.911	0.0000	0.0000	83.292	0.00748	0.00000	42387.1	27436.1	14325.5	S
336.911	0.0000	0.0000	83.029	0.00362	0.00000	42387.1	28003.9	14325.5	S
360.911	0.0000	0.0000	82.762	0.00033	0.00000	42387.1	28061.6	14325.5	S
384.911	0.0000	0.0000	82.540	0.00000	0.00000	42387.1	28061.6	14325.5	S
408.911	0.0000	0.0000	82.354	0.00000	0.00000	42387.1	28061.6	14325.5	S
432.911	0.0000	0.0000	82.193	0.00000	0.00000	42387.1	28061.6	14325.5	S
456.911	0.0000	0.0000	82.052	0.00000	0.00000	42387.1	28061.6	14325.5	S
480.911	0.0000	0.0000	81.927	0.00000	0.00000	42387.1	28061.6	14325.5	000000
504.911	0.0000	0.0000	81.815	0.00000	0.00000	42387.1	28061.6	14325.5	S
528.911	0.0000	0.0000	81.713	0.00000	0.00000	42387.1	28061.6	14325.5	S
552.911	0.0000	0.0000	81.621	0.00000	0.00000	42387.1	28061.6	14325.5	S
576.911	0.0000	0.0000	81.536	0.00000	0.00000	42387.1	28061.6	14325.5	S
600.911 624.911	0.0000		81.458	0.00000	0.00000	42387.1	28061.6	14325.5	S
648.911	0.0000	0.0000	81.386	0.00000	0.00000	42387.1	28061.6	14325.5	S
672.911	0.0000	0.0000	81.319	0.00000	0.00000	42387.1	28061.6	14325.5	S
696.911	0.0000	0.0000	81.256	0.00000	0.00000	42387.1	28061.6	14325.5	S
720.911	0.0000	0.0000	81.197 81.142	0.00000	0.00000	42387.1 42387.1	28061.6	14325.5	S
744.911	0.0000	0.0000	81.090	0.00000	0.00000	42387.1	28061.6	14325.5	0
768.911	0.0000	0.0000	81.041	0.00000	0.00000	42387.1	28061.6 28061.6	14325.5 14325.5	S
792.911	0.0000	0.0000	80.995	0.00000	0.00000	42387.1	28061.6	14325.5	0
816.911	0.0000	0.0000	80.951	0.00000	0.00000	42387.1	28061.6	14325.5	S
840.911	0.0000	0.0000	80.909	0.00000	0.00000	42387.1	28061.6	14325.5	0
864.911	0.0000	0.0000	80.870	0.00000	0.00000	42387.1	28061.6	14325.5	S
888.911	0.0000	0.0000	80.832	0.00000	0.00000	42387.1	28061.6	14325.5	S
912.911	0.0000	0.0000	80.796	0.00000	0.00000	42387.1	28061.6	14325.5	s
936.911	0.0000	0.0000	80.761	0.00000	0.00000	42387.1	28061.6	14325.5	s
960.911	0.0000	0.0000	80.728			42387.1	28061.6	14325.5	N.A

Detailed Results (cont,d.)

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft <sup>a</sup> )	Cumulative Infiltration Volume (ft <sup>3</sup> )	Cumulative Discharge Volume (ft <sup>a</sup> )	Flov Typ
0.000	515.0000	0.0000	78.500	0.00000	0.00000	0.0	0.0	0.0	N.A
0.002	515.0000	0.0000	84.152	0.39464	0.00000	3090.0	2.4	0.0	U/
2.400	0.0000	0.0000	83.143	0.19431	0.00000	3090.0	2797.7	0.0	U/
6.000	0.0000	0.0000				3090.0	3090.0	0.0	dr
12.000	0.0000	0.0000		-		3090.0	3090.0	0.0	d
24.000	0.0000	0.0000			increase.	3090.0	3090.0	0.0	d
36.000	0.0000	0.0000				3090.0	3090.0	0.0	di
48.000	0.0000	0.0000				3090.0	3090.0	0.0	d
60.000	0.0000	0.0000				3090.0	3090.0	0.0	d
72.000	0.0000	0.0000	in the second se			3090.0	3090.0	0.0	d
84.000	0.0000	0.0000				3090.0	3090.0	0.0	d
96.000	0.0000	0.0000				3090.0	3090.0	0.0	d



## Worksheet for Needed Fire Flow

Project Name :	Transmission Doctor
Customer / Builder Name :	M&M Properties of Alachua
Property Address :	14616 NW 128th Terrace
	Alachua, FL 32615

## **ISO** Calculation

NFF = (C)(O)(1+(X+P))		
NFF = the needed fire flow in gallons per minute (gpm)	=	692 gpm
C = a factor related to the type of construction	=	815
O = a factor related to the type of occupancy	=	0.85
X = a factor related to the exposure buildings	=	0.00
P = a factor related to the communication between buildings	=	0
C = 18F(A)^0.5	=	815
F = coefficient related to the class of construction	=	0.8
A= effective area	=	3,200

There are no exposure buildings on the project site.

Per GRU Standards ISO fire flow available must be at least 1,500 gpm.

REQUIRED Needed Fire Flow	1,500	gpm
	.,	9011



From: Rodolfo Valladares [mailto:ro\_valladares@cityofalachua.org]
Sent: Thursday, April 6, 2017 10:32 AM
To: Sergio Reyes <<u>sreyes@edafl.com</u>>
Cc: Adam Hall <<u>ad\_hall@cityofalachua.org</u>>; Stephanie Sutton <<u>ssutton@edafl.com</u>>
Subject: Re: Transmission Doctor

Mr. Reyes,

In reference to: Transmission Doctor Site Plan Application; 14616 NW 128th Terrace.

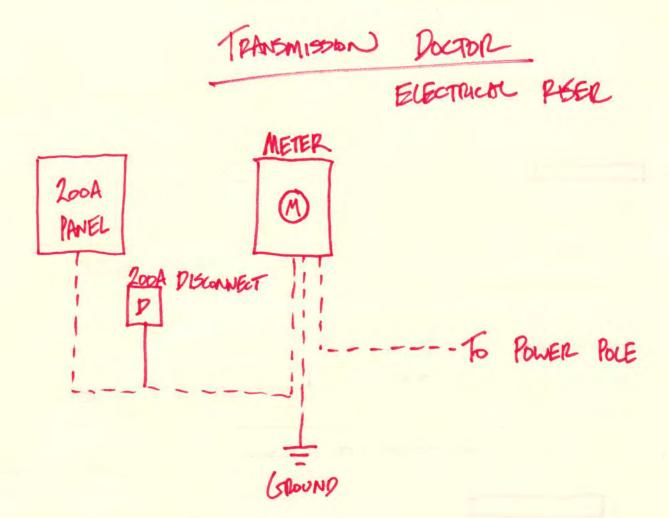
As noted by Adam Hall and in accordance with the City of Alachua (CoA) Requirements for Design and Construction, the CoA encourages the Engineer to meet with CoA Public Services Department with the purpose to firmly establish, for City approval, potable water capacity and fire flow requirements/needs.

Per our conversation/coordination, the potable water demand and fire flow requirements for the new facility will not exceed to-date demand of the existing facility. Given the design criteria noted, potable water may be provided at the proposed development site.

Regards,

Rodolfo Valladares, P.E. Public Service Director City of Alachua 386.418.6140





Hi James,

Clay is right about our general needs for the building. We will need the footprint and door locations/utility connections as soon as possible to finish our site plan (and will need elevations just prior to the city submittal).

As Clay mentioned, the city only takes submittals once a month. To meet the 2/28 deadline we would need the footprint and doors/utility info next week, and elevations by 2/27. Is that possible for you or should we plan on the end of March? Let us know what your projected schedule looks like-

I've pasted our general building information checklist below...I realize that some of these items may not apply for this project (sprinkler system, natural gas, communications), but it gives you a good starting point. We can fill in N/A for any items that don't apply.

Information Required	Received On:	Notes:
Footprint	V,	40×80 + 16×8 SHED
Floor Plans	VI	
Elevations		1
Dumpster Enclosure		NA
Construction Class	(2)	1.
Construction Type		11-B , , , , , , , , , , , , , , , , , , ,
Square Footage (Per Floor)	1101	3,200 (15T), 717 (MEZZ)
Square Footage (Area Adjustment – If Required)		
Roof Plan	GABLE 1:13	2 SUPPE
Building Height		19'-8"
Uses		AUTO REPAIR
Finished Floor (steps, flat, etc.)		FLAT

Information Required	Received On:	Notes:
Preferred Mechanical Connection Locations		
Water (Location, quantity of meters, sizes)	2" LINE	I-METER
Gravity Sewer/Septic (size, location, invert)		SEPTIC
<ul> <li>Grease Trap/Lint Trap/Chemical Neutralizer/Sump Pump (size, location, inverts)</li> </ul>	1	NA
Electric Loads	Cart	200AMP PANEL
Electric Phase		WHATEVER IS ONSITE -
Electric Riser Diagram		
Fire Sprinkler Flow (size, location, invert)	X	NA
<ul> <li>Air Conditioning Condensate Drain (locations, size)</li> </ul>		ON MERSON REAR LOPP
Natural Gas (and Loads)		NA
Communications		TEL/CABLE
Water Usages		STANDARD
Roof Downspout Location (location and size)	(4	LOCATED ON LOWIS SIDE

#### **VOLTAIRE ARCHITECTURAL WALL PACK** CATALOG #: TYPF LED PROJECT: NOTES VWP DBZ - SDGL - OPTIONS -UNV EXAMPLE н - T3 -DIM -L30/740 \_ w TYPE LUMEN DISTRIBUTION FINISH SHIELDING OPTIONS DRIVER VOLTAGE SERIES CRI & PACKAGE CCT **CROSS SECTIONS** VWPH Weight: 15 Lbs; maximum weight with EM/10WC or HSGX: 27 lbs. **FEATURES** GENERAL 5-11/16" Engineered with the highest quality materials to ensure reliability, performance, and quality. 16-3/4' 7-5/8" 10-1/2 Provides security and accent lighting for Side View Side View shown with Uplight Front view walkways, entries, perimeters, and EM/10WC or HSGX Application facades VWPV Weight: 23 Lbs Intended for use in both uplight and downlight applications. Aesthetically designed horizontal and vertical housings blend seamlessly with a 9-3/8' variety of architectural styles. Purposefully modeled to allow runoff of dirt and water for an always-clean appearance. Architectural housing extension option can be used with or without EM to maintain aesthetics throughout an entire project 14-3/4 6-5/8 7-3/8" Front View Side View Side View shown Uplight Application with FM/4W or HSGX Optional energy-saving photocell/ occupancy sensor available. **ORDERING INFORMATION** Rated >50,000 hours at 70% lumen maintenance (L70). SERIES **OPTIONS** ANSI 3000K, 4000K or 5000K CCT; VWP Voltaire Architectural Wall Pack See page 3 for option details minimum 70 CRI. EM/4W 4-watt integral emergency LED driver, 500 lumen output (120-277V only) Available in six standard finish options TYPE This fixture is proudly made in the USA. Horizonta VWPV includes housing extension ۷ Vertical (increases fixture depth) THERMAL Single fuse (120V, 277V, or 347V only; SF Integral die-cast aluminum heatsink LED PACKAGE must specify voltage) provides optimal passive thermal See back for fixture performance data Double fuse (208V, 240V, or 480V only; management. DF Concealed heatsink design preserves Example: L60/740 must specify voltage) architectural appearance. AVERAGE SYSTEM Factory-installed button-style PC LUMEN NOMINAL MINIMUM CRI Rated for -30°C to 40°C ambient operating photocell (120V, 208V, or 277V only; PACKAGE LUMENS & CCT WATTAGE temperature (-20°C to 40°C with must specify voltage) EM/10WC; 0°C to 39°C with EM/4W). L30 3,000 730 = 70 CRI, 3000K 36 HSGX Empty housing extension used to 740 = 70 CRI, 4000K 750 = 70 CRI, 5000K OPTICAL match units with EM or OCC option. L60 6,000 70 Acrylic precision optics produce standard Conduit entry options available. See page 3 for details. IES distributions. Right side conduit entry CR DISTRIBUTION Left side conduit entry Dual conduit entry (left and right) Full cutoff, dark-sky compliant optics CL CD (downlight only) place light where it's T3 TFT Type III needed with minimal glare. Type Forward Throw VWPH ONLY ELECTRICAL EM/10WC **FINISH OPTIONS** 10-watt emergency LED driver, 1000 lumen output, low temperature, 0-10V dimming standard. For custom color, visit the VWP at hew.com.<sup>1</sup> includes housing extension (increases 10kA/10kV surge protection standard. Black (RAL #9004) RIK fixture depth; 120-277V only) ► LED system is designed to minimize DBZ Dark bronze Factory-installed occupancy sensor, OCCWS FSP-211-L electrical connection points for increased DBR Medium bronze includes housing extension (increases reliability. GRAY Standard gray fixture depth), must specify lens, see Satin aluminum (RAL #9006) SLV page 2 for details. Optional FSIR-100 WHT White (RAL #9003) remote controller available, ordered separately, see accessories. SHIELDING ACCESSORIES (VWPH only) Solite® diffused textured tempered glass lens SDGL FSIR-100 Remote controller for occupancy sensor <sup>2</sup> Clear tempered glass lens CGL DRIVER DIM Dimming driver prewired for 0-10V controls VOLTAGE For custom colors other than RAL UNV 120-277V 120 120V manufacturers' code plus two swatches 208 208V 347 347V (minimum 1" square) required. Please specify quantity required per project. 277 277V 480 480V



# **VWP**

# **VOLTAIRE ARCHITECTURAL WALL PACK**

## **DISTRIBUTION**



Housing – Die-cast aluminum enclosure. Thermal Management – Integral die-cast aluminum heatsink and LED assembly provide passive thermal management. Rated -30°C to 40°C ambient operating temperature (-20°C to 40°C with EM/10WC; 0°C to 39°C with EM/4W).

**Optical System** – Precision, injectionmolded, refractive acrylic lensing produces standard IES distributions.

LED Assembly – ANSI 3000K, 4000K, or 5000K CCT, minimum 70 CRI LEDs.

LED Driver – 0-10V dimming.

Electrical – 120-277, 347, and 480 VAC input range; 50-60Hz; power factor >.90; THD <20% at full load. FCC Class A compliant. 10kA/10kV surge protection standard. Quick-disconnect wiring provided.

Finish – Super durable polyester powder coat bonded to phosphate-free, multi-stage pretreated metal, meets and exceeds AAMA 2604 specifications for outdoor durability. Available in six standard colors. Custom colors available.

Mounting – Surface mounts directly over a 4" maximum outlet box. Must be anchored to adequate structure that can safely support fixture weight (VWPH = 15 lbs, VWPV = 23 Lbs).

**Labels** – cCSAus certified as luminaire suitable for wet locations.

Certifications & Qualifications –

- Calculated L70 lumen maintenance >50,000 hours per IES TM-21.
   Tested to IES LM-79-08 standards.
- Tested to IES LM-79-08 standard
   Lighting Facts listed.
- DLC qualified products listed at www.designlights.org.
- IDA Dark-Sky approved (downlight applications only).
- RoHS compliant.
- IP65 rated.
   Title 24 com
- Title 24 compliant with OCCWS FSP-211-L\_ option.
- BUG classified per IES TM-15-11.

Warranty – 5-year limited warranty, see hew.com/warranty.





## Source G1992 (SDGE)

## FIXTURE PERFORMANCE DATA

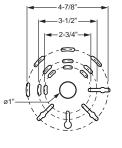
					CLEAR GLASS (CGL)		SOLITE GLASS (SDGL		
SERIES	DISTRIBUTION	LUMEN PACKAGE	WATTAGE <sup>1</sup>	CCT	DELIVERED LUMENS <sup>23</sup>	EFFICACY (LM/W) <sup>2 3</sup>	DELIVERED LUMENS <sup>23</sup>	EFFICACY (LM/W) <sup>2 3</sup>	BUG RATINGS
		L30	36	3000	3174	88.2	2963	82.3	B1-U0-G1
				4000	3327	92.4	3106	86.3	
	Т3			5000	3438	95.5	3209	89.1	
	15			3000	5933	84.8	5887	84.1	B1-U0-G1
		L60	70	4000	6611	94.4	6172	88.2	
VWPH				5000	6831	97.6	6376	91.1	
••••			36	3000	2713	75.4	2533	70.4	B1-U0-G1
		L30		4000	2844	79.0	2655	73.8	
	TFT			5000	2939	81.6	2743	76.2	
			70	3000	5470	78.1	5065	72.4	B2-U0-G2
		L60		4000	5688	81.3	5309	75.8	
				5000	5876	83.9	5486	78.4	
			36	3000	3115	86.5	2908	80.8	B1-U0-G1
		L30		4000	3403	94.5	3177	88.3	
	Т3			5000	3385	94.0	3160	87.8	
	13			3000	6171	88.2	5813	83.0	
		L60	<b>L60</b> 70	4000	6804	97.2	6351	90.7	B2-U0-G1
VWPV	av.			5000	6767	96.7	6317	90.2	
		L30 TFT L60	.30 36	3000	2840	78.9	2651	73.6	B1-U0-G1
				4000	3103	86.2	2896	80.4	
	TET			5000	3086	85.7	2881	80.0	
	111		70	3000	5822	83.2	5126	73.2	B3-U0-G1
				4000	5999	85.7	5600	80.0	
				5000	5967	85.2	5570	79.6	

Wattage shown is average for 120V through 277V input. *Continue of the second secon* 

Efficacy/lumen output shown is average based on voltage input of 120V through 277V.

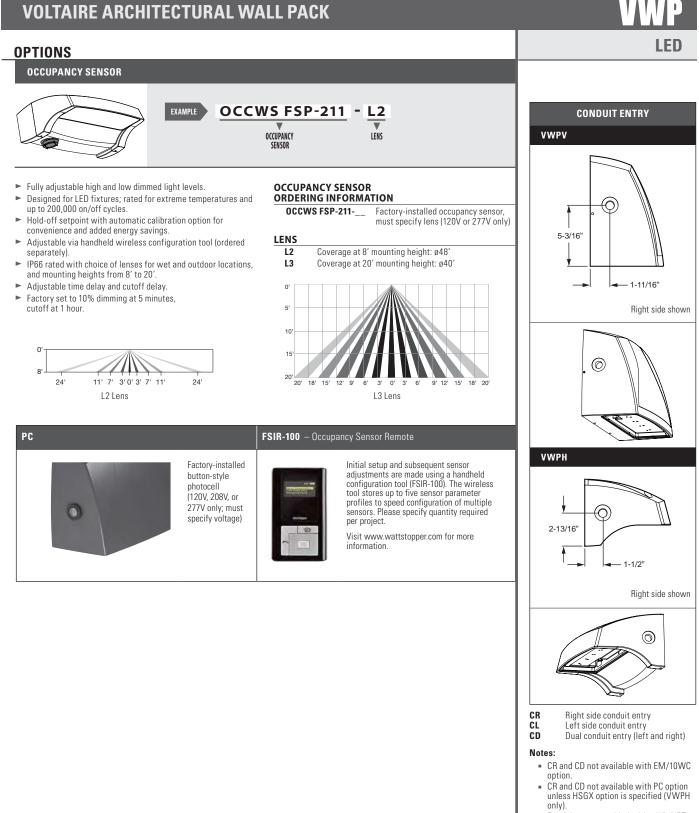
<sup>3</sup> Photometrics tested in accordance with IESNA LM-79. Results shown are based on 25°C ambient temperature.

#### **BOLT PATTERN DETAIL**









 Conduit entry provided with 1/2" -NPT tapered pipe thread and plug. Conduit fitting to be supplied by others.

<sup>6</sup> Williams

#### **VOLTAIRE MINI ARCHITECTURAL WALL PACK** CATALOG #: TYPE LED PROJECT: NOTES EXAMPLE VWM H - L20/740 - T3 - DBZ -SDGL -**OPTIONS** -DIM -UNV V V FINISH SHIELDING DISTRIBUTION OPTIONS DRIVER VOITAGE SERIES TYPE LUMEN CRI & PACKAGE CCT **CROSS SECTIONS** VWMH (Weight: 6.5 Lbs) **FEATURES** GENERAL 4-5/8' Engineered with the highest quality materials to ensure reliability, performance, and quality. 10-3/16" 6 Provides security and accent lighting Uplight for walkways, entries, perimeters, Side View Front view Application and facades. VWMV (Weight: 6.5 Lbs) Intended for use in both uplight and downlight applications. Aesthetically designed horizontal and vertical housings blend seamlessly with 5-5/8' a variety of architectural styles. Purposefully modeled to allow runoff of dirt and water for an always-clean appearance. Optional energy-saving photocell 8-7/8 -7/8' Uplight available. Front View Side View Application Rated >50,000 hours at 70% lumen maintenance (L70). ► ANSI 3000K, 4000K, or 5000K CCT. **ORDERING INFORMATION** • Minimum 70 CRI. SERIES **FINISH OPTIONS** Available in six standard finish options. VWM Voltaire Mini Architectural Wall Pack For custom color, visit the VWM at hew.com.<sup>1</sup> ► This fixture is proudly made in the USA. Black (RAL #9004) BLK THERMAL DBZ Dark bronze Integral die-cast aluminum heatsink ТҮРЕ DBR Medium bronze provides optimal passive thermal Н Horizontal GRAY Standard gray management. ۷ Vertical SLV Satin aluminum (RAL #9006) Concealed heatsink design preserves WHT White (RAL #9003) architectural appearance. Rated ambient operating temperature LED PACKAGE -30°C to 50°C (L10, L17), -30°C to 45°C See back for fixture performance data SHIELDING (L20). Example: L17/740 Solite<sup>®</sup> diffused textured tempered glass lens SDGI OPTICAL AVERAGE CGL Clear tempered glass lens MINIMUM CRI & CCT LUMEN PACKAGE NOMINAL LUMENS Full cutoff, dark-sky compliant optics SYSTEM (downlight applications only) place light WATTAGE where it's needed with minimal glare. 830 =80 CBL 3000K **OPTIONS** L10 1.000 13 840 = 80 CRI, 4000K ELECTRICAL See back for option details. L17 1,700 16 750 = 70 CRI, 5000K PC Factory-installed button-style photocell (120V, 0-10V dimming standard. 208V, or 277V only; must specify voltage) 10kA/10kV surge protection optional. **730** =70 CRI, 3000K **SP10** 10kA/10kV surge protection L20 2000 740 = 70 CRI, 4000K 25 750 = 70 CRI, 5000K DRIVER DIM Dimming driver prewired for 0-10V controls DISTRIBUTION Lambertian distribution (L10 & L17 only) TL VOLTAGE Type III (L20 only) 120V 120 208 208V 277 277V UNV 120-277V For custom colors other than BAL manufacturers' code plus two swatches (minimum 1" square) required. Williams

## **DISTRIBUTION DETAILS**

## **SPECIFICATIONS**

Housing – Die-cast aluminum enclosure. Thermal Management – Integral die-cast aluminum heatsink and LED source provide passive thermal management. Rated ambient operating temperature -30°C to 50°C (L10, L17), -30°C to 45°C (L20).

Optical System - General output provides full cutoff.

LED Source - ANSI 3000K, 4000K, or 5000K CCT; 70 or 80 CRI LEDs. L20: circuit board design. L10 & L17: Chip on board.

LED Driver - 0-10V dimming.

Electrical - 120-277 VAC input range; 50-60Hz; power factor >.90; THD <20% at full load. FCC Class A compliant. Quick-disconnect wiring provided.

Finish - Super durable polyester powder coat bonded to phosphate-free, multi-stage pretreated metal, meets and exceeds AAMA 2604 specifications for outdoor durability. Available in six standard colors. Custom colors available.

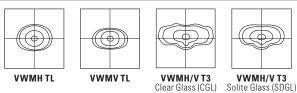
Mounting - Surface mounts directly over a 4" maximum outlet box. Must be anchored to adequate structure that can safely support fixture weight (6.5 Lbs).

**Labels** – cCSAus certified as luminaire suitable for wet locations.

#### Certifications & Qualifications – Calculated L70 > 50,000 hours per

- IES TM-21.
- Tested to IES LM-79-08 standards.
- Lighting Facts listed.
  DLC qualified products listed at www.designlights.org.
- IDA Dark-Sky approved (downlight) applications only).
- RoHS compliant.
- IP65 rated.
- Title 24 compliant with PC option.
  ADA compliant (VWMV only).
- BUG classified per IES TM-15-11.

Warranty - 5-year limited warranty, see hew.com/warranty.



## FIXTURE PERFORMANCE DATA

	Distribution	Lumen Package	Average System Wattage <sup>1</sup>	ССТ	Clear Glass (CGL)		Solite Glass (SDGL		
Series					Delivered Lumens <sup>23</sup>	Efficacy (Im/W) <sup>2 3</sup>	Delivered Lumens <sup>23</sup>	Efficacy (Im/W) <sup>2 3</sup>	BUG Ratings
			13	3000	1188	91.3	1104	85.0	B1-U0-G0
		L10		4000	1250	96.2	1163	89.4	
	TL			5000	1349	103.8	1255	96.5	
			16	3000	1644	102.8	1529	95.6	
VWMH		L17		4000	1731	108.2	1610	100.6	
				5000	1840	115.0	1711	107.0	
		L20	<b>D</b> 25	3000	2387	95.5	2197	87.9	B1-U0-G1
	T3			4000	2390	95.6	2229	89.2	
				5000	2510	100.4	2341	93.6	
	TL	L10	<b>0</b> 13	3000	1006	77.4	936	72.0	B1-U0-G0
				4000	1059	81.5	985	75.8	
				5000	1201	92.4	1117	85.9	
		L17	16	3000	1520	95.0	1414	88.4	
VWMV				4000	1600	100.0	1488	93.0	
				5000	1704	106.5	1585	99.0	
	ТЗ	3 L20	<b>L20</b> 25	3000	2139	85.6	2022	80.9	B1-U0-G1
				4000	2290	91.6	2136	85.4	
				5000	2301	92.0	2146	85.8	

Wattage shown is average for 120V through 277V input.

2 Efficacy/lumen output shown is average based on voltage input of 120V through 277V.

3 Photometrics tested in accordance with IESNA LM-79. Results shown are based on 25°C ambient temperature.

#### LUMEN MAINTENANCE

Lumen Package	Ambient Temp.	Reported L70 Hours (TM-21)
	25°C	>60,000
L20	35°C	>60,000
	45°C	>60,000
L10/L17	25°C	>55,000
	35°C	>55,000
	45°C	>55,000

- Predicted lumen maintenance calculated from LED manufacturer IES LM-80 data and in-situ temperature measurement
- Reported L70 hours in accordance with IES TM-21

**OPTIONS** 



Factory-installed button-style photocell (120V, 208V, or 277V only; must specify voltage)

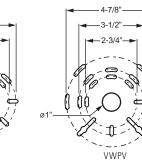
## **MOUNTING DETAILS**



-3-1/2"

-2-3/4"

VWPH







H.E. Williams, Inc. Carthage, Missouri Information contained herein is subject to change without notice

417-358-4065 www.hew.com HEW70489JL REV.09/07/16



Project

Туре

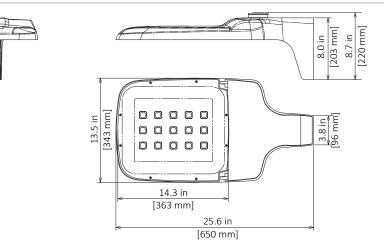
Catalog No.

# ARIETA™13 Architectural LED Area Luminaire AR13 M2 Series Specification Data Sheet

## **Luminaire Data**

 Weight
 15.4 lbs [7 kg]

 EPA
 0.47 ft<sup>2</sup>



## **Ordering Information**

Sample Catalog No. AR13 6M2 MV NW 5 BK 700 MSL3

Product	LED No. & Type	Voltage	Nominal Color Temperature	Distribution	Finish <sup>1</sup>	Drive Current <sup>2</sup>	Options
AR13	4M2 6M2 10M2 15M2 18M2 20M2	MV 120-277V HV 347-480V	<ul> <li>WW 3000K</li> <li>NW 4000K</li> <li>CW 5000K</li> </ul>	<ul> <li>2 Type 2</li> <li>3 Type 3</li> <li>4 Type 4</li> <li>5 Type 5</li> </ul>	BK Black DB Dark Bronze WH White GY Gray NA Natural Aluminum	350 350mA 530 530mA 700 700mA	HSS4House Side Shield (Factory Installed)FDC5Fixed Drive CurrentPCR3NEMA Photocontrol ReceptaclePCR53ANSI 5-wire Photocontrol ReceptaclePCR73ANSI 7-wire Photocontrol ReceptaclePCR74Control Ready 5-wire Photocontrol ReceptaclePCR7-CR3Control Ready 7-wire Photocontrol ReceptaclePCR7-CR3Control Ready 7-wire Photocontrol ReceptaclePCR7-CR3Control Ready 7-wire Photocontrol ReceptaclePCR7-CR3Motion Sensor with L7 Lens MSL36MSL36Motion Sensor with L3 Lens PND19PND19Part-Night Dimming Part-Night DimmingPND39Part-Night Dimming ORROptics Rotated Right ORLOptics Rotated Left WLWLUtility Wattage Label

#### Notes:

1 Black, Dark Bronze, White, Gray, or Natural Aluminum standard. Consult factory for other finishes.

- 2 Factory set drive current, field adjustable standard. Consult factory if wattage limits require a special drive current.
- 3 Specify with CR for control-ready wiring at factory for wireless node dimming. For details, see
- Wireless Control Options brochure link at www.leaotk.com, product page supporting documents.
  Flush mounted shield factory installed, also available for field installion. House Side Shield cuts light
- off at 1/2 mounting height behind luminaire. 5 Non-field adjustable drive current. Specify 350mA, 530mA or 700mA setting.
- 6 Motion Sensor available with MV only. See L7 or L3 Lens coverage details on page 5. Consult factory for MS specified with ANSI 5-wire or 7-wire Photocontrol Receptacle. PCR option is required for On/ Off control using light detection.
- 7 Specify Color (GY, DB, BK, WH, NA)
- 8 Specify MV (120-277V) or HV (347V or 480V)
- 9 For PND profile options see page 6. Only available with MV (120-277V).

## Accessories\*

HSS <sup>4,7</sup>	House Side Shield
RPA <sup>7</sup>	Round Pole Adapter
PTF1 <sup>7</sup>	Square Pole Top Fitter Single
PTF2 <sup>7</sup>	Square Pole Top Fitter Twin at 180°
PTF4 <sup>7</sup>	Square Pole Top Fitter Quad
WM <sup>7</sup>	Wall Mount
BSK	Bird Deterrent Spider Kit
PC <sup>8</sup>	Twist Lock Photocontrol
	Long-Life Twist Lock Photocontrol
SC	Twist Lock Shorting Cap
FSIR100	Motion Sensor Configuration Tool

\*Accessories are ordered separately and not to be included in the catalog number





## **Luminaire Specifications**

## Housing

Die cast aluminum housing with universal mounting design allows for attachment to existing pole without redrilling for retrofit applications. Aluminum housing provides passive heat-sinking of the LEDs and has upper surfaces that shed precipitation. Mounting provisions meet 3G vibration per ANSI C136.31-2001 Normal Application, Bridge & Overpass. Electrical components are accessed without tools and are mounted on removable power door.

## **Light Emitting Diodes**

Hi-flux/Hi-power white LEDs produce a minimum of 90% of initial intensity at 100,000 hours of life based on IES TM-21. LEDs are tested in accordance with IES LM-80 testing procedures. LEDs have correlated color temperature of 3000K (WW), 4000K (NW), or 5000K (CW) and 70 CRI minimum. LEDs are 100% mercury and lead free.

## **Optical Systems**

Micro-lens optical systems produce IESNA Type 2, Type 3, Type 4 or Type 5 distributions and are fully sealed to maintain an IP66 rating. Luminaire produces 0% total lumens above 90° (BUG Rating, U=0). Optional house side shield (HSS) cuts light off at 1/2 mounting height behind luminaire. Optics may be rotated right or left with options ORR/ORL, respectively.

## Electrical

Rated life of electrical components is 100,000 hours. Uses isolated power supply that is 1-10V dimmable. Power supply is wired with quick-disconnect terminals. LED drive current can be changed in the field to adjust light output for local conditions (not available with PCR5-CR or PCR7-CR options). Power supply features a minimum power factor of .90 and <20% Total Harmonic Distortion (THD). EMC meets or exceeds FCC CFR Part 15. Terminal block accommodates 6 to 14 gauge wire. Surge protection complies with IEEE/ANSI C62.41 Category C High, 20kV/10kA.

### Controls

3-Wire photocontrol receptacle (PCR) is available. ANSI C136.41 5-wire (PCR5) or 7-wire (PCR7) photocontrol receptacles are available. All photocontrol receptacles have tool-less rotatable bases. Wireless control module is provided by others.

## Finish

Housing receives a fade and abrasion resistant polyester powder coat finish. Finish tested to withstand 5000 hours in salt spray exposure per ASTM B117. Finish tested 500 hours in UV exposure per ASTM G154 and meets ASTM D523 gloss retention.

## Listings/Ratings/Labels

Luminaires are UL listed for use in wet locations in the United States and Canada. DesignLights Consortium™ qualified 120-277V product. International Dark Sky Association listed. Luminaire is qualified to operate at ambient temperatures of -40°C to 40°C. Assembled in the U.S.A

### Photometry

ARIETA™13 Architectural LED Area Luminaire

**AR13 M2 Series Specification Data Sheet** 

Luminaires photometrics are tested by certified independent testing laboratories in accordance with IES LM-79 testing procedures.

### Warranty

10-year limited warranty is standard on luminaire and components. 5-year limited warranty on luminaires and components with a motion sensor.



# ARIETA<sup>™</sup>13 Architectural LED Area Luminaire AR13 M2 Series Specification Data Sheet

ata nominal. IES files are available at leotek.com.		Туре 2, 3	Type 2, 3, 4		Type 5	
No. of LEDs & Type	Drive Current (mA)	System Wattage (W)	Delivered Lumens (Lm) <sup>1</sup>	Efficacy (Lm/W)	Delivered Lumens (Lm) <sup>1</sup>	Efficacy (Lm/W)
	350	20	2000	100	1930	96
4M2	530	28	2720	97	2630	94
	700	36	3410	94	3300	91
	350	29	2930	101	2750	95
6M2	530	41	4110	99	3860	93
	700	54	5040	94	4950	92
	350	41	4600	112	4500	109
10M2	530	63	6700	106	6600	104
	700	87	8500	97	8400	96
	350	63	7400	117	7300	116
15M2	530	90	9600	107	9500	106
	700	124	12900	104	12700	102
	350	81	9000	111	9100	112
18M2	530	122	12800	105	13000	107
	700	160	16400	103	16700	104
	350	84	10700	127	10800	129
20M2	530	132	15300	116	15500	117
	700	172	18200	106	18500	108

Notes:

1 Normal tolerance ± 10% due to factors including distribution type, LED bin variance, driver variance, and ambient temperatures.

## Performance Data 4000K (NW) & 5000K (CW)

ata nominal. IES files are available at leotek.com.		Туре 2, 3, 4		Type 5		
No. of LEDs & Type	Drive Current (mA)	System Wattage (W)	Delivered Lumens (Lm) <sup>1</sup>	Efficacy (Lm/W)	Delivered Lumens (Lm) <sup>1</sup>	Efficacy (Lm/W)
	350	20	2330	116	2290	114
4M2	530	28	3170	113	3110	111
	700	36	3990	111	3920	109
	350	29	3490	120	3480	120
6M2	530	41	4810	116	4850	117
	700	54	5980	111	5880	109
	350	41	5400	132	5300	129
10M2	530	63	7800	124	7700	122
	700	87	10000	115	9800	113
	350	63	8400	133	8300	132
15M2	530	90	11500	128	11300	126
	700	124	15000	121	14700	119
	350	81	9600	119	9700	120
18M2	530	122	13700	112	13900	114
	700	160	17500	109	17800	111
	350	84	10600	126	10800	129
20M2	530	132	15200	115	15500	117
	700	172	19500	114	19800	115

Notes:

1 Normal tolerance ± 10% due to factors including distribution type, LED bin variance, driver variance, and ambient temperatures.



# ARIETA<sup>™</sup>13 Architectural LED Area Luminaire AR13 M2 Series Specification Data Sheet

## BUG Ratings: 3000K (WW)

All data nominal. IES files for all CCTs are available at leotek.com.

No. of LEDs & Type	Drive Current (mA)	Type 2	Туре З	Type 4	Type 5
	350	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G0</b>	B1 U0 <b>G0</b>
4M2	530	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G0</b>
	700	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G1</b>
	350	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 G <b>0</b>
6M2	530	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G1</b>
	700	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B3 U0 <b>G1</b>
	350	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G1</b>
10M2	530	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G1</b>	B3 U0 <b>G1</b>
	700	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G1</b>
	350	B1 U0 <b>G1</b>	B1 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G1</b>
15M2	530	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
	700	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	350	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
18M2	530	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	700	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	350	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
20M2	530	B3 U0 <b>G3</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	700	B3 U0 <b>G3</b>	B3 U0 <b>G3</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>

## BUG Ratings: 4000K (NW) & 5000K (CW)

All data nominal. IES files for all CCTs are available at leotek.com.

No. of LEDs & Type	Drive Current (mA)	Type 2	Type 3	Type 4	Type 5
	350	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G0</b>	B1 U0 <b>G0</b>
4M2	530	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G0</b>
	700	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G1</b>
	350	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B2 U0 <b>G1</b>
6M2	530	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B3 U0 <b>G1</b>
	700	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B3 U0 <b>G1</b>
	350	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B1 U0 <b>G1</b>	B3 U0 <b>G1</b>
10M2	530	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G1</b>
	700	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
	350	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G1</b>
15M2	530	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
	700	B2 U0 <b>G2*</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	350	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
18M2	530	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	700	B3 U0 <b>G3</b>	B3 U0 <b>G2*</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	350	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>
20M2	530	B2 U0 <b>G2*</b>	B2 U0 <b>G2</b>	B3 U0 <b>G2</b>	B4 U0 <b>G2</b>
	700	B3 U0 <b>G3</b>	B3 U0 <b>G3</b>	B3 U0 <b>G3</b>	B4 U0 <b>G2</b>

\* These BUG ratings are slightly different for 5000K (CW). Refer to IES files for actual CW rating.



# ARIETA™13 Architectural LED Area Luminaire AR13 M2 Series Specification Data Sheet

## **Motion Sensor (Optional) Specifications**

## Description

Digital passive infrared luminaire integrated outdoor occupancy sensor provides high/low/off control based on motion detection. Initial setup and subsequent sensor adjustments are made using a handheld configuration tool. PCR option is required for On/Off control using light detection.

## Operation

Standard factory setting will dim the luminaire to 50% until motion is sensed and then it will power to 100%. When motion is not detected for five minutes, the luminaire will dim back to 50%. Ramp up and fade down times are adjustable, but initially set to NONE. The percent dimming and time durations may be field adjusted as required using FSIR-100 configuration tool. FSIR-100 user guide available at: www.wattstopper.com.

## **Optical System**

Multi-cell, multi-tier Fresnel lens with a 360 degree view detects unobstructed motion within one mounting height, up to 20 ft. maximum (MSL3) or 40 ft. maximum (MSL7). Consult factory for higher mounting height requirements.

## Finish

Sensor exterior ring and lens are white polycarbonate, UV and impact resistant.

## Listings/Ratings

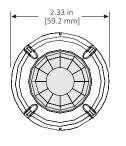
Sensor is TUV, UL and cUL listed, IP66 rated and CE compliant.

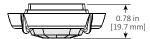
## Warranty

5-year limited warranty on luminaires and components with a motion sensor.

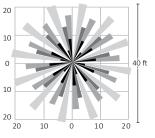
## Motion Sensor (Optional) Data

## **MSL3 Lens Dimensions**

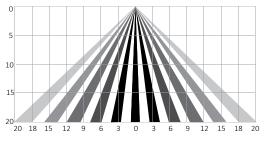




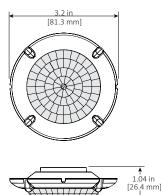
MSL3 Lens Coverage Top View



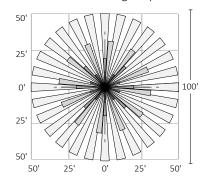


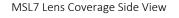


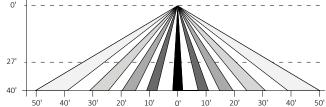
## MSL7 Lens Dimensions



MSL7 Lens Coverage Top View









# ARIETA<sup>™</sup>13 Architectural LED Area Luminaire AR13 M2 Series Specification Data Sheet

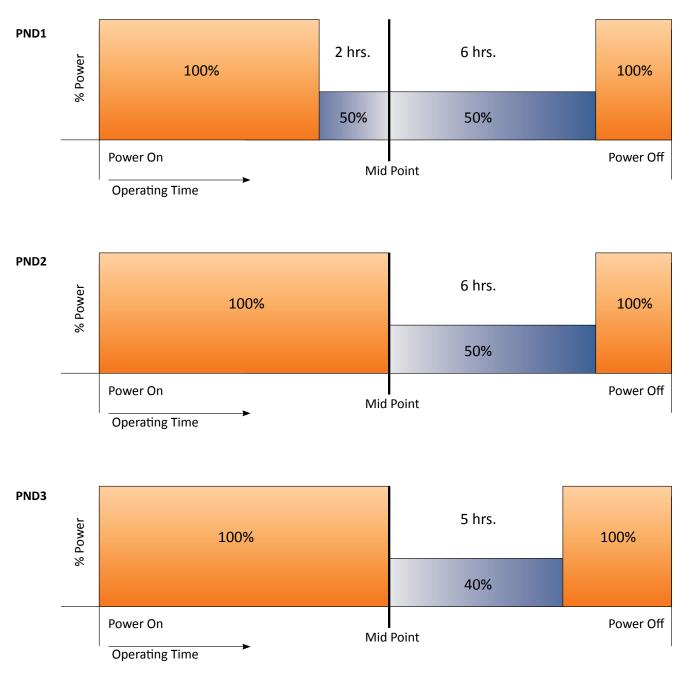
## **Part-Night Dimming Specifications**

## Description

Arieta's Part-Night Dimming (PND) option enables significant energy savings by automatically dimming the luminaire during early morning hours when infrequent use is expected. Factory programmed dimming profiles automatically take into account seasonal changes based on geographical location by continuously monitoring the nighttime midpoint. This option is fully compatible with photocells and time clock devices, but is not field adjustable.

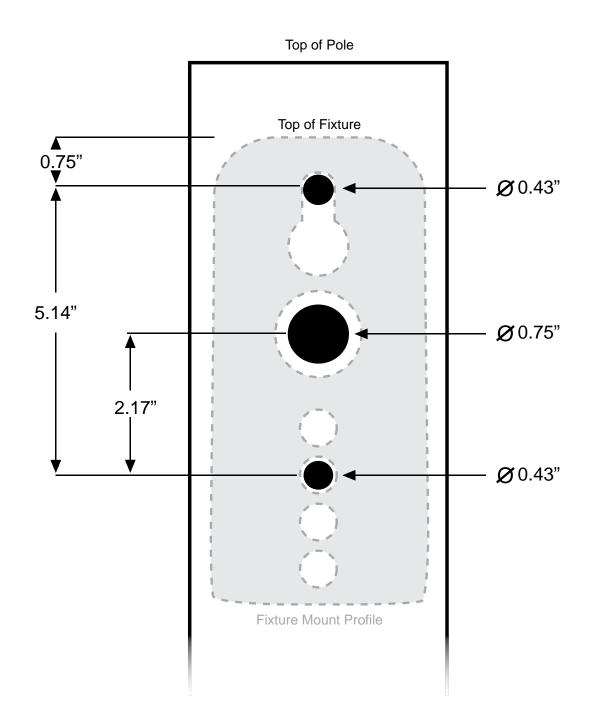
## Operation

Based on the PND profile that is selected, the luminaire dims to the corresponding % power for the corresponding length of time (based on the nighttime mid-point) as shown below. Mid-point is continuously recalculated in the luminaire by monitoring the average length of time between when the light turns on (power on) and turns off (power off) over the previous two days. In effect, this functionality will take two days to initialize after installation before any dimming will occur. Power interruptions are ignored and do not affect the determination of mid-point. Three factory programmed PND profiles are available for selection:





## Pole Mount Drilling Specifications





## Concurrency Impact Analysis Transmission Doctor

This proposed building is a 3,200 square foot automotive repair shop with associated parking and stormwater infrastructure improvements. There is an existing building (1,323 SF) on the site that will be used for storage when the new building is constructed.

## Stormwater:

A detailed stormwater management plan is included with this submittal. The proposed stormwater system is designed in compliance with City of Alachua and Suwannee River Water Management District requirements.

## Potable Water:

Goal 4: Provide an adequate supply of high quality potable water to customers throughout the service area.

Objective 4.1 Achieve and maintain acceptable levels of service for potable water quality and quality.

*Project Impact:* For the automotive repair shop, it is estimated that approximately 75 G.P.D. will be used for the proposed automotive repair shop (office) (Ch. 64E-6, F.A.C.). The proposed automotive repair shop (office) will generate approximately 75 G.P.D., based on this calculation 5 employees x 15 G.P.D./employee = 75 G.P.D.) As shown in the following table, there is adequate capacity available to support this development.

Table 3. Potable Water Impacts					
System Category	Gallons Per Day				
Current Permitted Capacity <sup>1</sup>	2,300,000				
Less Actual Potable Water Flows <sup>1</sup>	1,190,000				
Reserved Capacity <sup>2</sup>	140,504				
Transmission Doctor	75				
Residual Capacity	969,421				
Percentage of Permitted Design Capacity Utilized	57.86%				
Sources:					
1 City of Alachua Public Saniago Department April 2016					

1. City of Alachua Public Services Department, April 2016

## Sanitary Sewer:

Goal 1: Plan for and provide adequate, high quality and economical wastewater service while protecting the environment, especially groundwater resources.

Objective 1.2 Wastewater service will be made available to new development in a manner to promote compact urban growth, promoting development where wastewater service is available, and discouraging urban sprawl.

*Project Impact:* The proposed automotive repair shop is not connected to the sanitary sewer system. A septic system is proposed for the site and has been designed according to projected flows.

## Solid Waste:

Goal 2: The City of Alachua will provide for solid waste disposal service in a sanitary, economic, and environmentally safe manner.

*Project Impact:* Commercial uses generate approximately 12 pounds per day of solid waste per 1,000 square feet (Environmental Engineering: A Design Approach, Cincero and Cincero, 1996). The proposed facility will generate approximately 54.2 pounds of solid waste will be generated per day (4,523 SF / 1,000 SF x 12 = 38.4 pounds per day). As indicated in the following table, the proposed solid waste generated as part of this project will not reduce the level of service in the City of Alachua.

## Table 6. Solid Waste Impacts

System Category	Lbs Per Day	Tons Per Year
Existing Demand <sup>1</sup>	39,568.00	7,221.16
Reserved Capacity <sup>1</sup>	6,705.11	1,223.68
New River Solid Waste Facility Capacity <sup>3</sup>	50 years	

1. Bureau of Economic & Business Research, University of Florida, Estimates of Population by County and City in Florida, April 1, 2015; Policy 2.1.a, CFNGAR Element

2. Darrell O'Neal, Executive Director, New River Solid Waste Association, March 2016

## Traffic:

The proposed use of the project site for commercial (automotive repair) use will not create a traffic impact that will exceed the approved level of service standards for the impacted roadways.

## Affected Comprehensive Plan Roadway Segments:

Segment Number	Segment Description	Lanes	Functional Classification	Area Type	LOS		
3/4	US 441 (from NW 126 <sup>th</sup> Ave to SR 235)	4/D	Principal Arterial	Urban Trans	C		
1 Source: City of Ala	1 Source: City of Alachua Comprehensive Plan, Transportation Element.						

## **Trip Generation:**

Land Use	SAT (Enter/Exit)	Peak Hour (Enter/Exit)					
Auto Care Center	107.2	14.1					
(ITE Code 710)	(53.6/53.6)	<mark>(6.8/7.3)</mark>					
1 Source: ITE Trip Generation, 9th Edition. 2 Formulas: ITE Code 710 – SAT –23.72 trips per 1000 SF x 4323 SF (50% entering/50% exiting); PM Peak Hour – 3.11 trips per 1000 SF x 4323 SF (48% entering/52% exiting)							

## **Projected Impact on Affected Comprehensive Plan Roadway Segments:**

Traffic System Category	(FDOT Segme	ent 106, CP #3/4) <sup>1</sup>
	ADT	Peak Hour
Maximum Service Volume <sup>2</sup>	35,500	3,200
Existing Traffic <sup>3</sup>	17,295	1,643
Reserved Trips <sup>4</sup>	2,169	258
Available Capacity <sup>4</sup>	16,036	1,299
Increase/Decrease in Daily Trips Generated by Development	- 107	-14
Residual Capacity After Development's Impacts	15,929	1,285

1 FDOT roadway segment number shown in parenthesis (when applicable.) For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity. 2 Source: FDOT 2013 Quality/Level of Service Handbook, Generalized Annual Average Daily Volumes and Generalized Peak Hour Two-Way Volumes for Areas Transitioning to Urbanized Areas or Areas of 5,000 Not in Urbanized Areas.

3 Florida State Highway System Level of Service Report 2013, Florida Department of Transportation, District II, August 2014.

4 Source: City of Alachua March 2017 Development Monitoring Report.

**Consistency:** The impacts generated by the development will not adversely affect the Level of Service (LOS) of the roadway segment identified above.



## **Statement of Proposed Uses**

The proposed Transmission Doctor building will be used for auto repair and servicing on the site of the existing business. The existing building (1323 SF) will become storage for the automotive repair business (to be relocated to the) proposed building, 3200 SF. The automotive repair and servicing business is located within the Industrial Future Land Use area and within the General Industrial zoning district, which allows automobile repair and servicing as a permitted use (Table 4.1.1 of the Land Development Regulations).

## **Comprehensive Plan Consistency**

Vision Element:

## III. GOALS TO IMPLEMENT THE VISION

GOAL 1: Economic Development: The City of Alachua has a unique business climate. The City is home to corporations, technology incubators, local businesses, and start-up companies. The City will maintain its focus on a welcoming business environment and encourage business development in the downtown area and along the U.S. 441 corridor. Alachua desires to continue to be a home to innovative businesses and an employment center where jobs are provided at every level. The City will continue to encourage the growth and development of established industries, such as biotechnology, and encourage the diversification and expansion of commercial businesses which provide integral services to the City's residents.

<u>Consistency:</u> The proposed Transmission Doctor building will support and contribute to commercial development along the US 441 corridor. The proposed facility will increase the number of job opportunities in the City and provide services to the city's residents.

## Future Land Use Element:

## Objective 1.5: Industrial

The City of Alachua shall establish one industrial district: Industrial. This district shall provide a broad range of clean industry, warehousing, research, and technology industries, to provide a variety of job opportunities to the citizens of Alachua and the North Central Florida Region.

www.edafl.com

<u>Consistency</u>: The proposed Transmission Doctor building will serve the intent of the Industrial Future Land Use designation, as it will provide automotive repair services for guests and the citizens of Alachua in a commercialized, urban portion of the City located in close proximity to the population base and US 441. In addition, the site is consistent with the policies outlined in Future Land Use Policy 1.5.d as indicated below:

Policy 1.5.d: The City shall develop performance standards for industrial uses in order to address the following:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;

<u>Consistency:</u> The Site Plan includes an appropriately designed vehicular connection to an adjacent street. The project is within an existing industrial park and connects to the existing street network in the area.

2. Buffering from adjacent existing/potential uses;

<u>Consistency:</u> A 20 foot wide west setback is shown between the site and adjacent property. The existing building will remain at the north end of the site, adjacent to other industrial development in the industrial park and the new building and proposed improvements are to the south, along roadways.

3. Open space provisions and balance of proportion between gross floor area and site size;

<u>Consistency</u>: The proposed project site exceeds the 10% required open space and the proposed building is less than the maximum 0.75 floor area ratio, as demonstrated on the Site Plan.

4. Adequacy of pervious surface area in terms of drainage requirements;

<u>Consistency:</u> The Site Plan includes a detailed stormwater and open space plan with design details for an on-site basin area, which demonstrate compliance with all applicable local and state regulations.

5. Placement of signage;

<u>Consistency:</u> No additional signage is proposed at this time.

6. Adequacy of site lighting and potential impacts of lighting upon the surrounding area. Lighting should be designed to minimize impacts and preserve the ambiance and quality of the nighttime sky by reducing light trespass and light pollution on adjacent properties by utilizing lighting at an appropriate intensity, direction and times to ensure light is not overused or impacting areas where it is not intended;

<u>Consistency:</u> The existing site has appropriate site lighting for the proposed building addition.

7. Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;

<u>Consistency:</u> The parking area is designed for safe maneuvering of vehicles associated with the automotive repair business operations. Parking spaces are located close to the building office.

8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;

<u>Consistency</u>: The Site Plan includes a landscape plan prepared by a registered landscape architect that complies with all applicable elements of the Comprehensive Plan and Land Development Regulations.

9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and

<u>Consistency:</u> There are no unique features or resources that may constrain site development. The proposed site plan is for expansion of an existing business.

10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.

<u>Consistency</u>: No specific performance based zoning requirements apply to this project, other than the standard requirements found in the Land Development Code.

11. Industrial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres by 5 acres or greater, .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.

<u>Consistency:</u> The proposed development has less than a 0.75 floor area ratio, as demonstrated on the Site Plan.



# Memorandum

To: Krystal Williams/Ken Blake

DATE: 01/04/17

**FROM:** Rosa Trautz

SUBJECT: Neighborhood Meeting – Transmission Doctor

## PUBLIC NOTICE

A neighborhood workshop will be held to discuss a site plan for a proposed building and associated infrastructure located at 14616 NW 128<sup>th</sup> Terrace in Alachua, tax parcel number 03212-010-000. This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the potential development and to seek their comments.

The meeting will be held on January 24, 2017 at 6:00 p.m. in Meeting Room A of the Alachua Library branch located at 14913 NW 140th Street, Alachua, FL 32615.



Contact: Clay Sweger, AICP eda engineers – surveyors – planners, inc. (352) 373-3541

# FORT LAUDERDALE **Cruise line offered counseling** after airport shooting

#### By Tamara Lush The Associated Press

FORT LAUDERDALE - For many flight passengers arriving Friday in Fort Lauderdale, it was supposed to be the start of an idyllic week cruising the Caribbean. Then, the deadly shooting in baggage claim happened. Cruises have gone forward - but not without delays, arrangements for traumatized travelers and some dampening of the usual atmosphere.

Holland America Cruise line operates many of the massive ships that typically set out around the weekend from the tourism hub's Port Everglades terminal. The company offered free counseling via a hotline run by emergency management company Empathia on at least two of its ships one that departed Friday night and another that left Saturday, company spokeswoman Sally Andrews said.

"The staff are being very attentive to how folks are feeling on board," Andrews said.

The shooter retrieved his luggage at Fort Lauderdale-Hollywood International Airport's Terminal 2 in the early afternoon with a legally checked, unloaded pistol and ammunition, went to a bathroom to load up, then came out and started firing.

The rampage killed five people, wounded six and touched off chaos that prompted police to

SENIOR CRITTERS



Air Canada passengers pick up their luggage at the partially opened Terminal 2 at Fort Lauderdale-Hollywood International Airport Sunday, Jan. 8. A gunman opened fire Friday in a baggage claim area at the Fort Lauderdale international airport, killing five people and wounding eight before he was taken into CUSTODY. TAIMY ALV AREZ/SOUTH FLORIDA SUN-SENTINEL VIA AP

This file booking photo provided by the Broward Sheriff's Office shows suspect Esteban Santiago in Fort Lauderdale, Fla. Santiago, the Iraq war veteran accused of fatally shooting multiple people and wounding several others at a crowded Florida airport baggage claim, is due for his first court appearance Monday, Jan. 9. BROWARD SHERIFF'S OFFICE VIA AP

lock down the airport until evening, causing delays and separating many people from their belongings.

Mark Lea, 53, of Elk River, Minnesota, witnessed the shooting, helped a wounded

woman, photographed the shooter's weapon in the baggage claim area then spent much of Friday night in the airport. waiting to be released from the crime scene by authorities.

When he was, he went to his hotel. He and his wife decided to continue on with their cruise because it was their first anniversary. Once aboard his ship, he wasn't sure how many of the other passengers had witnessed the carnage in the airport and wasn't keen on finding out.

"Been keeping pretty low key and trying to decompress," he said. The mood aboard

the Holland American Eurodam ship late Saturday was "subdued," said Sandy Robertson, a 61-year-old retired school principal from the Chicago area.

Although Robertson and ship.

happened, others passengers aboard the ship had lived through the ordeal.

As Robertson was headed to dinner Saturday night before the ship left port, she was reflective. A veteran of 10 cruises, she noted that the ship's captain was making more announcements than usual, and had thanked the passengers for being patient. She also discovered at least one of the victims who died was scheduled to be on her ship.

We're all kind of feeling like, 'Oh my gosh, here we are on this beautiful cruise ship and these families were affected." she said. "I look at this beautiful ship right now and am having dinner thinking about folks who are supposed to be here." Holland America delayed the departure of its Nieuw Amsterdam ship Friday by several hours largely because passengers were kept at the airport while authorities investigated the shooting. There was also a matter of luggage; the

airport's director says 25,000 pieces of luggage, cellphones and other belongings were separated from their owners during the shooting rampage. The Eurodam was delayed until 9 p.m.,

Andrews said. Two passengers were unable to catch the Costa Deliziosa from Port Everglades on Saturday, so the company flew them to Nassau in the Bahamas to catch the

# Meet the oldest animals at South Florida parks

#### **By Johnny Diaz** The Sun-Sentinel

MIAMI - South Florida sure has some wild seniors. They hang off trees,

scurry in yards, lounge in pools. And at least one makes a whale of a splash.

They are South Florida's other seniors - the animals, some well into their 70s and up, that live in the spotlight at the region's zoos and entertainment parks.

Likeus humans, they also have their share of allergies and aches and pains that come with aging. So what's the key to their longevity? Diet? Social environment? The fabulous weather? Good health care?

Whatever it is that's

she knows it will make her feel better," she said. "There is a little mouthpiece that she puts in her mouth and she sucks on it."

Lion Country Safari plans to celebrate Little Mama's birthday on Valentine's Day, complete with plush toys, which she loves, popcorn and some extra fruits and veggies, "We don't know if that's her birthday, but we feel that birthday fits her personality."

After all, Little Mama just LOVES the color red.

#### Lancelot at Lion Country Safari

Lion Country Safari also has Lancelot, a 93-year-old Aldabra Tortoise,

Weighing 776 pounds,

food pellets as well as algae from a pond in the wildlife preserve.

Interesting fact: "The beta carotene in the algae makes them pink. If they didn't have that, they would be white," said Michael Ruggieri, director of wildlife at the park.

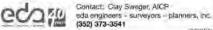
Hesaid Jane Doe is doing great for her age.

"She looks just as young as the other flamingos. You would not be able to tell that she is that old by looking at her," he said. "She must feel very comfortable where she is. She has a mate."

## PUBLIC NOTICE

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The meeting will be held on January 24, 2017 at 6:00 p.m. in Meeting Room A of the Alachua Library branch located at 14913 NW 140th Street, Alachua, FL 32615



# FLORIDA POLITICS Crisafulli won't run for agriculture commissioner

#### **By Brandon Larrabee** The News Service of Florida

TALLAHASSEE Former House Speaker Steve Crisafulli bowed out of a potential race for agriculture commissioner Tuesday, leaving the 2018 campaign for the Cabinet position without a clear front-runner.

Crisafulli, R-Merritt Island, comes from a family with deep roots in the state's citrus industry and had made no secret of the fact that he was considering a bid for agriculture commissioner. Adam Putnam, who currently holds the post, is term-limited and expected to run for governor in 2018. But in a statement issued

Monday, Crisafulli said he's out of the race.

"I plan to remain politically active, but after years of travel to fulfill my obligations to the House **Republican** Conference and as speaker of the FloridaHouse, there is nothing I want more than to spend time with my wife Kristen and our daughters as they finish out their final years of being at home before going off to college," he said.

Crisafulli, who served as speaker from 2014 to 2016, said he has "no doubt a capable field of candidates will emerge who will be dedicated to the success of this critical industry."

Crisafulli's stint as speaker was itself a surprise, as he took the place of former Rep. Chris Dorworth, R-Lake Mary. Dorworth had been slated to become speaker in November 2014, but was defeated in a bid for reelection in his district in 2012.

Crisafulli's tenure as speaker was tumultuous at times. During Crisafulli's first year at the helm, the House ended its session early as part of a healthcare showdown with the Senate, sparking the need for a special session to finish work on the budget.

The announcement Monday marked the second time in recent weeks that a former House speaker had taken himself out of the running for statewide office. Former Speaker Will Weatherford, a Wesley Chapel Republican who was speaker before Crisafulli, announced last month that he would not seek the governor's office in 2018. Like Crisafulli, Weatherford said he wanted to spend more time with his young family.

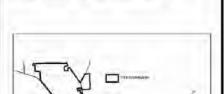
**Current House Speaker** Richard Corcoran, R-Land O' Lakes, is rumored to be considering a bid for governor, though he has been tight-lipped so far about his plans.

The maneuvering is sparked in part by what could be wide-open races for the four top spots in state government. Gov. Rick Scott also faces term limits in 2018, along with Putnam and the two other Republican members of the Cabinet: Attorney General Pam Bondi and **Chief Financial Officer Jeff** Atwater.

Bondi is reportedly in line to receive a White House appointment from President-elect Donald Trump, while Atwater has said he won't seek higher office in 2018. For his part, Scott is widely expected to run for the U.S. Senate seat currently held by Bill Nelson, a Democrat.

## HEARING BEFORE **City Plan Board** January 26, 2017

The Plan Board of the City of Gainesville, FL will consider the following item at a Public Hearing scheduled on Thursday, January 26, 2017 starting at 6:30 P.M. or as soon thereafter as may be heard, in the City Hall Building, 1st floor City Commission Auditorium, located at 200 E. University Avenue, Galnesville, FL FOR INFORMATION CALL, 352-334-5022.



# her husband Tom arrived before the shooting

## **NEIGHBORHOOD WORKSHOP NOTICE**

For a site plan for a proposed building and associated infrastructure located at 14616 NW 128th Terrace.

Date:	January 24, 2017
Time:	6:00 p.m.
Place:	Alachua Library Branch - Meeting Room A 14913 NW 140th Street, Alachua, FL 32615

Contact: eda engineers–surveyors–planners, inc. at (352) 373-3541

A neighborhood workshop will be held to discuss a site plan for a proposed building and associated infrastructure located at 14616 NW 128th Terrace in Alachua, tax parcel number 03212-010-000. This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the potential development and to seek their comments.





Name Antoinette Endelicato	Company	Street Address 5562 NW 93rd Avenue	<b>City</b> Gainesville	State FL	32653
Dan Rhine		288 Turkey Creek	Alachua	FL	32615
Tom Gorman		9210 NW 59th Street	Alachua	FL	32653
Richard Gorman		5716 NW 93rd Avenue	Alachua	FL	32653
Peggy Arnold		410 Turkey Creek	Alachua	FL	32615
David Forest		23 Turkey Creek	Alachua	FL	32615
John Tingue		333 Turkey Creek	Alachua	FL	32615
President	ТСМОА	1000 Turkey Creek	Alachua	FL	32615
Linda Dixon, AICP	Assistant Director Planning	PO Box 115050	Gainesville	FL	32611
Craig Parenteau	FL Deptarment of Environmental Protection	4801 Camp Ranch Road	Gainesville	FL	32641
Jeannette Hinsdale		P.O. Box 1156	Alachua	FL	32616
Lynn Coullias		7406 NW 126th Ave	Alachua	FL	32615
Lynda Coon		7216 NW 126 Avenue	Alachua	FL	32615
Tamara Robbins		PO Box 2317	Alachua	FL	32616
Dr. Lee A. Niblock	Alachua County Manager	12 SE 1st Street	Gainesville	FL	32601
John Amerson	All County Marion Property Management	2916 NE Jacksonville Rd	Ocala	FI	34479

## 03212-007-000, 03212-008-000, 03212-017-000, 03213-000-000 HIPP RENTAL PROPERTIES LLC PO BOX 1000 ALACHUA, FL 32616

## **Neighborhood Meeting Notice**

03212-003-000 R & R INTERNATIONAL SALES INC 4336 S PENINSULA DR PONCE INLET, FL 32127

## **Neighborhood Meeting Notice**

ANTOINETTE ENDELICATO 5562 NW 93RD AVENUE GAINESVILLE, FL 32653

### **Neighborhood Meeting Notice**

RICHARD GORMAN 5716 NW 93RD AVENUE ALACHUA, FL 32653

## **Neighborhood Meeting Notice**

JOHN TINGUE 333 TURKEY CREEK ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

CRAIG PARENTEAU - FL DEPT. OF ENVIRONMENTAL PROTECTION 4801 CAMP RANCH ROAD GAINESVILLE, FL 32641

## **Neighborhood Meeting Notice**

LYNDA COON 7216 NW 126 AVENUE ALACHUA, FL 32615 03212-016-000, 03212-016-001, 03212-013-000, 03212-009-000, 03212-019-000, 03212-014-000, 03212-015-000, 03212-018-000 CHRISTOPHER CORPORATION (THE) PO BOX 1000 ALACHUA, FL 32616-1000

## **Neighborhood Meeting Notice**

03212-010-000 M & M PROPERTIES OF ALACHUA 14616 NW 128TH TER ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

03212-011-000, 03212-011-001, 03212-012-000 SUNSTATE FEDERAL CREDIT UNION 14520 NW HIGHWAY 441 ALACHUA, FL 32615-8204

## **Neighborhood Meeting Notice**

DAN RHINE 288 TURKEY CREEK ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

PEGGY ARNOLD 410 TURKEY CREEK ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

ATTN: PRESIDENT, TCMOA 1000 TURKEY CREEK ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

JEANNETTE HINSDALE P.O. BOX 1156 ALACHUA, FL 32616

## **Neighborhood Meeting Notice**

TAMARA ROBBINS PO BOX 2317 ALACHUA, FL 32616

## **Neighborhood Meeting Notice**

03212-002-000 ELLISON, JUSTIN 330 N MAIN ST HIGH SPRINGS, FL 32643

03212-001-000, 03212-005-000, 03212-006-000, 03212-004-001, 03212-004-000 MARLOW ALACHUA HOLDINGS LLC 5212 SNEAD ISLAND RD PALMETTO, FL 34221

## **Neighborhood Meeting Notice**

JOHN AMERSON ALL CTY. MARION PROPERTY MGMT. 2916 NE JACKSONVILLE RD OCALA, FL 34479

## **Neighborhood Meeting Notice**

TOM GORMAN 9210 NW 59TH STREET ALACHUA, FL 32653

## **Neighborhood Meeting Notice**

DAVID FOREST 23 TURKEY CREEK ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

LINDA DIXON, AICP ASSISTANT DIRECTOR OF PLANNING PO BOX 115050 GAINESVILLE, FL 32611

## **Neighborhood Meeting Notice**

LYNN COULLIAS 7406 NW 126TH AVE ALACHUA, FL 32615

## **Neighborhood Meeting Notice**

DR. LEE A. NIBLOCK ALACHUA COUNTY MANAGER 12 SE 1ST STREET GAINESVILLE, FL 32601



engineers • surveyors • planners, inc. 2404 NW 43<sup>rd</sup> Street Gainesville, FL 32606



Neighborhood Meeting Notice 03212-002-000 ELLISON, JUSTIN 330 N MAIN ST HIGH SPRINGS, FL 32643





## Neighborhood Meeting Sign-in-Sheet

Project:	Site Plan for a proposed building and associated infrastructure located at 14616 NW 128th Terrace.
Date & Time:	January 24, 2017 @ 6:00pm
Location:	Alachua Library Branch - Meeting Room A 14913 NW 140th Street, Alachua, FL 32615

NAME	ADDRESS	PHONE	EMAIL
No Attenders	ADDRESS from public		
10 /11/00:0000	trent pastr -		
		4	



## **Neighborhood Meeting Minutes**

Project:	Site Plan for a proposed building and associated infrastructure located at 14616 NW 128th Terrace.			
Meeting Date & Time:	January 24, 2017 @ 6:00pm			
Location:	Alachua Library Branch - Meeting Room A 14913 NW 140th Street, Alachua, FL 32615			
Community Participants:	none			
Project Representatives:				
Engineer/Planner:	Clay Sweger, AICP			
Owner:	Jim Markle			
Meeting Minutes:				
No members of the community attended this meeting.				



## LEGAL DESCRIPTION

### for Parcel No. 03212-010-000

INSTRUMENT # 2551438 3 PGS

A tract of land situated in Woodbridges Subdivision of Stringfellow Farm in Section 14, Township 8 South, Range 18 East, Alachua County, Florida, said tract of land being more particularly described as follows:

Commence at an iron pipe being the Northwest corner of Lot 11 of the aforementioned Wodbridges Subdivision of Stringfellow Farm as per plat recorded in Plat Book "C", Page 68-A, of the Public Records of Alachua County, Florida for a point of reference and run South 01 deg.43'17" East, along the West line of said Lot 11, a distance of 899.72 feet to a concrete monument at the intersection of said West line with the North right of way line of the abandoned Seaboard Coastline Railroad (200 foot right of way); thence run South 83 deg.52'00" East, along said North right of way line, a distance of 423.98 feet to a concrete monument; thence run North 01 deg.43'17" West, parallel with said West line, a distance of 210.00 feet to the True Point of Beginning; thence continue North 01 deg.43'17" West, parallel with said West line, a distance of 200.74 feet to a concrete monument; thence run North 88 deg.16'43" East, a distance of 220.71 feet; thence run North 83 deg.52'00" West, parallel with said North right of way line of the abandoned Seaboard Coastline Railroad, a distance of 211.99 fet to the True Point of Beginning.

TOGETHER WITH AND SUBJECT TO a Non-exclusive Easement for the purpose of ingress-egress and public utilities, said Easement being described as the following tract of land:

A tract of land situated in Lot 11 of Woodbridge's Subdivision of the Stringfellow Farm, Alachua, Florida, a subdivision as recorded in Plat Book "C", Page 68-A of the Public Records of Alachua County, Florida, said tract of land being more particularly described as follows:

Commence at the Northwest corner of Lot 11 of the aforementioned Woodbridge's Subdivision for a point of reference; thence run South 01 deg.43'17" East, along the West line of said Lot 11, a distance of 21.19 feet to the Southerly Right of Way line of U.S. Highway No. 441; thence run South 84 deg.05'00" East, along said Right of Way line, a distance of 144.25 feet to a curve, said curve being concave Southerly, having a central angle of 00 deg.58'10", a radius of 2210.86 feet, a length of 37.41 feet and a chord bearing and distance of South 83 deg.35'45" East, 37.41 feet; thence run Southeasterly with said curve and along said Right of Way line, a distance of 37.41 feet to the point of beginning; thencer un South 01 deg.43'17" East, parallel to said West line of Lot 11, a distance of 878.90 feet to the Northerly Right of Way line of the abandoned Seaboard Coastline Railroad (200 feet Right of Way); thence run South 83 deg.52'00" East, along said Northerly Right of Way line, a distance of 60.57 feet; thence run North 01 deg.43'17" West, a distance of 179.72 feet; thence run South 83 deg.52'00" East, parallel to said Seaboard Coastline Railroad, a distance of 787.08 feet to a point, said point being the beginning of the arc of a culde-sac, said cul-de-sac being concave Westerly, having a central angle of 262 deg.49'10", a radius of 40.00 feet, an arc length of 183.49 feet and a chord bearing and distance of North 06 deg.08'00" East, 60.00 feet; thence run along said arc a distance of 183.49 feet to the end of said arc; thence run North 83 deg.52'00" West, a distance of 371.38 feet; thence run North 01 deg.43'17" West, a distance of 575.66 feet to a point, said point being a point on a curve of the Southerly Right of Way line of U.S. Highway No. 441, said curve being concave Southerly, having a central angle of 01 deg.39'43", a radius of 2210.86 feet, a length of 64.13 feet and a chord bearing and distance of North 71 deg.02'09" West, 64.13 feet; thence run Northwesterly with said curve and along said Right of Way line, a distance of 64.13 feet; thence run South 01 deg.43'17" East, a distance of 590.04 feet; thence run North 83 deg.52'00" West, a distance of 363.41 feet; thence run North 01 deg.43'17" West, a distance of 636.96 feet to a point, said point being a point on a curve of the Southerly Right of Way line of U.S. Highway No. 441, said curve being concave Southerly, having a central angle of 01 deg.34'34", a radius of 2210.86 feet, a length of 60.82 feet and a chord bearing and distance of North 82 deg.19'27" West, 60.82 feet; thence run Northwesterly with said curve and along said Right of Way, a distance of 60.82 feet to the point of beginning.

27 00 ZL. 875.00 DOC

This instrument prepared by: MARVIN W. BINGHAM, JR.,PA P.O. Box 1930 Alachua, Florida 32616 RECORDED IN OFFICIAL RECORDS INSTRUMENT # 2551438 3 PGS Jan 20, 2010 09:53 AM BOOK 3931 PAGE 1841 J. K. IRBY Clerk Of Circuit Court Alachua County, Florida CLERK10 Receipt # 431918

Doc Stamp-Deed: \$875.00



Tax Parcel # 03212-010-000

## 2551438 3 PGS

## WARRANTY DEED

*THIS INDENTURE*, made this <u>/</u> day of January, 2010 between Phyllis M. Wright, an unremarried widow, whose post office address is 9503 NW 59<sup>th</sup> Terrace, Gainesville, Florida 32653, Grantor, and M&M Properties of Alachua, LLC, a Florida Limited Liability Company, whose post office address is 14616 NW 128<sup>th</sup> Terrace, Alachua, Florida 32615, Grantee

WITNESSETH that said Grantor, for and in consideration of the sum of Ten and 00/100 Dollars (\$10.00), and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee, and Grantees' assigns forever, the following described land, situate, lying and being in Alachua County, Florida, to wit:

See Exhibit A, attached hereto and made a part hereof.

*SUBJECT TO* and together with covenants, easements, reservations and restrictions of record, and taxes for the year 2010and all subsequent years,

TOGETHER WITH all the tenements, hereditaments and appurtenances thereto belonging or in any-wise appertaining,

TO HAVE AND TO HOLD, the same in fee simple forever.

AND said Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has a good right and lawful authority to sell and convey said land; that the Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2009.

IN WITNESS WHEREOF, Grantor has hereunto set her hand and seal the day and year first above written.

Signed, sealed and delivered in our presence: SHARRON D. RUTHERFORD . Witness Print: ATUI Print:0

miller Phyllis M. Wright

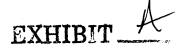
STATE OF FLORIDA COUNTY OF ALACHUA

The foregoing instrument was acknowledged before me this  $\frac{15}{2}$  day of January, 2010 by Phyllis M. Wright, who is D personally known to me or D who has produced Florida Drivers License as identification.

Notary Public - State of Florida

Sign: SHARRON D. RUTHERFORD Print:

My Commission Expires



A tract of land situated in Woodbridges Subdivision of Stringfellow Farm in Section 14, Township 8 South, Range 18 East, Alachua County, Florida, said tract of land being more particularly described as follows:

Commence at an iron pipe being the Northwest corner of Lot 11 of the aforementioned Wodbridges Subdivision of Stringfellow Farm as per plat recorded in Plat Book "C", Page 68-A, of the Public Records of Alachua County, Florida for a point of reference and run South 01 deg.43'17" East, along the West line of said Lot 11, a distance of 899.72 feet to a concrete monument at the intersection of said West line with the North right of way line of the abandoned Seaboard Coastline Railroad (200 foot right of way); thence run South 83 deg.52'00" East, along said North right of way line, a distance of 423.98 feet to a concrete monument; thence run North 01 deg.43'17" West, parallel with said West line, a distance of 210.00 feet to the True Point of Beginning; thence continue North 01 deg.43'17" West, parallel with said West line, a distance of 210.00 feet; thence run South 01 deg.43'17" East, parallel with said West line, a distance of 229.71 feet; thence run North 83 deg.52'00" West, parallel with said North right of way line of the abandoned Seaboard Coastline Railroad, a distance of 211.99 fet to the True Point of Beginning.

TOGETHER WITH AND SUBJECT TO a Non-exclusive Easement for the purpose of ingress-egress and public utilities, said Easement being described as the following tract of land:

A tract of land situated in Lot 11 of Woodbridge's Subdivision of the Stringfellow Farm, Alachua, Florida, a subdivision as recorded in Plat Book "C", Page 68-A of the Public Records of Alachua County, Florida, said tract of land being more particularly described as follows:

Commence at the Northwest corner of Lot 11 of the aforementioned Woodbridge's Subdivision for a point of reference; thence run South 01 deg.43'17" East, along the West line of said Lot 11, a distance of 21.19 feet to the Southerly Right of Way line of U.S. Highway No. 441; thence run South 84 deg.05'00" East, along said Right of Way line, a distance of 144.25 feet to a curve, said curve being concave Southerly, having a central angle of 00 deg.58'10", a radius of 2210.86 feet, a length of 37.41 feet and a chord bearing and distance of South 83 deg.35'45" East, 37.41 feet; thence run Southeasterly with said curve and along said Right of Way line, a distance of 37.41 feet to the point of beginning; thencer un South 01 deg.43'17" East, parallel to said West line of Lot 11, a distance of 878.90 feet to the Northerly Right of Way line of the abandoned Seaboard Coastline Railroad (200 feet Right of Way); thence run South 83 deg.52'00" East, along said Northerly Right of Way line, a distance of 60.57 feet; thence run North 01 deg.43'17" West, a distance of 179.72 feet; thence run South 83 deg.52'00" East, parallel to said Seaboard Coastline Railroad, a distance of 787.08 feet to a point, said point being the beginning of the arc of a culde-sac, said cul-de-sac being concave Westerly, having a central angle of 262 deg.49'10", a radius of 40.00 feet, an arc length of 183.49 feet and a chord bearing and distance of North 06 deg.08'00" East, 60.00 feet; thence run along said arc a distance of 183.49 feet to the end of said arc; thence run North 83 deg.52'00" West, a distance of 371.38 feet; thence run North 01 deg.43'17" West, a distance of 575.66 feet to a point, said point being a point on a curve of the Southerly Right of Way line of U.S. Highway No. 441, said curve being concave Southerly, having a central angle of 01 deg.39'43", a radius of 2210.86 feet, a length of 64.13 feet and a chord bearing and distance of North 71 deg.02'09" West, 64.13 feet; thence run Northwesterly with said curve and along said Right of Way line, a distance of 64.13 feet; thence run South 01 deg.43'17" East, a distance of 590.04 feet; thence run North 83 deg.52'00" West, a distance of 363.41 feet; thence run North 01 deg.43'17" West, a distance of 636.96 feet to a point, said point being a point on a curve of the Southerly Right of Way line of U.S. Highway No. 441, said curve being concave Southerly, having a central angle of 01 deg.34'34", a radius of 2210.86 feet, a length of 60.82 feet and a chord bearing and distance of North 82 deg.19'27" West, 60.82 feet; thence run Northwesterly with said curve and along said Right of Way, a distance of 60.82 feet to the point of beginning.

## Parcel: 03212-010-000

## Search Date: 3/22/2017 at 3:49:29 PM

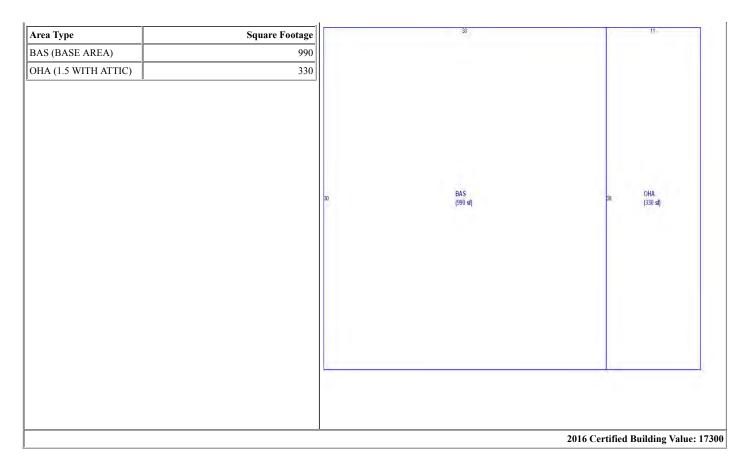
Taxpayer:	M & M PROPERTIES OF ALACHUA	Legal: HARRELL IND PARK AN UNRECORDED PLAT OF LOTS 11 THRU 14 OF
Mailing:	14616 NW 128TH TER ALACHUA, FL 32615	WOODBRIDGE S/D PB C-68A LOT 10 OR 3931/1841
Location:	14616 NW 128TH TER ALACHUA	
Sec-Twn-Rng:	14-08-18	
Property Use:	02500 - Service Shops	
Tax Jurisdiction:	Alachua - 1700	
Area:	Progress Center	
Subdivision:	Harrells Indus. Pk.	

	Property	Land	Land	Building	Misc	Total	Deferred	County	School	County	School	County	School	Total
YeaR	Use	Assessed Value	Just Value	Value	Value	Just Value	Value	Assessed	Assessed	Exempt	Exempt	Taxable	Taxable	Taxes
2016	Service Shops	54400	54400	17300	2500	74200	2240	71960	74200	0	0	71960	74200	1832.98
2015	Service Shops	54400	54400	17300	2500	74200	8780	65420	74200	0	8780	65420	65420	1758.09
2014	Service Shops	54400	54400	17300	2500	74200	14720	59480	74200	0	14720	59480	59480	1640.28
2013	Service Shops	54400	54400	17300	2500	74200	20120	54080	74200	0	20120	54080	54080	1551.22
2012	Service Shops	54400	54400	17300	2500	74200	25030	49170	74200	0	25030	49170	49170	1462.18
2011	Service Shops	24900	24900	17300	2500	44700	0	44700	44700	0	0	44700	44700	1175.36
2010	Service Shops	29400	29400	19200	2500	51100	0	51100	51100	0	0	51100	51100	1321.45
2009	Service Shops	29400	29400	22300	2500	54200	0	54200	54200	0	0	54200	54200	1396.74
2008	Service Shops	29400	29400	23500	2500	55400	0	55400	0	0	0	55400	0	1291.74
2007	Service Shops	29400	29400	22800	2500	54700	0	54700	0	0	0	54700	0	1280.5

Land

Use	Zoning Type	Zoning Desc	Unit Type	Units
Service Shop	IG		Square Feet	45302
			2016 Certified Land Just Value: 54400	2016 Certified Land Assessed Value: 54400

		Building
Actual Year Built	1979	Footprint_file = 12577_12577.jpg
Effective Year Built	1979	
Building Quality	Average	
Building Style	Warehouse	
Building Use	Service Shop	
Bedrooms:		
Baths:		
Stories:	1.0	
Exterior Wall 1:	Pre-Finsh Metl	
Exterior Wall 2:	N/A	
Interior Wall 1:	Minimum/Mason	
Interior Wall 2:	N/A	
Floor Cover 1:	Fin Concrete	
Floor Cover 2:	N/A	
Roof Cover:	Minimum	
Roof Structure:	Wood Truss	
AC:	None	
Heating Type:	None	
Heating System:	None	
Total Square Feet:	1320	
Heated Square Feet:	1485	



Miscellaneous

Description	Unit Type	Units			
5221 - Stg 1	SF	224			
4680 - Paving 1	SF	33			
3883 - Fence CL	SF	3660			
2016 Certified Miscellaneous Value: 2500					

Sale							
Date	Price	Vac/Imp	Qualified	OR Book	OR Page	Instrument	
01/15/2010	125000	Ι	U	3931	1841	WD	
03/01/1985	100	Ι	U	1588	448	MS	
07/01/1982	0	Ι	U	1324	914	WD	

Permit

County Permit information is supplied by the Alachua County Office of Codes Enforcement. The Alachua County Office of Codes Enforcement and the Property Appraiser's Office assume no liability whatsoever associated with the use or misuse of this public information data and will not be held liable as to the validity, correctness, accuracy, completeness, and / or reliability of this data.

Permit Number	Permit Type	Issue Date	Final Date	Appraisal Date	Comment			
A6-11-2202	SS	06/07/2011		01/18/2012	STACK OFF BUILDING			

DIVISION OF CORPORATIONS



Department of State / Division of Corporations / Search Records / Detail By Document Number /

## **Detail by Entity Name**

Florida Limited Liability Company M&M PROPERTIES OF ALACHUA, LLC.

## Filing Information

Document Number	L09000066634
FEI/EIN Number	27-1658903
Date Filed	07/09/2009
State	FL
Status	ACTIVE
Last Event	REINSTATEMENT
Event Date Filed	01/02/2013

## Principal Address

14616 NW 128TH TERR ALACHUA, FL 32615

## Mailing Address

14616 NW 128TH TERR ALACHUA, FL 32615

## Registered Agent Name & Address

MARKLE, JAMES A 14616 NW 128TH TERR ALACHUA, FL 32615

## Authorized Person(s) Detail

Name & Address

Title MGRM

MARKLE, JAMES A 14616 NW 128TH TERR ALACHUA, FL 32615

## Annual Reports

Report Year	Filed Date
2015	01/19/2015
2016	01/14/2016
2017	01/12/2017

## **Document Images**

01/12/2017 ANNUAL REPORT	View image in PDF format
01/14/2016 ANNUAL REPORT	View image in PDF format
01/19/2015 ANNUAL REPORT	View image in PDF format
01/08/2014 ANNUAL REPORT	View image in PDF format
01/02/2013 REINSTATEMENT	View image in PDF format
01/10/2010 ANNUAL REPORT	View image in PDF format
07/09/2009 Florida Limited Liability	View image in PDF format



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			🥁 Get Bills by	y Email			
			PAID 2016-11-21 Receipt #16-00				
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	ALACHUA, FI						
5	Situs: 14616 NW 12	28TH TER					
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	Key: 1012441 code: 1700						
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1 of 2

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## Florida Department of Environmental Protection

Bob Martinez Center 2600 Blair Stone Road Tallahassee, Florida 32399-2400 Rick Scott Governor

Carlos Lopez-Cantera Lt. Governor

> Ryan E. Matthews Interim Secretary

## SELF-CERTIFICATION FOR A STORMWATER MANAGEMENT SYSTEM IN UPLANDS SERVING LESS THAN 10 ACRES OF TOTAL PROJECT AREA AND LESS THAN 2 ACRES OF IMPERVIOUS SURFACES

Owner(s)/Permittee(s):	M&M Properties of Alachua
File No:	0352619001EG
File Name:	TRANSMISSION DOCTOR
Site Address:	14616 NW 128th Ter Alachua FL - 32615 6803
County:	Alachua
Latitude:	29° 47' 22.6421"
Longitude:	-82° 28' 56.6814"
Total Project Area:	1.04
<b>Total Impervious Surface Area:</b>	0.35
Approximate Date of Commencement of Construction:	06/01/2017
<b>Registered Florida Professional:</b>	Carol Chadwick
License No.:	82560
Company:	eda engineers~surveyors~planners, inc.

Date: March 30, 2017

**Caitlyn Drane** certified through the Department's Enterprise Self-Service Application portal that the project described above was designed by the above-named Florida registered professional to meet the following requirements:

(a)The total project area involves less than 10 acres and less than 2 acres of impervious surface; (b)Activities will not impact wetlands or other surface waters;

(c)Activities are not conducted in, on, or over wetlands or other surface waters;

(d)Drainage facilities will not include pipes having diameters greater than 24 inches, or the hydraulic equivalent, and will not use pumps in any manner;

(e)The project is not part of a larger common plan, development, or sale; and

(f)The project does not:

1. Cause adverse water quantity or flooding impacts to receiving water and adjacent lands;

2. Cause adverse impacts to existing surface water storage and conveyance capabilities;

3. Cause a violation of state water quality standards; or

4.Cause an adverse impact to the maintenance of surface or ground water levels or surface water flows established pursuant to s. 373.042 or a work of the district established pursuant to s. 373.086, F.S.

This certification was submitted before initiation of construction of the above project. The system is designed, and will be operated and maintained in accordance with applicable rules adopted pursuant to part IV of chapter 373, F.S. There is a rebuttable presumption that the discharge from such system will comply with state water quality standards. Therefore, construction, alteration, and maintenance of the stormwater management system serving this project is authorized in accordance with s.403.814(12), F.S.

In accordance with s. 373.416(2), F.S., if ownership of the property or the stormwater management system is sold or transferred to another party, continued operation of the system is authorized only if notice is provided to the Department within 30 days of the sale or transfer. This notice can be submitted to:

FDEP Northeast District 8800 Baymeadows Way West Jacksonville, FL 32256

This certification was submitted along with the following electronic documents:

File Description	
Drainage Design Note	
Plans	

If you have submitted this certification as a Florida Registered Professional, you may wish to sign and seal this certification, and return a copy to the Department, in accordance with your professional practice act requirements under Florida Statutes.

I, <u>Carol Chadwick</u>, License No. <u>82560</u>, do hereby certify that the above information is true and accurate, based upon my knowledge, information and belief. In the space below, affix signature, date, seal, company name, address and certificate of authorization (if applicable).

eda engineers~surveyors~planners, inc. 2404 NW 43rd St. Gainesville, FL 32606 EB 2389

This sealed certification may be submitted to the Department, either electronically (as an attachment in Adobe PDF or other secure, digital format) at Erp.selfcerts@dep.state.fl.us, or as a hardcopy, at the postal address below:

FDEP Northeast District 8800 Baymeadows Way West Jacksonville, FL 32256

